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ENGINEERS MEASURE THE STRENGTH OF A MONOLITHIC BRICK TEST SLAB

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# PUBLIC ROADS

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D. M. BEACH, *Editor*

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*The reports of research published in this magazine are necessarily qualified by the conditions of the tests from which the data are obtained. Whenever it is deemed possible to do so, generalizations are drawn from the results of the tests; and, unless this is done, the conclusions formulated must be considered as specifically pertinent only to described conditions.*

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# TESTS OF MONOLITHIC BRICK PAVEMENT SLABS

BY THE DIVISION OF TESTS, PUBLIC ROADS ADMINISTRATION

Reported by W. F. KELLERMANN, Materials Engineer

IN the construction of brick pavements with concrete bases it has been customary during recent years to lay the brick in a bedding course of sand, or a mixture of sand and bitumen, that has been spread on the hardened concrete base, and to fill the joints between the brick with bitumen. With this type of construction, known as the two-course type, the brick wearing course could not be expected to add appreciably to the strength of the concrete base.

A number of years ago monolithic brick pavements were constructed by laying the brick in the fresh concrete of the base, rolling with a light roller, and filling the joints with cement grout. At that time the need for expansion joints was not fully appreciated and much trouble was experienced with blow-ups caused by expansion. Also the riding qualities of monolithic brick pavements did not compare favorably with the riding qualities of other high-type pavements. As a result, the monolithic type fell into disuse and the two-course type was used exclusively in brick pavement construction.

With the introduction of surface vibration as a means of compacting the concrete in pavement slabs, new possibilities were presented for the construction of monolithic brick pavements. Consequently, several monolithic brick pavements have been built in which the brick have been embedded in the concrete base by means of high-frequency surface vibrators, rather than by rollers.

The procedure is first to build the usual concrete base using a finishing machine equipped with surface vibrators. The brick are then immediately placed on the fresh concrete and vibrated with a machine designed especially for the purpose. This machine is built along the same general lines as the conventional finishing machine except that in place of the conventional screeds there is a special screed consisting of a series of steel channels. These channels are vibrated as they ride over the brick, with the result that the brick are brought to proper elevation, at the same time being forced down about  $\frac{1}{8}$  inch into the concrete. A thin sand-cement grout is then squeegeed over the brick surface, filling the joints. After the grout has taken its initial set, a second application of somewhat stiffer grout is applied.

Some years ago monolithic pavements were constructed by placing the brick in the fresh concrete of the base and filling the joints with mortar. This type of pavement was discarded in favor of the conventional two-course brick pavement (1) because of trouble experienced with blow-ups which resulted from the absence of adequate expansion joints and (2) because the riding qualities were not as good as those of other high type pavements.

This report describes a recent development in the technique of constructing monolithic brick pavements and discusses the results of a series of tests made to determine the quality of pavement slabs so constructed as compared with the quality of pavement slabs constructed entirely of concrete.

The results of these tests have indicated that monolithic brick slabs, when tested with the concrete in tension, developed higher flexural strengths than comparable slabs of plain concrete. However, when the brick surface was placed in tension, the monolithic brick slabs developed much lower strengths than comparable slabs of plain concrete. In the latter case the angle at which the brick courses were placed with respect to the loading knife edge of the testing machine had an effect upon the results obtained.

In tests made with the brick surface in tension, the bond between the concrete and the brick was not affected by 100 applications of load equal to 50 percent of the ultimate load.

A final mortar finish over the entire surface is obtained by use of a burlap drag. It is claimed that this type of construction eliminates the objectionable features of the old-type monolithic pavement and that pavements constructed by this method are superior in many respects to the two-course type.

## TWO SERIES OF TESTS MADE

In order to determine the flexural strengths of monolithic brick slabs constructed in this manner as compared with those of plain concrete compacted by surface vibration, two series of laboratory tests (series A and series B) were initiated in which the following variables were studied:

### SERIES A

1. Effect of total depth of slab.
2. Effect of testing the top surface of the slab, as cast, in tension as compared with the bottom surface.
3. Effect of angle of brick courses with respect to the longitudinal axis of slab.
4. Effect of repeated loadings.

### SERIES B

1. Effect of testing the top surface of the slab in tension as compared with the bottom surface.
2. Effect of angle of brick courses with respect to longitudinal axis of slab.
3. Effect of richness of concrete mix.

The brick used in these tests were the conventional 3- by 4- by 8 $\frac{1}{2}$ -inch vertical fiber lug brick normally used in pavement construction. Grout used as a filler was proportioned 1:2 by dry loose volume with sufficient water to give the desired consistency. Tables 1 and 2 give the physical properties of the aggregate while the mix data for the concrete and the grout are given in tables 3 and 4, respectively. Different cements were used in the two series. Results of strength tests on the two cements are as follows:

	<i>Tensile strength, lb. per sq. in.</i>
Series A:	
At 7 days.....	405
At 28 days.....	485
Series B:	
At 7 days.....	380
At 28 days.....	435

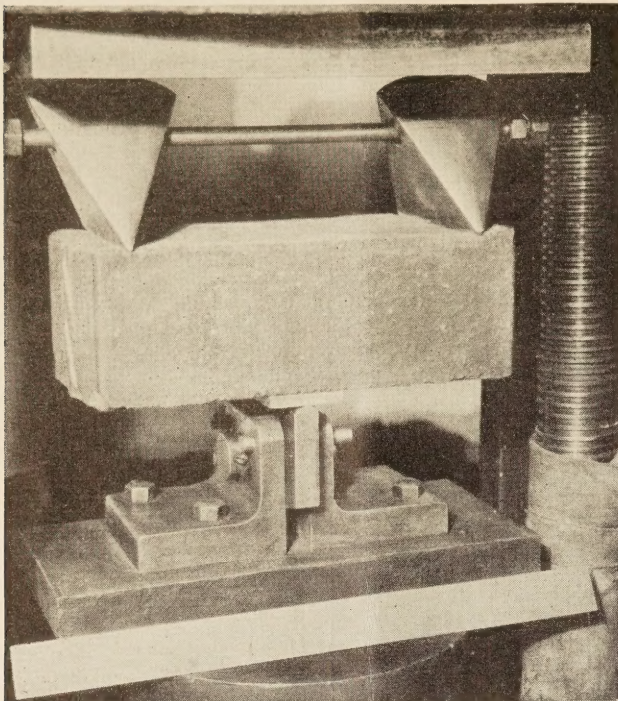


FIGURE 1.—SET-UP USED IN DETERMINING FLEXURAL STRENGTH OF PAVING BRICK.

TABLE 1.—Physical properties of aggregates for monolithic brick pavement slabs

Aggregates	Character of aggregate	Bulk specific gravity	Weight per cubic-foot		Voids, dry rodded	Absorption	Los Angeles abrasion loss <sup>1</sup>
			Solid	Dry rodded			
For concrete:			Pounds	Pounds	Percent	Percent	Percent
Sand, series A	Quartz	2.59	162	106	35	1.00	-----
Sand, series B	do	2.57	160	105	34	1.50	-----
Gravel	do	2.58	161	107	34	.99	28.0
For grout: Sand	do	2.60	162	<sup>2</sup> 90	<sup>2</sup> 44	1.00	-----

<sup>1</sup> Grading A used.      <sup>2</sup> Dry loose.

TABLE 2.—Grading of aggregates for monolithic brick pavement slabs

Sieve size	Percentage retained			
	Mortar sand	Concrete sand		Gravel
		Series A	Series B	
1½-inch	0	0	0	0
1-inch	0	0	0	38
¾-inch	0	0	0	57
½-inch	0	0	0	86
No. 4	0	1	1	100
No. 8	0	13	16	100
No. 16	0	25	31	100
No. 30	17	42	54	100
No. 50	65	74	83	100
No. 100	94	95	96	100
Fineness modulus	1.76	2.50	2.81	7.43

Results of tests made on the brick are given in table 5. The set-up for determining the flexural strength of the brick is shown in figure 1.

In this report the term "pavement concrete" is used to designate concrete containing 6 sacks of cement per cubic yard, which is a class commonly used in pavements. The term "base concrete" is used to designate concrete containing 4½ sacks of cement per cubic yard,

which is a class frequently used in the construction of bases.

Pavement concrete was used in series A, with sufficient water to produce a slump of approximately 1 inch.

Test specimens consisted of slabs approximately 26 inches wide by 60 inches long and 7, 8, and 10 inches in depth. Both plain concrete and monolithic brick slabs were cast for each thickness.

TABLE 3.—Data on concrete mixes for monolithic brick pavement slabs

Proportions by weight, pounds	W/C by volume	W <sub>c</sub> by volume <sup>(1)</sup>	b/b <sub>0</sub> <sup>(2)</sup>	Mortar voids ratio	Sand in total aggregate, by weight	Cement factor	Slump
Series A: 94 : 173 : 367	0.65	0.145	0.77	1.87	Percent 32	Sacks per cu. yd. 6.0	Inches 1.0
Series B:							
94 : 184 : 356	.65	.145	.74	2.02	34	6.0	.75
94 : 253 : 451	.84	.148	.74	2.04	36	4.75	.75

<sup>1</sup> Water per unit volume of concrete.

<sup>2</sup> Apparent volume of coarse aggregate per unit volume of concrete, dry rodded basis.

TABLE 4.—Data on grout used in monolithic brick pavement slabs, series A and B

Proportions		W/C by volume	
By dry loose volume	By weight	First grout	Second grout
1:2	Pounds 94:180	1.01	0.86

TABLE 5.—Physical properties of paving brick used in monolithic brick pavement slabs

Test	Average <sup>1</sup>	Maximum	Minimum
Absorption	percent 4.54	5.67	3.54
Specific gravity	2.36	2.39	2.32
Standard rattler	percent <sup>2</sup> 13.6	-----	-----
Modulus of rupture—pounds per square inch	2,250	2,540	1,980
Crushing strength	do 13,380	21,690	12,830

<sup>1</sup> Each result is the average for 10 tests except where otherwise noted.

<sup>2</sup> One test.

The plain concrete slabs were constructed in the following manner: The form was filled and an excess of concrete placed on top in order to allow for subsidence. Two complete passes were then made over the entire surface with a platform vibrator operating at a frequency of 7,000 pulsations per minute. The slab was then struck off with a wooden screed. In the case of the monolithic brick slabs the form was filled to within 2½ inches of the top and the concrete vibrated in the same manner. The brick were then set in the fresh concrete and vibrated in the same manner as for the plain concrete slabs (see fig. 2). This procedure resulted in twice as much vibration of the concrete in the monolithic as in the plain concrete slabs. It was adopted because it followed actual practice for the two different types of construction in the field.

After vibrating, a 1:2 mortar grout sufficiently thin to penetrate the joints was squeegeed over the surface. As this grout stiffened it also subsided, leaving the joints low. A second and somewhat thicker grout was then applied to fill the joints. The final finish was with a

burlap drag, leaving a thin mortar top over the entire surface.

All slabs were kept under wet burlap for 2 days, after which they were removed from the molds and cured in a fog room at 70° F. until tested at 29 days. In the monolithic brick slabs, three different arrangements of the brick were used. In one case the brick were laid with the courses parallel to the longitudinal axis of the slab. In the second case they were laid transversely to the longitudinal axis of the slab; while in the third case they were laid at an angle of 45°.

One group of slabs was tested with the bottom as cast in tension; the other group was tested with the top or brick surface in tension. Those slabs tested with the bottom in tension were made in groups of three, of which one was a plain concrete slab and two were monolithic brick. When testing the concrete slab and one of the monolithic slabs, each slab was loaded to failure with one application of loading. This method of loading will be referred to as instantaneous loading. The other monolithic slab was given 100 applications of a load equivalent to one-half the ultimate load obtained in the test of the first monolithic slab. The specimen was then loaded to failure. This procedure was followed in order to determine whether 100 repetitions of 50 percent of the ultimate load would produce any separation of the brick surface from the concrete base. Table 6 gives a complete outline of the procedure together with the strength results. All slabs were tested in a portable testing machine, using a pair of calibrated, heat-treated steel beams as a load-measuring device. The cover illustration shows a set-up of the testing equipment. The specimens were tested as simple beams with the load applied at the center of a 54-inch span.

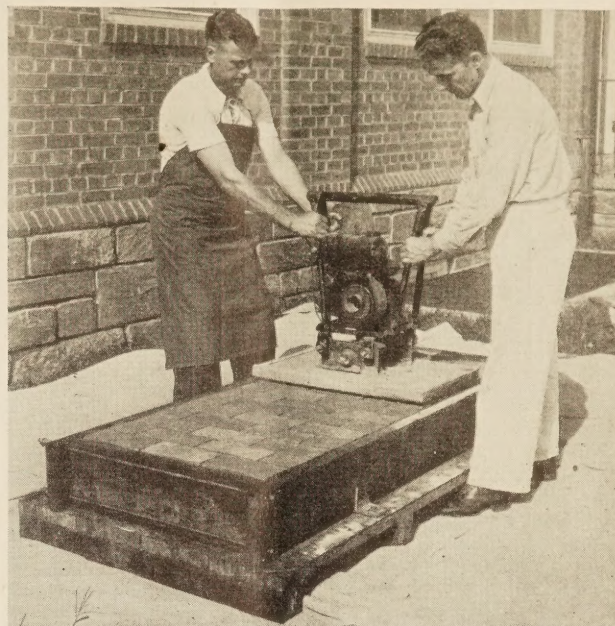


FIGURE 2.—VIBRATOR USED FOR COMPACTING PLAIN CONCRETE AND MONOLITHIC BRICK SLABS.

TABLE 6.—Summary of results of flexure tests on monolithic brick pavement slabs, series A<sup>1</sup>

Structure	Nominal depth of slab	Surface in tension <sup>2</sup>	Direction of knife edge of applied load <sup>3</sup>	Coarse aggregate broken	Flexural strength, age 29 days		Strength ratio <sup>4</sup>
					Lb./sq. in.	Percent	
Concrete	7	Bottom		55	730	100	
Monolithic brick	7	do	Parallel	60	868	119	
Do	7	do	do <sup>5</sup>	70	841	115	
Concrete	8	do		60	741	100	
Monolithic brick	8	do	Parallel	65	840	113	
Do	8	do	do <sup>5</sup>	55	788	106	
Concrete	10	do		50	728	100	
Monolithic brick	10	do	Parallel	65	859	118	
Do	10	do	do <sup>5</sup>	60	884	121	
Concrete	7	Top		55	797	100	
Monolithic brick	7	do	Perpendicular	65	636	80	
Do	7	do	45° angle	70	535	67	
Do	7	do	Parallel	45	341	43	
Concrete	8	do		60	845	100	
Monolithic brick	8	do	Perpendicular	50	724	86	
Do	8	do	45° angle	50	508	60	
Do	8	do	Parallel	40	367	43	
Concrete	10	do		50	776	100	
Monolithic brick	10	do	Perpendicular	60	790	102	
Do	10	do	45° angle	40	588	76	
Do	10	do	Parallel	55	443	57	

<sup>1</sup> Specimens 26 inches wide by 60 inches long (approximately); tested with center loading on 54-inch span after 29 days of moist curing.

<sup>2</sup> In all cases where the slabs were tested with the bottom surface in tension, each result is the average of two tests. All other values are individual test results.

<sup>3</sup> With respect to the line of continuous brick courses.

<sup>4</sup> Flexural strength computed with the neutral axis assumed at the center of the slab.

<sup>5</sup> Plain concrete slabs in each case were taken as standard.

<sup>6</sup> Given 100 repetitions of a load equal to 50 percent of the ultimate load of the corresponding slab and then tested to failure.

**RICHNESS OF CONCRETE MIXTURE A VARIABLE IN SERIES B**

In series B the same procedure of casting, curing, and testing was employed as in series A, except that in the brick slabs that were tested with the top, as cast, in

tension, the brick were placed at an angle of 45° with respect to the longitudinal axis instead of in three different ways, as was done in series A. Also, all specimens were tested by instantaneous loading. Tests were made at 28 days instead of at 29 days as in series A.

One variable, not included in series A but studied in series B, was the effect of the richness of the concrete mix. In series A pavement concrete was used in all slabs, while in series B both pavement concrete and base concrete were used. This was done in order to compare the strength of monolithic brick slabs of base concrete directly with the strength of vibrated pavement concrete of the same total thickness. The effect of depth of slab was not studied in series B, all specimens being 8 inches thick.

Strength results are tabulated in table 6 and shown in graphical form in figure 3. In computing the values for modulus of rupture it was assumed that the neutral axis was at the center of the slab. It is realized that this may not be a correct assumption, particularly in the case of the monolithic brick slabs. However, for purposes of determining comparative load-carrying capacity, this procedure is justified, since comparisons are made between monolithic brick and plain concrete specimens of equal thickness. The same comparisons could be made with the total breaking loads.

Considering first the results plotted in figure 3 for those specimens tested with the bottom, as cast, in tension (brick surface in compression) the monolithic brick slabs gave higher values for computed moduli of rupture than did those of plain concrete. This was true for slabs of all thicknesses tested. The modulus of rupture of the monolithic brick specimens tested instantaneously averaged 17 percent higher than that of the plain concrete, while for the monolithic slabs given 100 repetitions of 50 percent loading the modulus of rupture averaged 14 percent higher than that of the plain concrete. It will be noted that in two out of three cases the monolithic brick slabs given 100 repetitions of 50 percent of the ultimate load and then loaded to failure gave slightly lower strengths than did

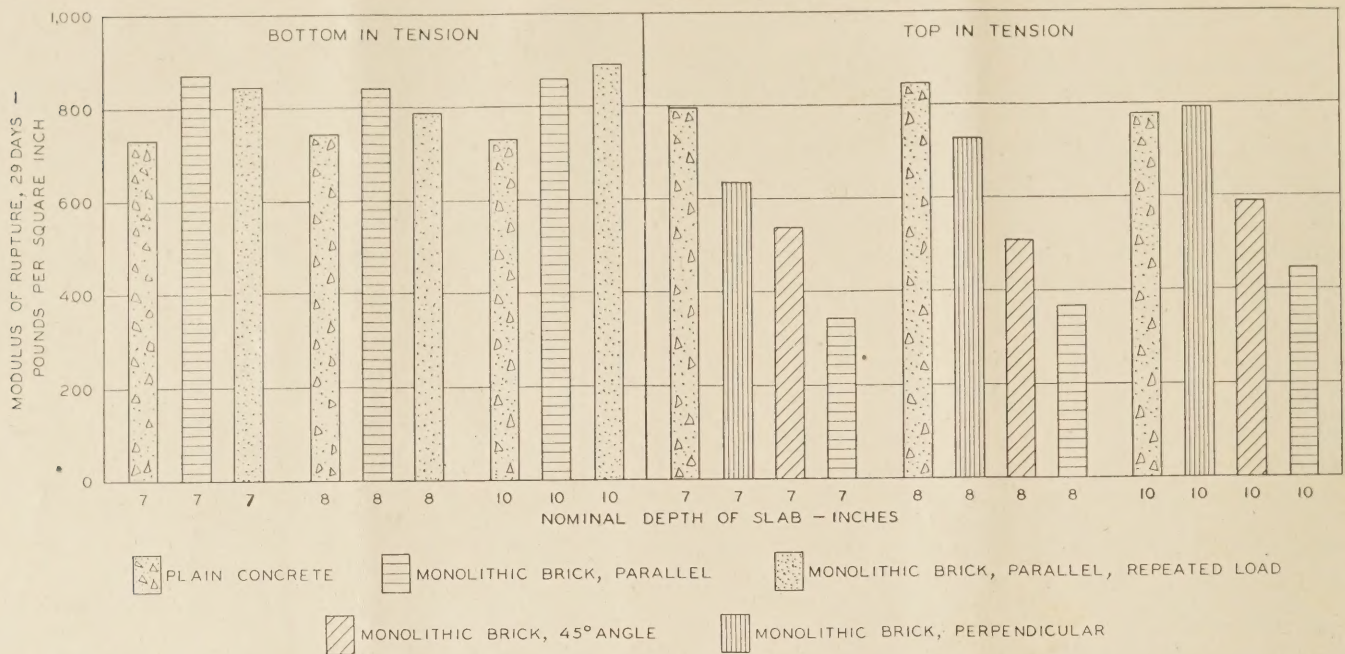


FIGURE 3.—RESULTS OF FLEXURE TESTS ON MONOLITHIC BRICK PAVEMENT SLABS, SERIES A, AT AGE OF 29 DAYS. "PARALLEL," "45° ANGLE," AND "PERPENDICULAR" REFER TO THE DIRECTION OF KNIFE EDGE OF APPLIED LOAD WITH RESPECT TO THE LINE OF CONTINUOUS BRICK COURSES. "REPEATED LOAD" INDICATES SLABS WERE GIVEN 100 REPETITIONS OF A LOAD EQUAL TO 50 PERCENT OF THE ULTIMATE LOAD OF THE CORRESPONDING SLAB AND THEN TESTED TO FAILURE.

the corresponding slabs tested with but one application of loading. It was anticipated that the repeated flexing of the slab might tend to destroy the bond between the brick surface and the concrete base but apparently this did not take place since the differences between the results obtained with the single loading and after repeated loading are within the limits of experimental error.

In all cases where the slabs were tested after repeated loadings, the percentage of brick broken at the section was less than in cases where they were tested with but one application of load. In the case of the 7-inch monolithic brick slabs tested by instantaneous loading the percentage of brick broken was 40 while for the companion slabs tested after repeated loading the percentage was 20. For the 8-inch slabs the differential was even greater whereas for the 10-inch slabs it was somewhat less.

Data for the slabs tested with the top surface, as cast, in tension are also plotted in figure 3. In this group no specimens were given repeated loadings. However, the brick were placed in three different directions so that there were four slabs in each group, one of plain concrete and three of monolithic brick. In each group the left bar represents the strength of the plain concrete while the second bar represents the strength of monolithic brick slabs in which the line of continuous courses of brick was perpendicular to the axis of the loading knife edge of the testing machine. In this case the maximum bending moment occurred at the center of the individual brick in three of the six brick courses in the cross section, while, due to the staggering of courses, the maximum bending moment occurred at the joint between brick and mortar in the other three cases. This is illustrated in figure 4-A, which shows a view of a fractured monolithic slab.

The third bar in the group gives the strength of monolithic brick slabs in which the direction of brick

courses was at an angle of 45° to the axis of the loading knife edge. In this test failure occurred at the joint between the mortar and the brick (see fig. 4-B). The fourth bar in each group gives the strength of monolithic brick slabs in which the courses of brick were parallel to the axis of the loading knife edge. As was to be expected from this procedure, failure occurred at the joint between the brick and the mortar as shown in figure 4-C.

#### STRENGTH AFFECTED BY DIRECTION OF BRICK COURSES

The direction of the courses of brick had a marked influence upon the strength. Of the three ways of placing the brick, the highest strengths were obtained by placing the line of brick courses perpendicular to the axis of the loading knife edge, while the lowest strengths were obtained by placing the line of brick courses parallel to the axis of the loading knife edge. Brick courses placed at an angle of 45° to the loading knife edge gave intermediate results. Because of the failure in bond between the brick and grout, the monolithic brick slabs tested with the brick in tension gave much lower results than did the plain concrete slabs, the only exception being the 10-inch monolithic slabs with the brick courses perpendicular to the axis of the loading knife edge, which gave results slightly higher than those obtained with the plain concrete.

In summary, it was found that monolithic brick slabs tested with the concrete surface in tension gave results about 17 percent higher than those for plain concrete of equal thickness. On the other hand, when tested with the brick surface in tension and with the courses of brick set parallel to the axis of the loading knife edge, monolithic brick averaged only about 50 percent of the strength of plain concrete. The direction of the brick courses in the slabs had a marked effect in this test. Those slabs in which the courses of brick were set

(Continued on page 97)



# VERTICAL CURVES FOR HIGHWAYS

BY THE DIVISION OF DESIGN, PUBLIC ROADS ADMINISTRATION

Reported by D. W. LOUTZENHEISER, Associate Highway Engineer

*Length of vertical curve.*—Any vertical curve on a highway should be as long as possible but the minimum length should be based on the sight distance requirements for the highway. The sight distances required are based on the assumed design speed for the highway. At any point on a highway the sight distance should be at least long enough that a vehicle traveling at the design speed can, upon sight of an object on the road, be brought to a stop before reaching it. Such sight distances are known as "nonpassing sight distances" and are not long enough on 2-lane roads for safe passing of vehicles in the same direction in the face of opposing traffic. Sight distances safe for passing on 2- and 3-lane roads are known as "passing sight distances." They rarely can be provided at all points, but they should be provided at frequent intervals.

The pamphlet "A Policy on Sight Distances for Highways," published by the American Association of State Highway Officials, presents a discussion of pertinent factors, calculations of and recommendations for minimum values of both nonpassing and passing sight distances for various highway design speeds. Tables 1 and 2 give the minimum lengths of vertical curve necessary for the various design speed-sight distance requirements. Reference is made to the sight distance policy for the numerical values of the various sight distances and formulas for calculation.

TABLE 1.—Minimum length of vertical curves to provide minimum nonpassing sight distance

A=algebraic difference of grades—percent	Length of vertical curve for assumed design speed of—				
	30 mi. per hr.	40 mi. per hr.	50 mi. per hr.	60 mi. per hr.	70 mi. per hr.
2	0	0	0.9	2.2	4.7
3	0	.6	2.2	4.6	7.4
4	.4	1.8	3.4	6.2	9.9
5	1.1	2.5	4.2	7.7	12.4
6	1.6	3.1	5.0	9.3	14.8
7	1.9	3.6	5.9	10.8	17.3
8	2.2	4.2	6.7	12.4	19.8
9	2.5	4.7	7.6	13.9	22.2
10	2.8	5.2	8.4	15.5	24.7
11	3.0	5.7	9.2	17.0	-----
12	3.3	6.2	10.1	18.6	-----
13	3.6	6.8	10.9	20.1	-----
14	3.8	7.3	11.8	21.7	-----
15	4.1	7.8	12.6	23.2	-----
16	4.4	8.3	13.5	-----	-----
17	4.7	8.8	14.3	-----	-----
18	4.9	9.3	15.1	-----	-----
19	5.2	9.8	16.0	-----	-----
20	5.5	10.4	16.8	-----	-----

*Calculations of elevations on vertical curves.* Terms used are as follows (see fig. 1):

- $g_1$  and  $g_2$ =grades, percent;
- A=algebraic difference of grades, percent;
- L=length of vertical curve, stations;
- t=distance from end of curve, feet; and
- d=vertical distance between grade line and vertical curve, feet.

The basic formulas are:

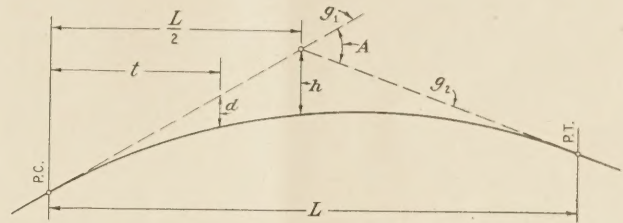


FIGURE 1.—CALCULATION OF ELEVATIONS ON VERTICAL CURVES.

TABLE 2.—Minimum length of vertical curves to provide minimum passing sight distance

A=algebraic difference of grades—percent	Length of vertical curve on 2-lane roads for assumed design speed of—					Length of vertical curve on 3-lane roads for assumed design speed of—		
	30 mi. per hr.	40 mi. per hr.	50 mi. per hr.	60 mi. per hr.	70 mi. per hr.	50 mi. per hr.	60 mi. per hr.	70 mi. per hr.
1.00	-----	-----	-----	6.0	22.0	-----	-----	0
1.25	-----	-----	0	13.2	29.2	-----	0	7.2
1.50	-----	-----	4.0	18.0	35.0	-----	2.0	12.0
1.75	-----	-----	7.4	21.4	40.9	-----	5.4	15.4
2.00	-----	0	10.0	24.5	46.7	-----	8.0	18.0
2.5	-----	3.6	13.6	30.6	-----	3.6	11.6	22.5
3.0	-----	6.0	16.3	36.8	-----	6.0	14.1	27.0
3.5	0	7.7	19.1	42.9	-----	7.7	16.4	31.5
4.0	1.0	9.0	21.8	49.0	-----	9.0	18.8	36.0
4.5	2.0	10.1	24.5	-----	-----	10.1	21.1	40.5
5.0	2.8	11.2	27.2	-----	-----	11.2	23.5	45.0
5.5	3.5	12.4	29.9	-----	-----	12.4	25.8	-----
6.0	4.0	13.5	32.7	-----	-----	13.5	28.2	-----
7	4.9	15.8	38.1	-----	-----	15.8	32.9	-----
8	5.6	18.0	43.6	-----	-----	18.0	37.6	-----
9	6.2	20.2	49.0	-----	-----	20.2	42.2	-----
10	6.9	22.5	-----	-----	-----	22.5	46.9	-----
11	7.6	24.8	-----	-----	-----	24.8	-----	-----
12	8.3	27.0	-----	-----	-----	27.0	-----	-----
13	9.0	29.2	-----	-----	-----	29.2	-----	-----
14	9.7	31.5	-----	-----	-----	31.5	-----	-----
15	10.4	33.8	-----	-----	-----	33.8	-----	-----
16	11.1	36.0	-----	-----	-----	36.0	-----	-----

$$g_1 - g_2 = A$$

$$h = \frac{AL}{8}$$

$$\frac{d}{h} = \frac{t^2}{2,500 L^2}$$

The value of d may be expressed as

$$d = \frac{A}{L} \times K,$$

where

$$K = \frac{t^2}{20,000}$$

Table 3 gives values of  $\frac{A}{L}$ . Values given are for whole numbers of A and L; for intermediate values interpolate or calculate  $\frac{A}{L}$  directly. Table 4 gives values of K for various values of t. Then d=value of  $\frac{A}{L}$  from table 3 times value of K from table 4.

Figure 2 is a nomograph which permits a more rapid but less accurate solution for one of the above factors when the other three are known.

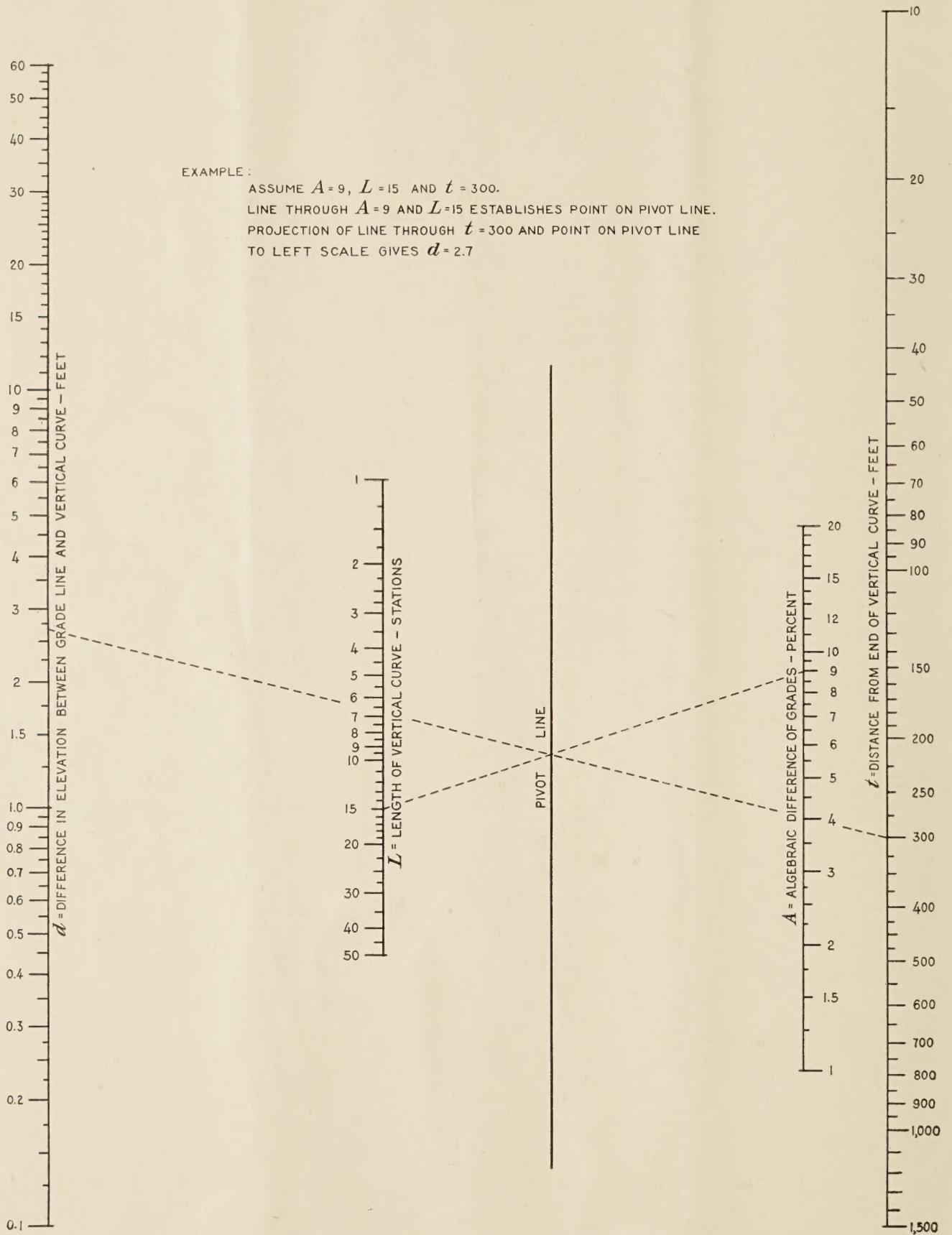


FIGURE 2.—NOMOGRAPH FOR CALCULATION OF ELEVATION OF A POINT ON A PARABOLIC VERTICAL CURVE.

TABLE 3.—Values of  $\frac{A}{L}$ , for values of A from 1 to 16 and values of L from 1 to 30

L (stations)	A=1 percent	A=2 percent	A=3 percent	A=4 percent	A=5 percent	A=6 percent	A=7 percent	A=8 percent	A=9 percent	A=10 percent	A=11 percent	A=12 percent	A=13 percent	A=14 percent	A=15 percent	A=16 percent
1	1.0000	2.0000	3.0000	4.0000	5.0000	6.0000	7.0000	8.0000	9.0000	10.0000	11.0000	12.0000	13.0000	14.0000	15.0000	16.0000
2	.5000	1.0000	1.5000	2.0000	2.5000	3.0000	3.5000	4.0000	4.5000	5.0000	5.5000	6.0000	6.5000	7.0000	7.5000	8.0000
3	.3333	.6667	1.0000	1.3333	1.6667	2.0000	2.3333	2.6667	3.0000	3.3333	3.6667	4.0000	4.3333	4.6667	5.0000	5.3333
4	.2500	.5000	.7500	1.0000	1.2500	1.5000	1.7500	2.0000	2.2500	2.5000	2.7500	3.0000	3.2500	3.5000	3.7500	4.0000
5	.2000	.4000	.6000	.8000	1.0000	1.2000	1.4000	1.6000	1.8000	2.0000	2.2000	2.4000	2.6000	2.8000	3.0000	3.2000
6	.1667	.3333	.5000	.6667	.8333	1.0000	1.1667	1.3333	1.5000	1.6667	1.8333	2.0000	2.1667	2.3333	2.5000	2.6667
7	.1428	.2857	.4286	.5714	.7143	.8571	1.0000	1.1428	1.2857	1.4286	1.5714	1.7143	1.8571	2.0000	2.1428	2.2857
8	.1250	.2500	.3750	.5000	.6250	.7500	.8750	1.0000	1.1250	1.2500	1.3750	1.5000	1.6250	1.7500	1.8750	2.0000
9	.1111	.2222	.3333	.4444	.5556	.6667	.7778	.8889	1.0000	1.1111	1.2222	1.3333	1.4444	1.5556	1.6667	1.7778
10	.1000	.2000	.3000	.4000	.5000	.6000	.7000	.8000	.9000	1.0000	1.1000	1.2000	1.3000	1.4000	1.5000	1.6000
11	.0909	.1818	.2727	.3636	.4545	.5454	.6364	.7273	.8182	.9091	1.0000	1.0909	1.1818	1.2727	1.3636	1.4545
12	.0833	.1667	.2500	.3333	.4167	.5000	.5833	.6667	.7500	.8333	.9167	1.0000	1.0833	1.1667	1.2500	1.3333
13	.0769	.1538	.2308	.3077	.3846	.4615	.5385	.6154	.6923	.7692	.8462	.9231	1.0000	1.0769	1.1538	1.2308
14	.0714	.1428	.2143	.2857	.3571	.4286	.5000	.5714	.6428	.7143	.7857	.8571	.9286	1.0000	1.0714	1.1428
15	.0667	.1333	.2000	.2667	.3333	.4000	.4667	.5333	.6000	.6667	.7333	.8000	.8667	.9333	1.0000	1.0667
16	.0625	.1250	.1875	.2500	.3125	.3750	.4375	.5000	.5625	.6250	.6875	.7500	.8125	.8750	.9375	1.0000
17	.0588	.1176	.1765	.2353	.2941	.3529	.4118	.4706	.5294	.5882	.6470	.7059	.7647	.8235	.8823	.9412
18	.0556	.1111	.1667	.2222	.2778	.3333	.3889	.4444	.5000	.5556	.6111	.6667	.7222	.7778	.8333	.8889
19	.0526	.1053	.1579	.2105	.2632	.3158	.3684	.4210	.4737	.5263	.5789	.6316	.6842	.7368	.7894	.8421
20	.0500	.1000	.1500	.2000	.2500	.3000	.3500	.4000	.4500	.5000	.5500	.6000	.6500	.7000	.7500	.8000
21	.0476	.0952	.1428	.1905	.2381	.2857	.3333	.3809	.4286	.4762	.5238	.5714	.6190	.6667	.7143	.7619
22	.0454	.0909	.1364	.1818	.2273	.2727	.3182	.3636	.4091	.4545	.5000	.5454	.5909	.6364	.6818	.7273
23	.0434	.0870	.1304	.1739	.2174	.2609	.3043	.3478	.3913	.4348	.4783	.5217	.5652	.6087	.6522	.6956
24	.0416	.0833	.1250	.1667	.2083	.2500	.2917	.3333	.3750	.4167	.4583	.5000	.5417	.5833	.6250	.6667
25	.0400	.0800	.1200	.1600	.2000	.2400	.2800	.3200	.3600	.4000	.4400	.4800	.5200	.5600	.6000	.6400
26	.0385	.0769	.1154	.1538	.1923	.2308	.2692	.3077	.3462	.3846	.4231	.4615	.5000	.5385	.5769	.6154
27	.0370	.0740	.1111	.1481	.1852	.2222	.2593	.2963	.3333	.3704	.4074	.4444	.4815	.5185	.5556	.5926
28	.0357	.0714	.1071	.1428	.1786	.2143	.2500	.2857	.3214	.3571	.3928	.4286	.4643	.5000	.5357	.5714
29	.0345	.0690	.1034	.1379	.1724	.2069	.2414	.2759	.3103	.3448	.3793	.4138	.4483	.4828	.5172	.5517
30	.0333	.0667	.1000	.1333	.1667	.2000	.2333	.2667	.3000	.3333	.3667	.4000	.4333	.4667	.5000	.5333

TABLE 4.—Values of K for values of t from 1 to 1,500

TABLE 4.—Values of K for values of t from 1 to 1,500—Con.

t	K	t	K	t	K	t	K
1	0.000	51	0.130	101	0.510	151	1.140
2	.000	52	.135	102	.520	152	1.155
3	.000	53	.140	103	.530	153	1.170
4	.001	54	.146	104	.541	154	1.186
5	.001	55	.151	105	.551	155	1.201
6	.002	56	.157	106	.562	156	1.217
7	.002	57	.162	107	.572	157	1.232
8	.003	58	.168	108	.583	158	1.248
9	.004	59	.174	109	.594	159	1.264
10	.005	60	.180	110	.605	160	1.280
11	.006	61	.186	111	.616	161	1.296
12	.007	62	.192	112	.627	162	1.312
13	.008	63	.198	113	.638	163	1.328
14	.010	64	.205	114	.650	164	1.345
15	.011	65	.211	115	.661	165	1.361
16	.013	66	.218	116	.673	166	1.378
17	.014	67	.224	117	.684	167	1.394
18	.016	68	.231	118	.696	168	1.411
19	.018	69	.238	119	.708	169	1.428
20	.020	70	.245	120	.720	170	1.445
21	.022	71	.252	121	.732	171	1.462
22	.024	72	.259	122	.744	172	1.479
23	.026	73	.266	123	.756	173	1.496
24	.029	74	.274	124	.769	174	1.514
25	.031	75	.281	125	.781	175	1.531
26	.034	76	.289	126	.794	176	1.549
27	.036	77	.296	127	.806	177	1.566
28	.039	78	.304	128	.819	178	1.584
29	.042	79	.312	129	.832	179	1.602
30	.045	80	.320	130	.845	180	1.620
31	.048	81	.328	131	.858	181	1.638
32	.051	82	.336	132	.871	182	1.656
33	.054	83	.344	133	.884	183	1.674
34	.058	84	.353	134	.898	184	1.693
35	.061	85	.361	135	.911	185	1.711
36	.065	86	.370	136	.925	186	1.730
37	.068	87	.378	137	.938	187	1.748
38	.072	88	.387	138	.952	188	1.767
39	.076	89	.396	139	.966	189	1.786
40	.080	90	.405	140	.980	190	1.805
41	.084	91	.414	141	.994	191	1.824
42	.088	92	.423	142	1.008	192	1.843
43	.092	93	.432	143	1.022	193	1.862
44	.097	94	.442	144	1.037	194	1.882
45	.101	95	.451	145	1.051	195	1.901
46	.106	96	.461	146	1.066	196	1.921
47	.110	97	.470	147	1.080	197	1.940
48	.115	98	.480	148	1.095	198	1.960
49	.120	99	.490	149	1.110	199	1.980
50	.125	100	.500	150	1.125	200	2.000

t	K	t	K	t	K	t	K
201	2.020	251	3.150	301	4.530	351	6.160
202	2.040	252	3.175	302	4.560	352	6.195
203	2.060	253	3.200	303	4.590	353	6.230
204	2.081	254	3.226	304	4.621	354	6.266
205	2.101	255	3.251	305	4.651	355	6.301
206	2.122	256	3.277	306	4.682	356	6.337
207	2.142	257	3.302	307	4.712	357	6.372
208	2.163	258	3.328	308	4.743	358	6.408
209	2.184	259	3.354	309	4.774	359	6.444
210	2.205	260	3.380	310	4.805	360	6.480
211	2.226	261	3.406	311	4.836	361	6.516
212	2.247	262	3.432	312	4.867	362	6.552
213	2.268	263	3.458	313	4.898	363	6.588
214	2.290	264	3.485	314	4.930	364	6.625
215	2.311	265	3.511	315	4.961	365	6.661
216	2.333	266	3.538	316	4.993	366	6.698
217	2.354	267	3.564	317	5.024	367	6.734
218	2.376	268	3.591	318	5.056	368	6.771
219	2.398	269	3.618	319	5.088	369	6.808
220	2.420	270	3.645	320	5.120	370	6.845
221	2.442	271	3.672	321	5.152	371	6.882
222	2.464	272	3.699	322	5.184	372	6.919
223	2.486	273	3.726	323	5.216	373	6.956
224	2.509	274	3.754	324	5.249	374	6.994
225	2.531	275	3.781	325	5.281	375	7.031
226	2.554	276	3.809	326	5.314	376	7.069
227	2.576	277	3.836	327	5.346	377	7.106
228	2.599	278	3.864	328	5.379	378	7.144
229	2.622	279	3.892	329	5.412	379	7.182
230	2.645	280	3.920	330	5.445	380	7.220
231	2.668	281	3.948	331	5.478	381	7.258
232	2.691	282	3.976	332	5.511	382	7.296
233	2.714	283	4.004	333	5.544	383	7.334
234	2.738	284	4.033	334	5.578	384	7.373
235	2.761	285	4.061	335	5.611	385	7.411
236	2.785	286	4.090	336	5.645	386	7.450
237	2.808	287	4.118	337	5.678	387	7.488
238	2.832	288	4.147	338	5.712	388	7.527
239	2.856	289	4.176	339	5.746	389	7.566
240	2.880	290	4.205	340	5.780	390	7.605
241	2.904	291	4.234	341	5.814	391	7.644
242	2.928	292	4.263	3			

TABLE 4.—Values of K for values of t from 1 to 1,500—Con.

Table with 8 columns (t, K, t, K, t, K, t, K) containing values for t from 401 to 500.

TABLE 4.—Values of K for values of t from 1 to 1,500—Con.

Table with 8 columns (t, K, t, K, t, K, t, K) containing values for t from 801 to 1,500.

TABLE 4.—Values of *K* for values of *t* from 1 to 1,500—Con.

<i>t</i>	<i>K</i>	<i>t</i>	<i>K</i>	<i>t</i>	<i>K</i>	<i>t</i>	<i>K</i>
1,201	72.120	1,246	77.626	1,291	83.334	1,336	89.245
1,202	72.240	1,247	77.750	1,292	83.463	1,337	89.378
1,203	72.360	1,248	77.875	1,293	83.592	1,338	89.512
1,204	72.481	1,249	78.000	1,294	83.722	1,339	89.646
1,205	72.601	1,250	78.125	1,295	83.851	1,340	89.780
1,206	72.722			1,296	83.981		
1,207	72.842	1,251	78.250	1,297	84.110	1,341	89.914
1,208	72.963	1,252	78.375	1,298	84.240	1,342	90.048
1,209	73.084	1,253	78.500	1,299	84.370	1,343	90.182
1,210	73.205	1,254	78.626	1,300	84.500	1,344	90.317
1,211	73.326	1,255	78.751			1,345	90.451
1,212	73.447	1,256	78.877	1,301	84.630	1,346	90.586
1,213	73.568	1,257	79.002	1,302	84.760	1,347	90.720
1,214	73.690	1,258	79.128	1,303	84.890	1,348	90.855
1,215	73.811	1,259	79.254	1,304	85.021	1,349	90.990
1,216	73.933	1,260	79.380	1,305	85.151	1,350	91.125
1,217	74.054			1,306	85.282		
1,218	74.176	1,261	79.506	1,307	85.412	1,351	91.260
1,219	74.298	1,262	79.632	1,308	85.543	1,352	91.395
1,220	74.420	1,263	79.758	1,309	85.674	1,353	91.530
1,221	74.542	1,264	79.885	1,310	85.805	1,354	91.666
1,222	74.664	1,265	80.011			1,355	91.801
1,223	74.786	1,266	80.138	1,311	85.936	1,356	91.937
1,224	74.909	1,267	80.264	1,312	86.067	1,357	92.072
1,225	75.031	1,268	80.391	1,313	86.198	1,358	92.208
1,226	75.154	1,269	80.518	1,314	86.330	1,359	92.344
1,227	75.276	1,270	80.645	1,315	86.461	1,360	92.480
1,228	75.399			1,316	86.593		
1,229	75.522	1,271	80.772	1,317	86.724	1,361	92.616
1,230	75.645	1,272	80.899	1,318	86.856	1,362	92.752
1,231	75.768	1,273	81.026	1,319	86.988	1,363	92.888
1,232	75.891	1,274	81.154	1,320	87.120	1,364	93.025
1,233	76.014	1,275	81.281			1,365	93.161
1,234	76.138	1,276	81.409	1,321	87.252	1,366	93.298
1,235	76.261	1,277	81.536	1,322	87.384	1,367	93.434
1,236	76.385	1,278	81.664	1,323	87.516	1,368	93.571
1,237	76.508	1,279	81.792	1,324	87.649	1,369	93.708
1,238	76.632	1,280	81.920	1,325	87.781	1,370	93.845
1,239	76.756			1,326	87.914		
1,240	76.880	1,281	82.048	1,327	88.046	1,371	93.982
1,241	77.004	1,282	82.176	1,328	88.179	1,372	94.119
1,242	77.128	1,283	82.304	1,329	88.312	1,373	94.256
1,243	77.252	1,284	82.433	1,330	88.445	1,374	94.393
1,244	77.377	1,285	82.561			1,375	94.531
1,245	77.501	1,286	82.690	1,331	88.578	1,376	94.669
		1,287	82.818	1,332	88.711	1,377	94.806
		1,288	82.947	1,333	88.844	1,378	94.944
		1,289	83.076	1,334	88.978	1,379	95.082
		1,290	83.205	1,335	89.111	1,380	95.220

TABLE 4.—Values of *K* for values of *t* from 1 to 1,500—Con.

<i>t</i>	<i>K</i>	<i>t</i>	<i>K</i>	<i>t</i>	<i>K</i>	<i>t</i>	<i>K</i>
1,381	95.358	1,411	99.546	1,441	103.824	1,471	108.192
1,382	95.496	1,412	99.687	1,442	103.968	1,472	108.339
1,383	95.634	1,413	99.828	1,443	104.112	1,473	108.486
1,384	95.773	1,414	99.970	1,444	104.257	1,474	108.634
1,385	95.911	1,415	100.111	1,445	104.401	1,475	108.781
1,386	96.050	1,416	100.253	1,446	104.546	1,476	108.929
1,387	96.188	1,417	100.394	1,447	104.690	1,477	109.076
1,388	96.327	1,418	100.536	1,448	104.835	1,478	109.224
1,389	96.466	1,419	100.678	1,449	104.980	1,479	109.372
1,390	96.605	1,420	100.820	1,450	105.125	1,480	109.520
1,391	96.744	1,421	100.962	1,451	105.270	1,481	109.668
1,392	96.883	1,422	101.104	1,452	105.415	1,482	109.816
1,393	97.022	1,423	101.246	1,453	105.560	1,483	109.964
1,394	97.162	1,424	101.389	1,454	105.706	1,484	110.113
1,395	97.301	1,425	101.531	1,455	105.851	1,485	110.261
1,396	97.440	1,426	101.674	1,456	105.997	1,486	110.410
1,397	97.580	1,427	101.816	1,457	106.142	1,487	110.558
1,398	97.720	1,428	101.959	1,458	106.288	1,488	110.707
1,399	97.860	1,429	102.102	1,459	106.434	1,489	110.856
1,400	98.000	1,430	102.245	1,460	106.580	1,490	111.005
1,401	98.140	1,431	102.388	1,461	106.726	1,491	111.154
1,402	98.280	1,432	102.531	1,462	106.872	1,492	111.303
1,403	98.420	1,433	102.674	1,463	107.018	1,493	111.452
1,404	98.561	1,434	102.818	1,464	107.165	1,494	111.602
1,405	98.701	1,435	102.961	1,465	107.311	1,495	111.751
1,406	98.842	1,436	103.105	1,466	107.458	1,496	111.901
1,407	98.982	1,437	103.248	1,467	107.604	1,497	112.050
1,408	99.123	1,438	103.392	1,468	107.751	1,498	112.200
1,409	99.264	1,439	103.536	1,469	107.898	1,499	112.350
1,410	99.405	1,440	103.680	1,470	108.045	1,500	112.500

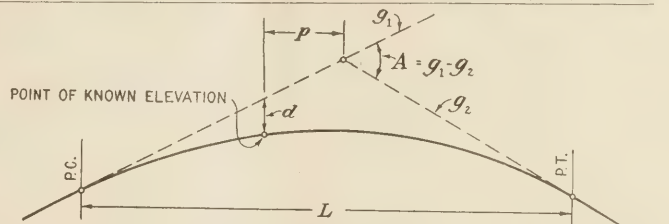


FIGURE 3.—LENGTH OF VERTICAL CURVE TO PASS THROUGH A POINT OF KNOWN ELEVATION.

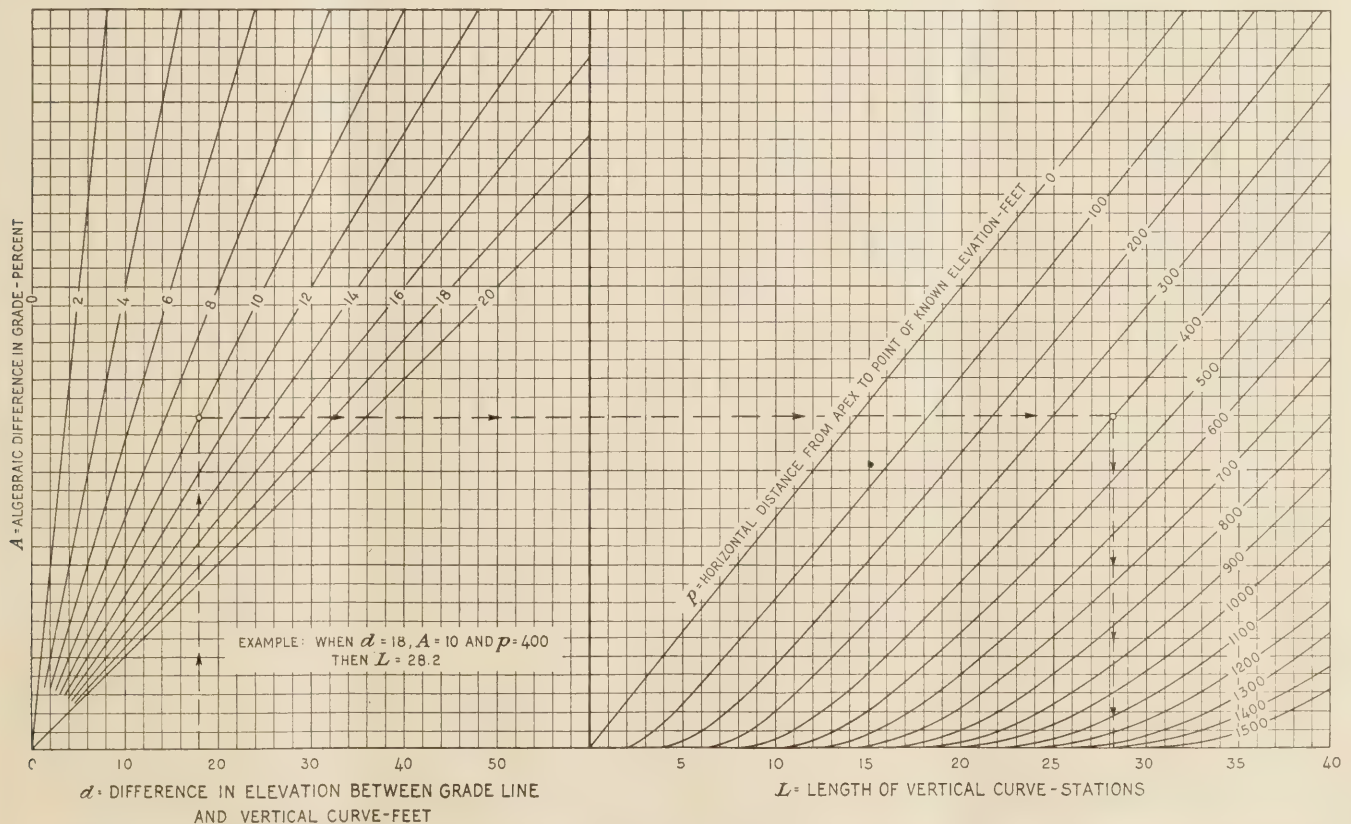


FIGURE 4.—GRAPHICAL SOLUTION FOR LENGTH OF VERTICAL CURVE TO PASS THROUGH A POINT OF KNOWN ELEVATION.

Sheet 4 of 10

PROJECT 121-C

Length V. C. = 600 Ft. h = 1.875  
 Station of V. P. I. = 83+50 Elevation of V. P. I. = 122.00  
 A = 2.50 A/L (To Nearest 0.0001) = 0.4167

Station	Grade, per-cent, G	Elevation on grade line	t (from table 2)	k (from table 2)	d (d=Ah-L)	Elevation		Difference in elevation between ground and grade at centerline	
						Ground	Grade	Out	Fill
80+00	+0.50	120.25	-	-	-	120.4	120.25	0.1	-
+50	PC	120.50	0	0	0	121.0	120.50	0.5	-
81+00		120.75	50	0.125	0.05	121.4	120.70	0.7	-
+50		121.00	100	0.500	0.21	121.6	120.79	0.8	-
82+00		121.25	150	1.125	0.47	121.8	120.78	1.0	-
+50		121.50	200	2.000	0.83	121.5	120.67	0.8	-
83+00		121.75	250	3.125	1.30	120.9	120.45	0.4	-
+50	PI	122.00	300	4.500	1.88	120.5	120.12	0.4	-
84+00		121.00	250	3.125	1.30	119.6	119.70	-	0.1
+50		120.00	200	2.000	0.83	118.4	119.17	-	0.8
85+00		119.00	150	1.125	0.47	117.9	118.53	-	0.6
+50		118.00	100	0.500	0.21	117.4	117.79	-	0.4
86+00		117.00	50	0.125	0.05	116.7	116.95	-	0.3
+50	PT	116.00	0	0	0	116.2	116.00	0.2	-
87+00	-2.00	115.00	-	-	-	116.7	115.00	1.7	-

FIGURE 5.—SAMPLE SHEET FOR CALCULATION OF A VERTICAL CURVE.

Length of vertical curve to pass through a point of known elevation. Values of the following are known (see fig. 3):

- A=algebraic difference of grades, percent;
- p=horizontal distance from apex to point of known elevation, feet; and
- d=difference in elevation between grade line and vertical curve at point of known elevation, feet.

It is desired to determine L, the length of vertical curve (in stations) to pass through point of known elevation. The following basic formula is used:

$$d = \frac{A(50L - p)^2}{20,000L}$$

A direct graphical solution for value of L is given in figure 4. The steps in using this figure are:

1. Enter at value of d (lower left scale) and project upward to value of A (left sloping lines).
2. Project horizontally across to value of p (right sloping lines).
3. Project down to read value of L (lower right scale).

Figure 4 may also be used to solve for value of d when values of A, L, and p are known.

Figure 5 is a sample sheet for calculation of a vertical curve using the values of tables 3 and 4.

# COTTON FABRIC FOR EROSION CONTROL

## REPORT OF EXPERIMENTS ON ROADSIDE SLOPES AND DITCHES IN MISSISSIPPI

### BY DISTRICT 8, PUBLIC ROADS ADMINISTRATION

Reported by W. B. KING, Associate Bridge Construction Engineer

COTTON FABRIC was used experimentally for erosion control on roadside slopes and ditches on eight test sections in Mississippi during the summer and fall of 1940. The fabric was furnished by the Division of Marketing and Marketing Agreements of the United States Department of Agriculture, and was installed by the Mississippi State Highway Department.

The fabric used was of the following types: S1-54, S1-40, S2-54, and S2-40, the latter figure in each designation being the width of the fabric in inches. Approximately equal amounts of each type of fabric were used, 15,948 square yards being used in the eight test sections. Both types of cotton fabric have relatively large mesh openings, type S1 having 6 openings per inch and type S2 having 4. Table 1 shows the amount of each type of fabric and use on each test section.

The fabric was delivered in rolls. On some areas of the test sections the fabric was run horizontally; on others it was run from top to toe of the slope, or "vertically."

Descriptions of each test section and the results obtained follow:

Section A.—The soil was sand-clay, clay, and marl. The areas were first treated by spreading a mixture of bermuda grass roots and topsoil to a depth of approximately 2 inches. The grass mulch was spread uniformly

with shovels and rakes. The fabric was then applied and pegged down with galvanized wire staples. After passing through one growing season there appeared to be little difference in the amount of growth on the covered and uncovered areas. The fabric-covered slope was greener and the ground somewhat more moist than was the uncovered slope. The fabric in the ditch bottom appeared definitely to have retarded erosion.

Figure 1 shows the appearance of portions of Section A soon after the fabric was placed and the appearance of the same portions after one growing season.

TABLE 1.—Amount of each type of cotton fabric and use on each test section

Section No.	Project No.	Amount of cotton fabric, type—		Used on—
		S1	S2	
A.....	PWS 72-C.....	Sq. yd. 1,305	Sq. yd. 750	3:1 to 6:1 slopes; ditch bottom.
B.....	PWS 123.....	1,500	2,250	2:1 backslopes.
C.....	PWS 82-C.....	749	999	2½:1 and 4:1 backslopes; ditch bottom.
D.....	NRH 74-G.....	1,361	109	2½:1 backslopes; ditch bottom.
E.....	PWA 254-A.....	1,200	1,215	3:1 backslope; ditch bottom.
F.....	FAP 173-A.....	1,100	1,105	1½:1 and 2:1 backslopes.
G.....	NRS 236.....	555	750	2:1 backslopes.
H.....	FAP 45.....	500	500	2:1 backslopes.
Total..	.....	8,270	7,678	

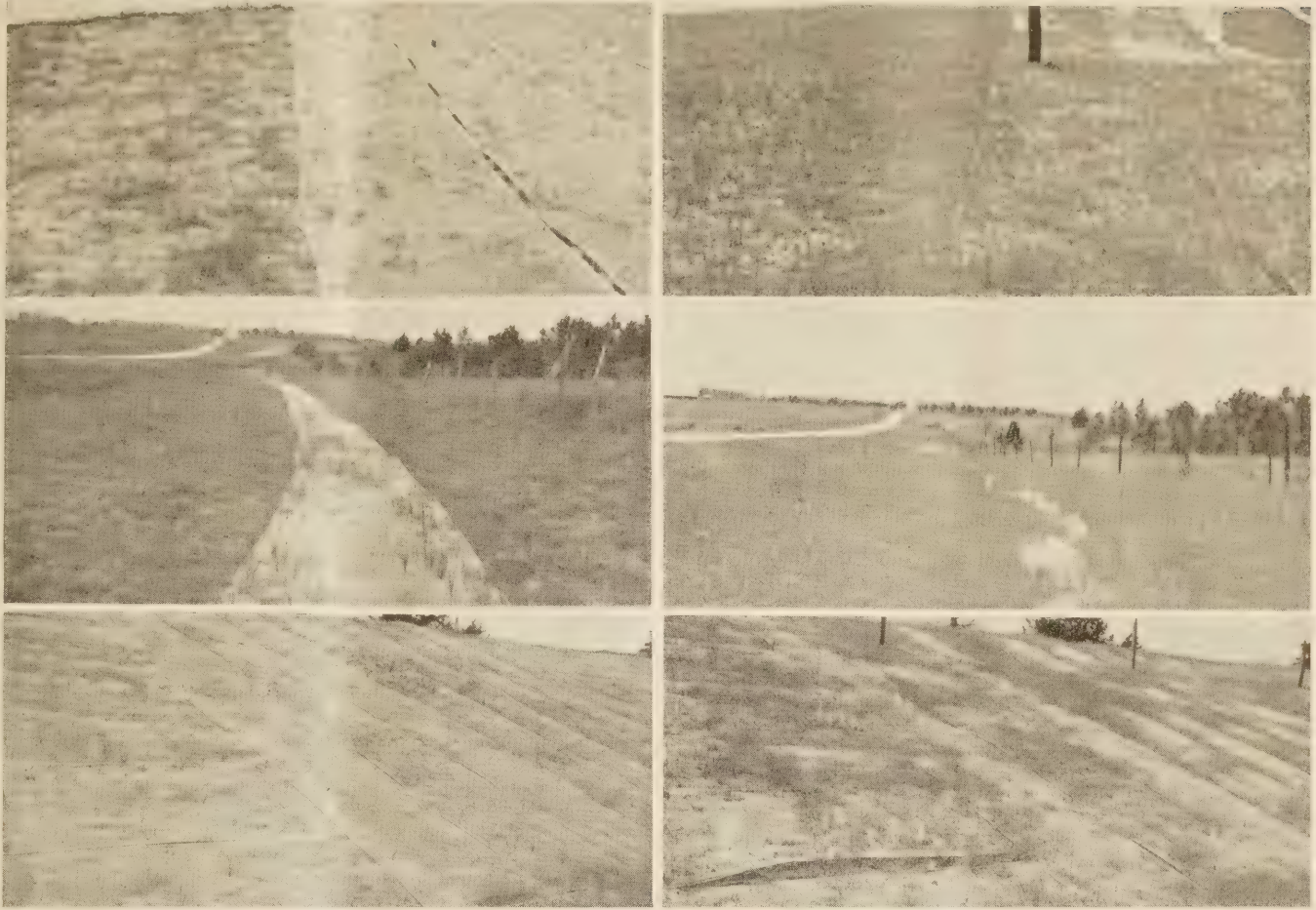


FIGURE 1.—LEFT, APPEARANCE OF PORTIONS OF SECTION A SOON AFTER FABRIC WAS PLACED; RIGHT, APPEARANCE OF THE SAME RESPECTIVE PORTIONS OF SECTION A AFTER ONE GROWING SEASON.

*Section B.*—The soil was sand, sand-clay, and clay. The backslopes were treated the same as the above section; and in addition commercial 4-8-4 fertilizer was mixed into the topsoil at the rate of 750 pounds per acre, and bermuda and carpet grass seed were planted. Part of the treated area was covered with cotton fabric pegged down with wooden stakes, and part was covered with straw mulch. After one growing season the fabric appeared to have helped the growth by shading and retarding erosion. However, the straw mulch appeared to have given better results more cheaply.

*Section C.*—The soil was topsoil, clay, sand-clay, and rock. The 2½:1 slopes and the ditch bottom were treated with topsoil and grass roots the same as on Section A. The 4:1 slope, consisting of clay and sand-clay soil, received no treatment except the cotton fabric covering. The fabric was pegged down with galvanized wire staples. After one growing season, the fabric in the ditch bottom appeared definitely to have retarded erosion and aided the growth. The fabric on the 2½:1 slopes helped by shading the growth. No difference was noted in the amount of erosion on the covered and uncovered portions. On the 4:1 slope, where no grass or topsoil was provided, the fabric appeared definitely to have retarded erosion and promoted native growth.

*Section D.*—The soil was a mixture of sand and clay. The areas were treated with topsoil and grass roots the same as on Section A, and the fabric similarly pegged

with galvanized wire staples. At the time of inspection after one growing season, a large portion of the fabric on the slopes had disappeared. No appreciable difference could be noted in the portion of the slopes that was originally covered with fabric and the portion that was left uncovered. The fabric in the ditch bottom appeared to have helped retard erosion.

Figure 2 shows the appearance of portions of Sections B, C, and D after the fabric was placed, and the appearance of the same sections after one growing season.

*Section E.*—The soil was a sandy clay. A short time prior to placing the fabric the slopes were plowed, fertilized, seeded with grass, and sprigged. The grass had attained a fair growth prior to applying the fabric. A short time after the fabric was placed, two-thirds of it was destroyed by fire. No appreciable difference was noted between areas where the bank remained covered and where it was uncovered. Some erosion was noted between the grass rows beneath the fabric that remained in place. The fabric helped to retard erosion in the ditch bottom.

*Section F.*—The soil was limestone, hard-pan, sand-clay, and clay. The fabric was placed on backslopes that had been strip-sodded about 2 years previously. Some fabric was used on backslopes without sod. The fabric was pegged down with wooden stakes. Nine months after installation an inspection revealed that on the upper part of the north bank growth had benefited



FIGURE 2.—LEFT, APPEARANCE OF PORTIONS OF SECTIONS B (TOP), C (MIDDLE), AND D (BOTTOM) DURING OR SOON AFTER PLACING OF FABRIC; RIGHT, APPEARANCE OF THE SAME RESPECTIVE PORTIONS AFTER ONE GROWING SEASON.

by the shade afforded by the fabric. No benefit was noted on the limestone and clay soil at the bottom of the north bank. On the south bank the growth under the fabric appeared to have benefited by the shade, it being greener than the uncovered portions. No other difference between the covered and uncovered portions was noted.

*Section G.*—The slopes had been covered with 4 inches of good topsoil containing bermuda and other native grasses. The grass had attained a fair growth before the fabric was placed. However, the results could not be compared with those for other sections because on two of the banks the fabric had been covered with straw mulch and on the third bank the fabric had been removed and the bank covered with straw mulch.

*Section H.*—The soil in the slopes was sand-clay. All of the fabric was placed on sodded slopes where the grass was already well established. No results could be obtained because all of the fabric was torn loose by cattle and hogs a short time after it was placed.

The results obtained on these eight experimental sections indicate that:

1. There is no apparent advantage in using cotton fabric on 3:1 or flatter slopes that have been treated

with a mixture of grass roots and topsoil.

2. Use of cotton fabric has some advantage in shading growth and retarding erosion on 2:1 and 1½:1 slopes that have been treated with a mixture of grass roots and topsoil, or sprigged, or strip sodded. However, straw mulch serves better and is more economical wherever available.

3. Cotton fabric aids natural growth and retards erosion on 4:1 slopes of sandy clay and clay soils that have received no other treatment. However, straw mulch serves better wherever available.

4. There is definite advantage in using fabric in roadside ditch bottoms in promoting growth and retarding erosion. The fabric should be pegged at short intervals across its width to hold it close to the ground.

5. The S2-54 fabric is more effective than the other types. The open mesh of the S2 fabric allows vegetation to grow through it, and the wider material is more economical to install per square yard.

6. For greatest effectiveness, the fabric should be placed up and down on slopes and parallel to the flow line in ditches.

7. Galvanized wire staples are more effective than wooden stakes in pegging the fabric.



(Continued from page 88)

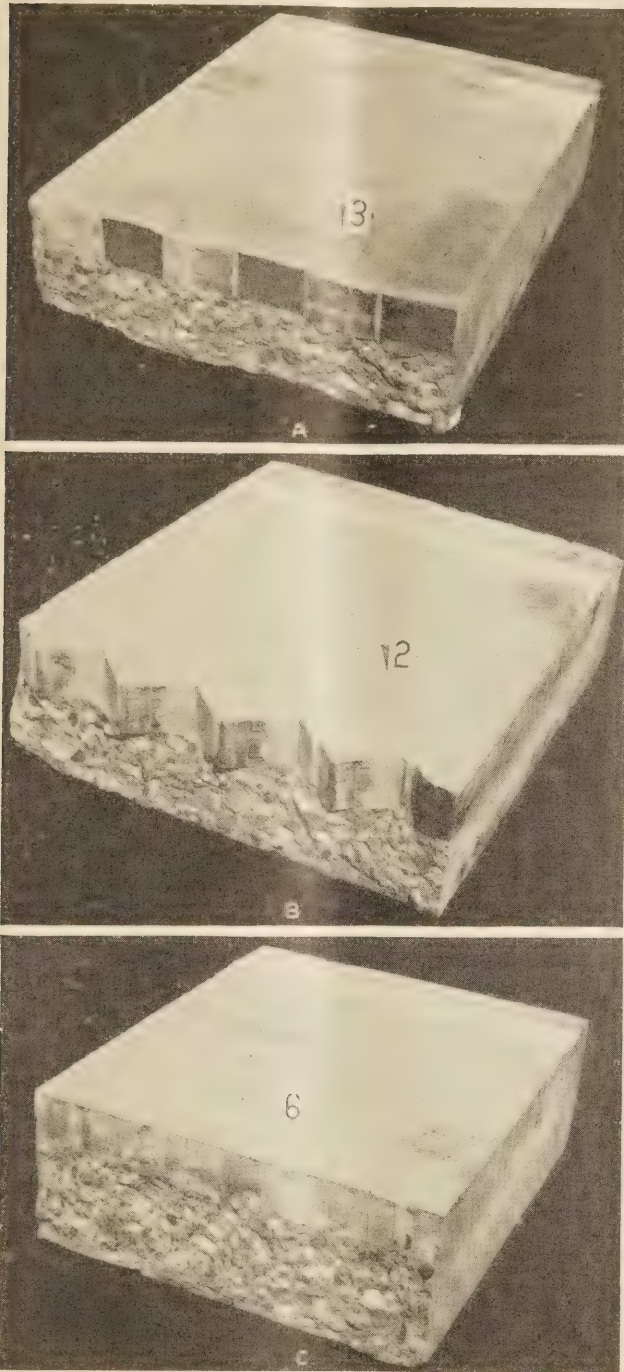


FIGURE 4.—MONOLITHIC BRICK SLABS TESTED WITH BRICK SURFACE IN TENSION. BRICK COURSES LAID (A) PERPENDICULAR, (B) AT AN ANGLE OF 45°, AND (C) PARALLEL TO THE AXIS OF THE LOADING KNIFE BLADE. NOTE FAILURE BETWEEN BRICK AND MORTAR IN (C).

perpendicular to the axis of the loading knife edge exhibited strengths equal to about 90 percent of the strength of plain concrete slabs of equal thickness, while the strength of the slabs in which the brick were set at a 45° angle averaged 67 percent of the strength of plain concrete slabs. In these tests the concrete used in the monolithic brick slabs was of the same quality (6 sacks of cement per cubic yard) as that used in the plain concrete slabs.

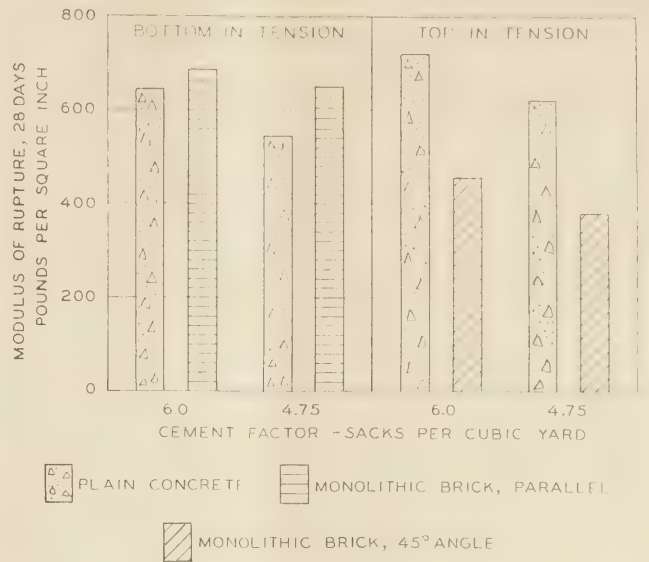


FIGURE 5.—RESULTS OF FLEXURE TESTS ON MONOLITHIC BRICK PAVEMENT SLABS, SERIES B, AT AGE OF 28 DAYS.

Strength results for series B are shown in table 7 and in figure 5. In this series all specimens were 8 inches thick. However, two different classes of concrete were used; one the pavement mix (6 sacks of cement per cubic yard), the other the base mix (4 3/4 sacks of cement per cubic yard). Strength results for slabs tested with the bottom, as cast, in tension are shown in the left panel of figure 5. It will be observed that, for both the pavement and base concretes, the monolithic brick slabs gave higher strengths than did the plain specimens, the average differential in strength being 13 percent. Referring to the results for series A as given in table 6, it will be observed that the differential in favor of the 8-inch monolithic brick slabs, tested with the bottom in tension was also 13 percent.

TABLE 7.—Summary of results of flexure tests on monolithic brick pavement slabs, series B<sup>1</sup>

Structure	Cement factor	Surface in tension	Direction of knife edge of applied load <sup>2</sup>	Flexural strength <sup>3</sup>	Strength ratio <sup>4</sup>
				Lb. per sq. in.	Percent
Concrete	6.0	Bottom		646	100
Monolithic brick	6.0	do	Parallel	686	106
Concrete	4.75	do		545	100
Monolithic brick	4.75	do	Parallel	650	119
Concrete	6.0	Top		721	100
Monolithic brick	6.0	do	45°	454	63
Concrete	4.75	do		622	100
Monolithic brick	4.75	do	45°	379	61

<sup>1</sup> Specimens 26 inches wide by 60 inches long by 8 inches thick (approximately); tested with center loading on a 54-inch span after 28 days of moist curing.

<sup>2</sup> With respect to the line of continuous brick courses.

<sup>3</sup> Flexural strength computed with neutral axis assumed at the center of the slab; each result the average of three tests.

<sup>4</sup> Plain concrete slabs were taken as standard.

The right panel in figure 5 gives results for slabs tested with the top surface, as cast, in tension, the courses of brick being placed at an angle of 45° to the axis of the loading knife edge. The effect of having the brick in tension was to lower the strength to 62 percent of that of plain concrete. This reduction in strength is also in close agreement with the results obtained in

series A where the strength ratio was 60 percent for 8-inch monolithic brick slabs tested with the brick surface in tension and the brick courses at an angle of 45°.

The data plotted in figure 5 are of interest in giving a comparison of monolithic brick slabs using concrete of base mix proportions with plain concrete slabs of pavement proportions. As will be noted by comparing the first and fourth bars in the left panel of figure 5, the monolithic brick slabs using base mix proportions gave almost identical results with those obtained for the plain concrete slabs of pavement proportions. This, of course, was with the brick surface in compression. However, where the slabs were tested with the brick surface in tension, the monolithic brick slabs using base concrete were very much lower in strength.

Summarizing the data for series B, the results check very closely with those found in series A, where comparable tests were made. In addition, it was found that monolithic brick slabs made with base concrete were equal in strength to vibrated slabs of pavement concrete, when tested with the bottom in tension.

#### SUMMARY

The wheel load of a vehicle on a rigid pavement produces tensile stress in the bottom of the slab when the load is at the interior of the slab at some distance from an edge, or at the edge of the slab at some distance from a corner. Therefore, with respect to resistance to loads at the interior and edges of monolithic brick pavements, the most significant results of these tests were the strengths obtained with the slabs tested with the bottoms in tension.

On the other hand, a concentrated load applied at the corner of a rigid pavement produces tensile stress in the top of the slab and, if rupture occurs it will take place along a line that is roughly at an angle of 45° with the axis of the slab. Thus, if a monolithic brick pavement fails under a corner load, the slab will be ruptured along a line that is at an angle of approximately 45° with the direction of the courses of brick. The test slabs with the brick placed at an angle of 45° with the direction of the applied load were built to simulate this corner condition. When tested with the brick surface in tension they ruptured in the same direction, with respect to the brick courses, as the corner of a monolithic brick pavement would be expected to rupture. Therefore, of the results obtained with the

brick slabs tested with the tops in tension, the most significant, with respect to their application in pavement design, are the strengths of the slabs with the brick placed at an angle of 45°.

With the above discussion in mind, the results of these tests may be summarized as follows, all comparisons being made on the basis of slabs of equal thickness:

1. Monolithic brick slabs, when tested with the concrete in tension, developed flexural strengths ranging from 106 to 119 percent of the strengths of comparable slabs of plain concrete.

2. Monolithic brick slabs of base concrete and plain concrete slabs of pavement concrete, when tested with the concrete in tension, developed approximately the same strengths.

3. In monolithic brick slabs tested with the concrete in tension, the bond between the brick surface and the concrete base apparently was not affected by 100 applications of a load equal to 50 percent of the ultimate load.

4. Monolithic brick slabs with the brick at an angle of 45° with the axis of the slab and tested with the brick surface in tension developed flexural strengths ranging from 60 to 76 percent of the strengths of comparable slabs of plain concrete.

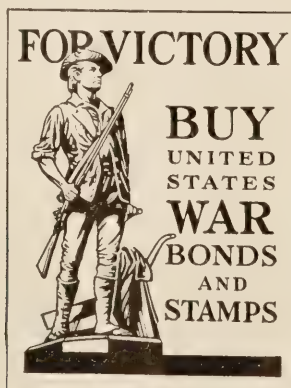
5. Monolithic brick slabs of base concrete, with the brick at an angle of 45° with the axis of the slab and tested with the brick surface in tension, developed about 53 percent of the strength of plain concrete slabs of pavement concrete.

#### HIGHWAY RESEARCH BOARD MEETS IN ST. LOUIS, DECEMBER 2-4, 1942

The Twenty-second Annual Meeting of the Highway Research Board will be held December 2-4, 1942, at the Hotel Statler in St. Louis, Mo.

The customary sessions of the board for the discussion of topics relating to highway finance, economics, design, materials, construction, maintenance, traffic, and soils investigation will be held.

It is expected that time and travel facilities will be greatly conserved by holding this meeting contiguous to that of the American Association of State Highway Officials which will be held in St. Louis the following week.



# MOTOR-FUEL CONSUMPTION-1941

TABLE G-2, 1941  
ISSUED JUNE 1942

COMPILED FOR CALENDAR YEAR FROM REPORTS OF STATE AUTHORITIES 1/

STATE	TAX RATE PER GALLON ON DECEMBER 31	GROSS AMOUNT REPORTED 2/	AMOUNT EXEMPTED FROM PAYMENT OF TAX 2/	GROSS AMOUNT ASSESSED FOR TAXATION	AMOUNT SUBJECT TO REFUND OF ENTIRE TAX	NET AMOUNT TAXED				AMOUNT TAXED AT PREVAILING RATE DURING 1940	INCREASE DURING 1941		STATE
						TOTAL	AT PREVAILING RATE	AT OTHER RATES			AMOUNT	PER-CENT-AGE	
								RATE PER GALLON	AMOUNT				
	CENTS	GALLONS	GALLONS	GALLONS	GALLONS	GALLONS	GALLONS	CENTS	GALLONS	GALLONS	GALLONS		
ALABAMA	6	310,603	-	310,603	-	310,603	310,603	-	-	259,915	50,688	19.5	ALABAMA
ARIZONA	5	130,954	8,060	122,894	12,883	110,011	110,011	-	-	95,707	14,304	14.9	ARIZONA
ARKANSAS	6.5	223,634	8,872	214,762	-	214,762	190,095	(4/)	24,667	162,328	27,767	17.1	ARKANSAS
CALIFORNIA	3	2,272,732	119,055	2,153,677	175,130	1,978,547	1,978,547	-	-	1,758,326	220,221	12.5	CALIFORNIA
COLORADO	4	270,287	14,279	256,008	34,489	221,519	221,519	-	-	206,742	14,777	7.1	COLORADO
CONNECTICUT	3	422,586	14,302	408,284	9,735	398,549	398,549	-	-	304,594	33,955	9.3	CONNECTICUT
DELAWARE	4	69,122	1,837	67,285	4,197	63,088	63,088	-	-	58,397	4,691	8.0	DELAWARE
FLORIDA	7	475,905	49,446	426,459	-	426,459	426,459	-	-	388,717	37,742	9.7	FLORIDA
GEORGIA	6	449,335	22,277	427,058	-	427,058	427,058	-	-	386,847	40,211	10.4	GEORGIA
IDAHO	5.1	117,242	4,124	113,118	-	113,118	102,255	(5/)	10,863	94,565	7,690	8.1	IDAHO
ILLINOIS	3	1,670,862	-	1,670,862	135,285	1,535,577	1,535,577	-	-	1,410,967	124,610	8.8	ILLINOIS
INDIANA	4	799,091	7,484	791,607	70,074	721,533	721,533	-	-	641,164	80,369	12.5	INDIANA
IOWA	3	612,211	-	612,211	93,072	519,140	519,140	-	-	485,095	34,045	6.8	IOWA
KANSAS	3	541,494	162,862	378,632	-	378,632	378,632	-	-	346,720	31,912	9.2	KANSAS
KENTUCKY	5	352,743	15,527	337,216	-	337,216	337,216	-	-	295,205	42,011	14.2	KENTUCKY
LOUISIANA	4	343,599	29,263	314,336	6	314,330	304,900	2	9,430	264,057	40,843	15.5	LOUISIANA
MAINE	7	173,874	1,461	172,413	-	172,413	163,942	1	8,471	149,130	14,812	9.9	MAINE
MARYLAND	4	365,530	10,006	355,524	22,626	332,898	329,903	3	2,995	286,636	43,267	15.1	MARYLAND
MASSACHUSETTS	3	801,957	9,609	792,348	34,277	758,071	758,071	-	-	713,358	44,713	6.3	MASSACHUSETTS
MICHIGAN	3	1,392,274	128,012	1,264,262	64,764	1,199,498	1,198,472	1.5	2/ 1,026	1,082,006	116,466	10.8	MICHIGAN
MINNESOTA	10/ 4	625,173	33,640	591,533	-	591,533	520,007	11/	71,526	497,556	23,451	4.5	MINNESOTA
MISSISSIPPI	6	254,877	14,897	239,980	-	239,980	232,458	1	12/ 7,522	197,570	34,888	17.7	MISSISSIPPI
MISSOURI	2	776,432	-	776,432	38,710	737,722	737,722	-	-	658,321	79,401	12.1	MISSOURI
MONTANA	5	148,262	6,717	141,545	28,296	113,249	113,249	-	-	104,451	8,798	8.4	MONTANA
NEBRASKA	5	259,765	11,255	248,510	21	248,489	248,339	1	13/ 150	233,438	14,901	6.4	NEBRASKA
NEVADA	4	49,018	2,909	46,109	3,108	43,001	40,807	5	14/ 2,194	35,717	5,090	14.3	NEVADA
NEW HAMPSHIRE	4	102,395	896	101,499	5,055	96,444	96,444	-	-	90,822	5,622	6.2	NEW HAMPSHIRE
NEW JERSEY	3	1,066,334	42,230	964,104	85,920	878,184	878,184	-	-	822,426	55,758	6.8	NEW JERSEY
NEW MEXICO	5	122,275	6,986	115,289	12,221	103,068	103,068	-	-	96,106	8,872	9.4	NEW MEXICO
NEW YORK	4	2,058,071	90,529	1,967,542	69,061	1,898,481	1,898,481	-	-	1,836,977	61,504	3.2	NEW YORK
NORTH CAROLINA	6	526,341	-	526,341	-	526,341	511,064	1	12/ 15,277	440,548	70,516	16.0	NORTH CAROLINA
NORTH DAKOTA	4	160,815	70,770	90,045	-	90,045	90,045	-	-	85,780	4,265	5.0	NORTH DAKOTA
OHIO 15/	4	1,638,913	90,737	1,548,176	18,675	1,529,501	1,446,626	1/	82,875	1,320,895	125,741	9.5	OHIO
OKLAHOMA	16/ 5.5	473,597	78,555	395,042	-	395,042	395,042	17/	395,042	378,275	16,767	4.4	OKLAHOMA
OREGON	5	306,484	9,348	297,136	31,167	265,969	264,132	1	18/ 1,837	228,396	35,736	15.6	OREGON
PENNSYLVANIA	4	1,704,947	11,201	1,693,746	-	1,693,746	1,693,746	-	-	1,575,078	118,668	7.5	PENNSYLVANIA
RHODE ISLAND	3	147,280	3,605	143,675	1,445	142,230	142,230	-	-	131,558	10,672	8.1	RHODE ISLAND
SOUTH CAROLINA	6	275,514	-	275,514	9,416	266,098	266,098	-	-	229,398	36,700	16.0	SOUTH CAROLINA
SOUTH DAKOTA	4	156,036	2,681	153,355	33,804	119,551	119,551	-	-	113,659	5,892	5.2	SOUTH DAKOTA
TENNESSEE	7	386,300	26,316	359,984	4,955	355,029	355,029	-	-	304,042	50,987	16.8	TENNESSEE
TEXAS	4	1,664,602	101,499	1,563,103	221,435	1,341,668	1,341,668	-	-	1,195,694	145,974	12.2	TEXAS
UTAH	4	118,062	4,356	113,706	-	113,706	113,706	-	-	102,425	11,281	11.0	UTAH
VERMONT	4	75,189	944	74,245	-	74,245	74,245	-	-	69,926	4,319	6.2	VERMONT
VIRGINIA	5	503,281	-	503,281	31,484	471,797	471,797	3	19/ 778	392,396	79,401	20.0	VIRGINIA
WASHINGTON	5	450,478	28,855	421,623	28,753	392,870	392,870	-	-	347,872	45,000	13.0	WASHINGTON
WEST VIRGINIA	5	242,157	-	242,157	8,227	233,930	233,930	-	-	214,983	18,957	8.8	WEST VIRGINIA
WISCONSIN	4	695,760	18,844	676,916	42,334	634,582	634,582	-	-	529,733	104,849	16.7	WISCONSIN
WYOMING	4	79,804	2,531	77,273	-	77,273	77,273	-	-	68,999	8,274	12.0	WYOMING
DISTRICT OF COLUMBIA	20/ 2	191,920	9,758	182,162	3,578	178,584	178,584	-	-	160,954	17,630	11.0	DISTRICT OF COLUMBIA
<b>TOTAL</b>	<b>21/ 3.99</b>	<b>26,937,361</b>	<b>1,276,535</b>	<b>25,660,826</b>	<b>1,405,829</b>	<b>24,254,997</b>	<b>24,086,922</b>	<b>-</b>	<b>168,075</b>	<b>21,913,441</b>	<b>2,734,481</b>	<b>9.9</b>	<b>TOTAL</b>

1/ AN ANALYSIS OF MOTOR-FUEL USAGE WILL BE GIVEN IN TABLE G-21, TO BE PUBLISHED LATER.  
 2/ EXPORT SALES AND OTHER AMOUNTS NOT REPRESENTING CONSUMPTION IN STATE HAVE BEEN ELIMINATED AS FAR AS POSSIBLE. IN CASES WHERE STATES FAILED TO REPORT AMOUNTS EXEMPTED FROM TAXATION, THE GROSS AMOUNT TAXED IS SHOWN IN THIS COLUMN.  
 3/ INCLUDES ALLOWANCES FOR EVAPORATION AND OTHER LOSSES, FEDERAL USE, OTHER PUBLIC USE, AND NONHIGHWAY USE, WHERE INITIAL EXEMPTIONS RATHER THAN REFUNDS ARE MADE.  
 4/ WITHIN 300 FEET OF BORDER, TAX IS REDUCED TO THAT OF ADJACENT STATE. GALLONS TAXED AT 2 CENTS, 6,731,000; AT 4 CENTS, 10,019,000; AT 5.5 CENTS, 7,917,000.  
 5/ AVIATION FUEL TAXED AT 2.5 CENTS, 607,000 GALLONS; MOTOR FUEL TAXED AT 0.1 CENT (5 CENTS REFUNDED ON NONHIGHWAY USE), 10,246,000 GALLONS.  
 6/ REPRESENTS EVAPORATION OR LOSS ALLOWANCE UNDER 5-CENT TAX NOT ALLOWED UNDER ADDITIONAL 2-CENT TAX, WHICH IS ADMINISTERED UNDER A SEPARATE LAW.  
 7/ THREE CENTS PER GALLON REFUNDED ON NONHIGHWAY USES.  
 8/ ONE CENT PER GALLON REFUNDED ON MOTOR FUEL USED IN VEHICLES LICENSED TO OPERATE EXCLUSIVELY IN CITIES.  
 9/ ONE AND ONE-HALF CENTS PER GALLON REFUNDED ON MOTOR FUEL USED IN INTRASTATE AVIATION.  
 10/ RATE CHANGED FROM 3 CENTS TO 4 CENTS MAY 1.  
 11/ TAXED AT 3 CENTS, 142,044,000 GALLONS; AT 4 CENTS, 377,963,000 GALLONS.  
 12/ FIVE CENTS PER GALLON REFUNDED ON NONHIGHWAY USES.  
 13/ FOUR CENTS PER GALLON REFUNDED ON MOTOR FUEL USED IN FLYING INSTRUCTION.  
 14/ DIESEL FUEL AND BUTANE.  
 15/ AMOUNTS GIVEN DO NOT INCLUDE 63,317,000 GALLONS OF LIQUID FUEL (KEROSENE, FUEL OIL, ETC.) TAXED AT 1 CENT PER GALLON BUT NOT SUBJECT TO THE 5-CENT TAX ON MOTOR-VEHICLE FUEL.  
 16/ RATE CHANGED FROM 4 CENTS TO 5.5 CENTS JUNE 1.  
 17/ TAXED AT 4 CENTS, 163,980,000 GALLONS; AT 5.5 CENTS, 231,062,000 GALLONS.  
 18/ FOUR CENTS PER GALLON REFUNDED ON MOTOR FUEL USED IN AVIATION.  
 19/ TWO CENTS PER GALLON REFUNDED ON MOTOR FUEL USED IN INTRASTATE AVIATION.  
 20/ RATE CHANGED FROM 2 CENTS TO 3 CENTS JANUARY 1, 1942.  
 21/ WEIGHTED AVERAGE RATE.

# STATE MOTOR-FUEL TAX RECEIPTS - 1941

COMPILED FOR CALENDAR YEAR FROM REPORTS OF STATE AUTHORITIES

TABLE C-1, 1941  
ISSUED JUNE 1942

STATE	TAX RATE PER GALLON ON DECEMBER 31	RECEIPTS FROM TAXATION OF MOTOR FUEL					OTHER RECEIPTS IN CONNECTION WITH MOTOR-FUEL TAX 2/					NET TOTAL RECEIPTS	LESS TAX ON AVIATION GASOLINE	ADJUSTED NET TOTAL RECEIPTS	STATE	
		GROSS TAX COLLECTED	DEDUCTIONS BY DISTRIBUTORS FOR EXPENSES 1/	GROSS RECEIPTS BY STATE	REFUNDS PAID	NET RECEIPTS BY STATE	DISTRIBUTORS AND DEALERS LICENSES	INSPECTION FEES 3/	FINES AND PENALTIES	MISCELLANEOUS RECEIPTS	TOTAL					
	CENTS	1,000 DOLLARS	1,000 DOLLARS	1,000 DOLLARS	1,000 DOLLARS	1,000 DOLLARS	1,000 DOLLARS	1,000 DOLLARS	1,000 DOLLARS	1,000 DOLLARS	1,000 DOLLARS	1,000 DOLLARS	1,000 DOLLARS	1,000 DOLLARS	1,000 DOLLARS	
ALABAMA	6	18,323	-	18,323	-	18,323	-	78	-	*	78	18,401	-	18,401	ALABAMA	
ARIZONA	5	6,148	-	6,148	787	5,361	-	-	1	-	5,362	-	5,362	ARIZONA		
ARKANSAS	6.5	13,100	-	13,100	-	13,100	-	119	-	-	13,219	-	13,219	ARKANSAS		
CALIFORNIA	3	63,330	-	63,330	5,254	58,076	17	-	-	1	58,094	-	58,094	CALIFORNIA		
COLORADO	4	10,195	-	10,195	1,362	8,833	-	-	-	-	8,833	-	8,833	COLORADO		
CONNECTICUT	3	12,180	122	12,058	300	11,758	45	-	1	-	11,804	-	11,804	CONNECTICUT		
DELAWARE	4	2,665	-	2,665	164	2,501	3	-	-	-	2,504	-	2,504	DELAWARE		
FLORIDA	7	29,832	-	29,832	-	29,832	41	524	-	-	30,397	-	30,397	FLORIDA		
GEORGIA	6	26,211	262	26,049	-	26,049	37	-	-	-	26,086	-	26,086	GEORGIA		
IDAHO	5.1	5,722	-	5,722	491	5,231	1	-	-	1	5,232	-	5,232	IDAHO		
ILLINOIS	3	49,656	993	48,663	3,901	44,762	-	498	1	-	45,261	-	45,261	ILLINOIS		
INDIANA	4	31,270	-	31,270	2,752	28,518	*	700	24	2	29,244	-	29,244	INDIANA		
IOWA	3	18,199	-	18,199	2,861	15,338	62	-	-	-	15,400	-	15,400	IOWA		
KANSAS	3	11,234	-	11,234	-	11,234	15	118	-	22	11,389	-	11,389	KANSAS		
KENTUCKY	5	16,338	163	16,175	-	16,175	-	-	5	-	16,180	-	16,180	KENTUCKY		
LOUISIANA	7	21,425	-	21,425	-	21,425	-	107	30	-	21,562	-	21,562	LOUISIANA		
MAINE	4	6,918	-	6,918	261	6,657	-	-	*	*	6,657	-	6,657	MAINE		
MARYLAND	4	14,000	-	14,000	930	13,070	-	-	-	-	13,070	-	13,070	MARYLAND		
MASSACHUSETTS	3	23,576	-	23,576	1,088	22,488	48	-	-	-	22,536	-	22,536	MASSACHUSETTS		
MICHIGAN	3	37,586	-	37,586	2,001	35,585	5	-	2	-	35,592	88	35,504	MICHIGAN		
MINNESOTA	4	21,570	-	21,570	2,700	18,870	*	182	-	185	19,055	-	19,055	MINNESOTA		
MISSISSIPPI 5/	6	14,126	-	14,126	373	13,753	-	-	-	-	13,753	-	13,753	MISSISSIPPI 5/		
MISSOURI	2	15,252	-	15,252	975	14,277	-	165	9	-	14,451	-	14,451	MISSOURI		
MONTANA	5	6,878	-	6,878	1,411	5,467	-	4	-	-	5,471	-	5,471	MONTANA		
NEBRASKA	5	12,711	94	12,617	349	12,268	11	128	-	34	12,439	84	12,357	NEBRASKA		
NEVADA	4	1,842	34	1,808	125	1,683	*	24	1	-	1,708	-	1,708	NEVADA		
NEW HAMPSHIRE	4	3,913	-	3,913	198	3,715	-	-	1	-	3,716	-	3,716	NEW HAMPSHIRE		
NEW JERSEY	3	28,814	-	28,814	2,599	26,221	69	-	2	14	26,306	-	26,306	NEW JERSEY		
NEW MEXICO	5	5,695	-	5,695	619	5,076	26	-	-	-	5,102	-	5,102	NEW MEXICO		
NEW YORK	4	78,041	780	77,261	2,805	74,456	62	-	*	62	74,520	-	74,520	NEW YORK		
NORTH CAROLINA	6	31,417	-	31,417	768	30,649	-	1,271	-	11	31,931	-	31,931	NORTH CAROLINA		
NORTH DAKOTA	4	3,569	54	3,515	-	3,515	1	83	-	26	3,625	-	3,625	NORTH DAKOTA		
OHIO	4	61,274	-	61,274	3,161	58,113	-	-	-	-	58,113	-	58,113	OHIO		
OKLAHOMA	5.5	19,141	-	19,141	2	18,661	-	315	-	-	18,976	-	18,976	OKLAHOMA		
OREGON	5	14,702	478	14,224	1,673	13,029	-	-	*	-	13,029	-	13,011	OREGON		
PENNSYLVANIA	4	65,627	760	64,867	-	64,867	-	-	5	-	64,872	-	64,872	PENNSYLVANIA		
RHODE ISLAND	3	4,627	-	4,627	355	4,272	3	-	-	-	4,275	-	4,275	RHODE ISLAND		
SOUTH CAROLINA	6	16,356	-	16,356	539	15,817	-	330	-	330	16,147	87	16,060	SOUTH CAROLINA		
SOUTH DAKOTA	4	6,112	215	5,897	1,338	4,559	-	79	-	79	4,638	21	4,617	SOUTH DAKOTA		
TENNESSEE	7	24,505	-	24,505	315	24,190	-	1,386	-	69	25,645	246	25,399	TENNESSEE		
TEXAS	4	62,972	633	62,339	9,592	52,747	-	-	-	21	52,768	-	52,768	TEXAS		
UTAH	4	4,900	68	4,832	-	4,832	1	-	1	*	4,834	64	4,370	UTAH		
VERMONT	4	2,930	-	2,930	-	2,930	-	-	*	-	2,930	-	2,930	VERMONT		
VIRGINIA	5	24,501	-	24,501	1,585	22,916	1	-	2	*	22,919	23	22,896	VIRGINIA		
WASHINGTON	5	20,844	104	20,740	1,419	19,321	3	-	-	10	19,334	-	19,334	WASHINGTON		
WEST VIRGINIA	5	11,956	-	11,956	403	11,553	6	-	-	-	11,559	-	11,559	WEST VIRGINIA		
WISCONSIN	4	24,432	-	24,432	1,695	22,737	-	189	-	-	22,926	-	22,926	WISCONSIN		
WYOMING	4	3,065	-	3,065	-	3,065	2	-	-	-	3,067	-	3,014	WYOMING		
DISTRICT OF COLUMBIA	8/ 2	3,550	-	3,550	71	3,479	*	-	-	*	3,479	-	3,479	DISTRICT OF COLUMBIA		
TOTAL	2/ 3.99	1,012,990	4,760	1,008,170	57,214	950,956	459	6,300	88	210	7,057	958,013	701	957,312	TOTAL	

1/ THE STATES FOR WHICH AMOUNTS ARE SHOWN MAKE ALLOWANCES TO DISTRIBUTORS FOR EXPENSE OF COLLECTING THE TAX. IN KENTUCKY, SOUTH DAKOTA, UTAH, AND WASHINGTON ALLOWANCES OF 2 1/4, 4, 3, AND 1 PERCENT, RESPECTIVELY, OF THE TAX OTHERWISE DUE ARE MADE IN CONSIDERATION OF BOTH EXPENSE OF COLLECTION AND GALLONAGE LOSSES IN HANDLING. IN THESE STATES THE ALLOWANCES FOR EXPENSES ONLY HAVE BEEN ESTIMATED AS 1, 3 1/2, 1 1/2, AND 1/2 PERCENT, RESPECTIVELY.

2/ STARS INDICATE AMOUNTS LESS THAN \$500.

3/ FEES FOR INSPECTION OF MOTOR-VEHICLE FUEL. WHEREVER POSSIBLE, FEES FOR INSPECTION OF KEROSENE AND OTHER NON-MOTOR-VEHICLE FUELS HAVE BEEN ELIMINATED.

4/ RATE CHANGED FROM 3 CENTS TO 4 CENTS MAY 1.

5/ SPECIAL COUNTY TAXES OF 3 CENTS PER GALLON IN HANCOCK COUNTY AND 2 CENTS PER GALLON IN HARRISON AND JACKSON COUNTIES, AMOUNTING TO \$306,000 IN 1941, ARE IMPOSED FOR SEAWALL PROTECTION AND ARE NOT INCLUDED IN THIS TABLE.

6/ OHIO IMPOSES A 5-CENT TAX ON MOTOR-VEHICLE FUEL AND A 1-CENT TAX ON ALL LIQUID FUELS. THE RECEIPTS FROM THE 1-CENT TAX APPLICABLE TO NON-MOTOR-VEHICLE FUELS (KEROSENE, FUEL OIL, ETC.) WERE \$627,000. THESE RECEIPTS HAVE BEEN ELIMINATED FROM THE TOTAL GIVEN, WHICH REPRESENTS A 4-CENT TAX ON MOTOR-VEHICLE FUEL.

7/ RATE CHANGED FROM 4 CENTS TO 5.5 CENTS JUNE 1.

8/ THIS RATE NO LONGER IN EFFECT; RATE CHANGED FROM 2 CENTS TO 3 CENTS JANUARY 1, 1942.

9/ WEIGHTED AVERAGE RATE.

STATE MOTOR-VEHICLE REGISTRATIONS - 1941

COMPILED FOR CALENDAR YEAR FROM REPORTS OF STATE AUTHORITIES 1/

TABLE MV-3, 1941  
ISSUED JUNE 1942

Table with columns: STATE, TOTAL, PRIVATE AND COMMERCIAL, PASSENGER VEHICLES (AUTOMOBILES, TAXICABS, BUSES, TRUCKS AND TRACTOR TRUCKS), PUBLICLY OWNED (FEDERAL, STATE, COUNTY, MUNICIPAL), TRAILERS AND SEMITRAILERS (PRIVATE AND COMMERCIAL, PUBLICLY OWNED), MOTORCYCLES (PRIVATE AND COMMERCIAL, PUBLICLY OWNED), DEALERS REGISTRATIONS (REGULAR, EXTRA PLATES), 1940 REGISTERED (PRIVATE, COMMERCIAL, TRUCKS), YEAR'S CHANGE IN REGISTRATIONS (INCREASE, PERCENT INCREASE), STATE.

1/ REGISTRATION PERIODS ENDING NOT EARLIER THAN NOVEMBER 30 AND NOT LATER THAN JANUARY 31 ARE CONSIDERED CALENDAR-YEAR PERIODS. IN THOSE STATES WHERE THE REGISTRATION PERIOD IS DEFINITELY MOVED FROM THE CALENDAR YEAR, REGISTRATION FIGURES ARE SHOWN UNDER THE YEAR IN WHICH THE REGISTRATION PERIOD ENDED. WHEREVER POSSIBLE PUBLICLY OWNED VEHICLES AND VEHICLES NOT FOR HIGHWAY USE HAVE BEEN ELIMINATED FROM THIS COLUMN. 2/ WHEREVER POSSIBLE PUBLICLY OWNED VEHICLES AND VEHICLES NOT FOR HIGHWAY USE HAVE BEEN ELIMINATED FROM THIS COLUMN. 3/ THE BUS FIGURES GIVEN REPRESENT COMMERCIAL BUSES IN MOST CASES, ALTHOUGH IN SOME STATES CONTRACT SCHOOL BUSES ARE INCLUDED. 4/ DATA ON FEDERAL VEHICLES OBTAINED THROUGH AGENCY OF PRODUCTION DIVISION, DEPARTMENT OF THE TREASURY. VEHICLES OWNED BY THE MILITARY SERVICES, INCLUDED IN PRIOR YEARS, ARE OMITTED FROM THESE COLUMNS. FEDERAL VEHICLES NOT ASSIGNED TO ANY PARTICULAR STATE WERE LISTED AS "AT LARGE" IN PREVIOUS TABLES. IN THIS TABLE SUCH VEHICLES HAVE BEEN ASSIGNED TO STATES IN ACCORDANCE WITH THE FOLLOWING LIST: ALABAMA, ARIZONA, ARKANSAS, CALIFORNIA, COLORADO, CONNECTICUT, DELAWARE, FLORIDA, GEORGIA, ILLINOIS, IOWA, KANSAS, KENTUCKY, LOUISIANA, MASSACHUSETTS, MICHIGAN, MINNESOTA, MISSISSIPPI, MISSOURI, MONTANA, NEBRASKA, NEVADA, NEW HAMPSHIRE, NEW JERSEY, NEW MEXICO, NEW YORK, NORTH CAROLINA, NORTH DAKOTA, OHIO, OKLAHOMA, OREGON, PENNSYLVANIA, RHODE ISLAND, SOUTH CAROLINA, SOUTH DAKOTA, TENNESSEE, TEXAS, UTAH, VERMONT, VIRGINIA, WASHINGTON, WEST VIRGINIA, WISCONSIN, WYOMING, DIST. OF COL., TOTAL.

STATE MOTOR-VEHICLE RECEIPTS - 1941

COMPILED FOR CALENDAR YEAR FROM REPORTS OF STATE AUTHORITIES

TABLE M-2, 1941  
ISSUED JUNE 1942

Main data table with columns for STATE, REGISTRATION FEES, OTHER VEHICLES, MISCELLANEOUS RECEIPTS, and TOTAL RECEIPTS.

RECEIPTS FOR PERIODS ENDING NOT EARLIER THAN NOVEMBER 30 AND NOT LATER THAN JANUARY 31... SPECIAL TITLE TAXES... MOTOR-VEHICLE REGISTRATION FEES... FEES OF LIGHT TRAILERS AND COMMERCIAL SEMI-TRAILERS ONLY...

STATUS OF FEDERAL-AID HIGHWAY PROJECTS

AS OF JULY 31, 1942

STATE	COMPLETED DURING CURRENT FISCAL YEAR			UNDER CONSTRUCTION			APPROVED FOR CONSTRUCTION			BALANCE OF FUNDS AVAILABLE FROM GRANT-MADE PROJECTS
	Estimated Total Cost	Federal Aid	Miles	Estimated Total Cost	Federal Aid	Miles	Estimated Total Cost	Federal Aid	Miles	
Alabama										
Arizona										
Arkansas										
California	\$281,700	\$165,500	11.2	\$4,261,349	\$2,315,384	115.2	\$198,900	\$99,450	0.3	\$2,565,784
Colorado	79,600	79,600		665,314	552,164	28.2	487,374	301,290	4.8	1,515,952
Connecticut				1,275,742	635,815	59.9	810,614	589,082	16.9	1,172,121
Delaware				4,473,982	3,124,185	42.8	2,604,776	1,898,813	75.5	2,226,698
Florida				3,541,754	2,270,952	155.4	938,023	529,825	30.9	1,954,086
Georgia				2,254,409	1,090,542	27.8	1,185,822	774,873	6.7	1,424,512
Illinois				428,810	210,460	9.0	268,040	134,020	8.4	1,513,348
Indiana				3,419,614	2,017,687	52.7	150,000	112,500	9	2,909,315
Iowa	65,718	45,075	3.3	8,062,680	4,254,197	282.7	3,030,687	1,584,005	107.2	6,189,049
Kansas	193,521	96,760	10.9	1,109,663	612,177	57.5	425,377	227,494	23.1	1,929,251
Kentucky	739,050	377,875	7.7	7,008,025	4,078,892	114.5	2,924,528	1,602,996	10.6	5,291,261
Louisiana	805,800	402,900	15.5	6,425,689	3,129,737	89.7				2,907,857
Maine				1,159,857	798,961	22.4	123,176	27,200	4.6	2,309,213
Maryland	155,480	77,740	2.9	1,789,292	977,337	19.2	78,610	39,305	1.1	1,006,899
Massachusetts				2,253,781	1,102,489	12.0	810,000	573,625	4.3	780,082
Michigan	444,074	221,658	5.5	1,493,894	821,486	8.2	1,170,360	581,593	8.4	3,802,058
Minnesota	28,000	21,000	4	3,526,516	2,343,458	73.7	1,477,100	1,006,775	61.0	2,349,634
Mississippi	934,753	576,391	14.1	8,463,111	4,597,110	37.7	499,190	248,595	9.3	2,644,051
Missouri				2,923,908	1,456,504	157.4	331,300	248,475	5.3	2,077,457
Montana	699,456	349,728	14.3	9,323,372	5,599,469	158.2	2,044,687	1,057,614	21.8	3,263,308
Nebraska	680,704	409,067	18.3	3,076,532	2,018,054	158.3	238,580	158,250	25.7	4,088,704
Nevada	687,804	358,622	57.6	2,260,130	1,143,999	76.6	1,167,560	874,778	76.6	3,644,790
New Hampshire	97,818	85,002	6.4	1,479,326	1,324,000	40.8	544,729	486,949	13.9	413,339
New Jersey				1,682,125	1,131,880	19.4				683,756
New Mexico				85,262	42,631		758,830	551,250	2.2	2,381,651
New York	55,592	38,583	11.0	726,233	566,823	26.9	818,715	671,755	58.5	1,377,719
North Carolina	804,019	514,788	9.3	7,952,443	5,067,685	77.0	429,000	193,500	6.4	5,234,953
North Dakota	868,374	455,347	24.2	1,122,567	599,147	190.0	882,016	443,952	10.0	3,484,755
Ohio	338,180	215,387	41.9	2,922,673	1,757,395	190.0	2,841,800	1,636,882	181.8	3,010,930
Oklahoma	2,757,360	1,755,944	16.2	12,047,192	7,049,032	70.2	3,527,610	1,938,866	27.7	757,003
Oregon				2,717,873	1,521,583	57.2				5,758,946
Pennsylvania	112,243	54,000	10.4	3,755,534	2,128,876	75.7	397,962	318,810	26.2	930,823
Rhode Island	365,287	162,489	4.3	2,491,968	1,527,019	61.0	3,336,620	2,235,739	18.6	2,867,949
South Carolina				1,032,274	662,318	51.1	479,631	239,757	2.6	602,239
South Dakota	577,100	414,170	57.5	3,279,041	2,073,216	71.1	1,183,990	781,625	19.5	901,776
Tennessee	326,308	163,154	1.5	3,775,133	2,491,345	444.0	615,240	365,050	70.4	2,867,481
Texas	1,431,060	715,100	51.6	4,250,786	2,496,980	95.3				3,485,842
Utah	577,845	434,555	14.8	7,197,714	3,399,398	189.4				8,532,202
Vermont	237,535	107,173	2.8	1,182,810	603,211	26.8	1,330,145	651,830	40.9	766,837
Virginia	305,788	153,839	8.8	955,791	608,067	18.0	374,733	476,706	23.2	1,996,743
Washington				2,963,766	1,625,853	42.2	36,936	18,453	1	396,743
West Virginia	40,957	27,150	4.8	2,639,710	1,473,993	30.4	211,457	124,500	3.4	2,757,658
Wisconsin	423,506	208,401	4.4	2,485,653	1,502,080	24.1	884,959	488,979	8.8	1,059,368
Wyoming	1,289,778	794,390	51.9	5,010,079	3,311,467	117.5	352,531	251,350	10.4	3,369,384
District of Columbia	491,888	353,584	27.4	1,358,229	1,040,201	104.7	140,715	115,749	12.1	1,069,972
Hawaii				353,622	206,688	9	1,008,052	593,356	3.6	28,530
Puerto Rico	3,444	3,444		951,269	951,269	111.8	476,174	456,702	4.7	1,206,281
TOTALS	18,452,645	10,693,930	600.8	177,026,119	102,393,305	4,421.4	50,935,127	31,308,034	1,227.6	121,659,890

STATUS OF FEDERAL-AID SECONDARY OR FEEDER ROAD PROJECTS

AS OF JULY 31, 1942

STATE	COMPLETED DURING CURRENT FISCAL YEAR			UNDER CONSTRUCTION			APPROVED FOR CONSTRUCTION			PLANS FOR AID AVAILABLE FOR PRO-GRAMMETS PROJECTS
	Estimated Total Cost	Federal Aid	Miles	Estimated Total Cost	Federal Aid	Miles	Estimated Total Cost	Federal Aid	Miles	
Alabama	\$61,345	\$46,046	2.9	\$562,252	\$300,210	10.9	\$23,031	\$11,050	10.9	\$90,870
Arizona				142,693	103,688	6.7	69,383	49,650	8.0	414,777
Arkansas	4,995			662,760	329,780	32.4	132,656	67,416		160,997
California				793,607	555,121	8.8	18,050	9,900		954,372
Colorado				137,809	78,621	3.2	152,387	35,323	5.0	497,995
Connecticut				275,575	120,602	4.8				194,940
Delaware	43,350	21,675	5.0	8,040	4,020		102,873	37,613	3.9	251,159
Florida	108,248	54,124	.2	188,120	103,530	6.0	604,762	302,331	36.6	399,766
Georgia				1,326,307	775,503	82.8				938,371
Idaho	40,000	20,000	2.2	391,287	259,816	12.2	38,149	23,987	2.8	185,196
Illinois	80,000	40,000	5.3	1,265,384	631,692	63.2	219,000	199,500	3.7	771,146
Indiana				1,269,466	599,282	48.8				927,827
Iowa	109,341	51,500	20.6	543,562	242,570	79.1	32,056	15,050	8.8	585,167
Kansas	26,060	17,170		1,808,040	904,857	97.9	177,337	88,669	23.4	1,039,534
Kentucky	140,794	22,951	4.5	1,041,841	378,281	29.9	35,726	6,400	15.1	319,167
Louisiana				15,460	7,730		674,013	267,134	15.5	533,140
Maine	64,770	32,385	3.8	116,796	58,398	4.1	39,383	19,614	.5	142,227
Maryland				226,410	113,205	1.8				345,564
Massachusetts	241,682	128,841	3.4	462,552	264,842	6.7	219,670	109,835	.2	522,088
Michigan	51,800	25,900		672,898	336,449	29.4	298,260	148,730	19.6	655,791
Minnesota	34,062	17,031	.1	1,031,533	521,505	98.0				560,306
Mississippi	162,104	81,052	12.5	1,504,183	714,265	59.0				388,361
Missouri	39,842	22,609	7.3	560,911	271,114	57.7	186,244	63,193	32.4	1,030,471
Montana	54,955	29,603	10.8	252,900	148,113	23.7	13,569	7,715	4.5	904,396
Nevada				131,727	68,477	13.9				695,483
New Hampshire				88,578	56,842	4.6	107,839	99,444	11.9	144,428
New Jersey	278,480	149,240	6.2	241,995	164,766	3.6				87,696
New Mexico				153,825	100,815	7.6				561,260
New York				282,325	169,211	19.5				299,178
North Carolina	240,526	120,230	17.3	1,431,250	756,263	15.9				1,076,253
North Dakota				394,121	214,458	20.8				638,786
Ohio				7,382	7,382		69,820	20,000	5.0	638,786
Oklahoma				901,610	498,775	14.5	808,050	793,860	42.7	755,172
Oklahoma				120,588	63,680	4.0	236,950	118,475	5.0	1,303,467
Rhode Island	108,767	66,350	3.5	286,601	163,737	16.8	1,215,706	644,924	71.9	860,552
South Carolina	2,207	1,096		232,174	146,052	5.5	30,482	18,000	1.5	335,992
South Carolina	202,900	65,124	.5	78,192	42,846	.8	194,147	97,051	2.5	724,357
South Dakota				24,944	18,355		33,245	16,622	.2	80,038
Tennessee	295,522	147,761	8.4	9,056	433,000	34.8	1,143,430	1,047,500	114.5	394,792
Tennessee				846,000	433,000	34.8	158,042	79,021	4.6	757,118
Texas				360,490	177,895	26.5				749,292
Utah	55,085	27,543	2.5	99,045	73,878	1.5				2,241,944
Vermont	322,312	142,308	7.7	222,515	117,328	8.9				305,370
Virginia				49,604	25,572	.7				53,580
Washington				337,818	135,362	7.8	786,758	409,402	64.8	679,170
West Virginia	175,950	87,975	5.3	115,470	57,122	1.1				1,029
West Virginia	129,993	61,830	.3	1,157,387	528,605	42.4				581,441
Wyoming				484,690	204,982	31.7				214,778
District of Columbia				1,279	1,279		107,026	52,000	.9	111,600
Hawaii				74,942	40,110	3.1	49,687	49,687	.4	285,187
Puerto Rico										216,668
TOTALS	3,076,190	1,485,339	130.3	23,332,199	12,099,662	1,055.1	7,980,731	4,806,451	490.5	27,945,338



# PUBLICATIONS of the PUBLIC ROADS ADMINISTRATION

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Any of the following publications may be purchased from the Superintendent of Documents, Government Printing Office, Washington, D. C. As his office is not connected with the Agency and as the Agency does not sell publications, please send no remittance to the Federal Works Agency.

## ANNUAL REPORTS

- Report of the Chief of the Bureau of Public Roads, 1931. 10 cents.
- Report of the Chief of the Bureau of Public Roads, 1932. 5 cents.
- Report of the Chief of the Bureau of Public Roads, 1933. 5 cents.
- Report of the Chief of the Bureau of Public Roads, 1934. 10 cents.
- Report of the Chief of the Bureau of Public Roads, 1935. 5 cents.
- Report of the Chief of the Bureau of Public Roads, 1936. 10 cents.
- Report of the Chief of the Bureau of Public Roads, 1937. 10 cents.
- Report of the Chief of the Bureau of Public Roads, 1938. 10 cents.
- Report of the Chief of the Bureau of Public Roads, 1939. 10 cents.
- Work of the Public Roads Administration, 1940, 10 cents.
- Work of the Public Roads Administration, 1941, 15 cents.

## HOUSE DOCUMENT NO. 462

- Part 1 . . . Nonuniformity of State Motor-Vehicle Traffic Laws. 15 cents.
- Part 2 . . . Skilled Investigation at the Scene of the Accident Needed to Develop Causes. 10 cents.
- Part 3 . . . Inadequacy of State Motor-Vehicle Accident Reporting. 10 cents.
- Part 4 . . . Official Inspection of Vehicles. 10 cents.
- Part 5 . . . Case Histories of Fatal Highway Accidents. 10 cents.
- Part 6 . . . The Accident-Prone Driver. 10 cents.

## MISCELLANEOUS PUBLICATIONS

- No. 76MP . . . The Results of Physical Tests of Road-Building Rock. 25 cents.
- No. 191MP . . . Roadside Improvement. 10 cents.
- No. 272MP . . . Construction of Private Driveways. 10 cents.
- No. 279MP . . . Bibliography on Highway Lighting. 5 cents.
- Highway Accidents. 10 cents.
- The Taxation of Motor Vehicles in 1932. 35 cents.
- Guides to Traffic Safety. 10 cents.
- An Economic and Statistical Analysis of Highway-Construction Expenditures. 15 cents.
- Highway Bond Calculations. 10 cents.
- Transition Curves for Highways. 60 cents.
- Highways of History. 25 cents.
- Specifications for Construction of Roads and Bridges in National Forests and National Parks. 1 dollar.

## DEPARTMENT BULLETINS

- No. 1279D . . . Rural Highway Mileage, Income, and Expenditures, 1921 and 1922. 15 cents.
- No. 1486D . . . Highway Bridge Location. 15 cents.

## TECHNICAL BULLETINS

- No. 55T . . . Highway Bridge Surveys. 20 cents.
- No. 265T . . . Electrical Equipment on Movable Bridges. 35 cents.

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Single copies of the following publications may be obtained from the Public Roads Administration upon request. They cannot be purchased from the Superintendent of Documents.

## MISCELLANEOUS PUBLICATIONS

- No. 296MP . . . Bibliography on Highway Safety.
- House Document No. 272 . . . Toll Roads and Free Roads.
- Indexes to PUBLIC ROADS, volumes 6-8 and 10-21, inclusive.

## SEPARATE REPRINT FROM THE YEARBOOK

- No. 1036Y . . . Road Work on Farm Outlets Needs Skill and Right Equipment.

## REPORTS IN COOPERATION WITH UNIVERSITY OF ILLINOIS

- No. 303. . . Solutions for Certain Rectangular Slabs Continuous Over Flexible Support.
- No. 304. . . A Distribution Procedure for the Analysis of Slabs Continuous Over Flexible Beams.
- No. 313. . . Tests of Plaster-Model Slabs Subjected to Concentrated Loads.
- No. 314. . . Tests of Reinforced Concrete Slabs Subjected to Concentrated Loads.
- No. 315. . . Moments in Simple Span Bridge Slabs With Stiffened Edges.

## UNIFORM VEHICLE CODE

- Act I.—Uniform Motor Vehicle Administration, Registration, Certificate of Title, and Antitheft Act.
- Act II.—Uniform Motor Vehicle Operators' and Chauffeurs' License Act.
- Act III.—Uniform Motor Vehicle Civil Liability Act.
- Act IV.—Uniform Motor Vehicle Safety Responsibility Act.
- Act V.—Uniform Act Regulating Traffic on Highways.
- Model Traffic Ordinances.

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A complete list of the publications of the Public Roads Administration, classified according to subject and including the more important articles in PUBLIC ROADS, may be obtained upon request addressed to Public Roads Administration, Willard Bldg., Washington, D. C.

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# STATUS OF FEDERAL-AID GRADE CROSSING PROJECTS

AS OF JULY 31, 1942

STATE	COMPLETED DURING CURRENT FISCAL YEAR				UNDER CONSTRUCTION				APPROVED FOR CONSTRUCTION				BALANCE OF FUNDABLE FOR FISCAL YEAR ENDED 6/30/42			
	Estimated Total Cost	Federal Aid	NUMBER		Estimated Total Cost	Federal Aid	NUMBER		Estimated Total Cost	Federal Aid	NUMBER					
			Grade Crossings by Separate Acts Reauthorized	Grade Crossings Projected, Sublicensed or Otherwise			Grade Crossings by Separate Acts Reauthorized	Grade Crossings Projected, Sublicensed or Otherwise			Grade Crossings by Separate Acts Reauthorized	Grade Crossings Projected, Sublicensed or Otherwise				
Alabama	\$19,100	\$19,100	1	1	\$504,525	\$502,903	7	2	\$52,335	\$52,335	2	1	5	\$908,129		
Arizona					132,529	129,838	1	1	14,095	14,095				230,684		
Arkansas	9,364	9,364	3	3	171,586	169,876	1	1	13,954	13,954				653,828		
California					682,453	682,453	3	3	3,775	3,775				2,308,368		
Connecticut					612,609	612,609	5	5	11,111	11,111				741,933		
Delaware					8,652	8,652			231,374	222,740		1	5	532,031		
Florida					191,599	189,867	1	1	508,406	321,785	2			871,496		
Georgia					843,067	840,887	8	6	78,433	78,433			13	179,564		
Idaho					665,593	665,593	3	5	785,985	785,985	2	4	9	1,504,838		
Illinois					185,563	176,891	2	2	4,189	4,189				448,975		
Indiana					957,927	954,981	6	2	384,399	384,399	1	1	29	2,549,863		
Iowa					440,361	440,361	2	1	85,861	85,861			26	1,160,246		
Kansas					1,401,276	1,348,026	10	1	79,765	78,845			21	638,171		
Kentucky					616,568	616,568	9	1	82,621	82,621	1	1	5	1,294,058		
Louisiana					408,223	408,223	4		23,850	23,850	1			396,031		
Maine					338,486	338,486	6		658,006	658,006	5			736,600		
Maryland					253,884	253,884	1							284,591		
Massachusetts					7,150	7,150	1	1	30,175	30,175			6	242,604		
Michigan					84,036	83,820	1		608,552	607,941	3	1	1	1,250,593		
Minnesota									1,623,692	1,623,692	4	4	1	1,29,822		
Mississippi					244,800	244,800	4		943,112	943,112	5	4		1,241,354		
Missouri					13,356	13,356			610,828	610,828	5			641,634		
Montana					5,363	5,352			1,297,892	1,297,892	5	4		1,462,449		
Nebraska					96,908	96,836	1	1	93,828	93,828	1			734,621		
Nevada									410,892	410,892	8			480,668		
New Hampshire									56,484	56,484	2			196,853		
New Jersey									76,060	75,735	2			313,962		
New Mexico									632,648	507,098	2	2		222,538		
New York					71,000	71,000	1	1	71,000	71,000	1	1		519,348		
North Carolina					566,900	566,900	2	1	1,352,338	1,352,338	1	6		3,726,599		
North Dakota					78,355	78,355	1		99,847	99,847	1			872,117		
Ohio					216,790	216,790	3		192,655	192,655	1			1,411,984		
Oklahoma									2,752,253	2,428,331	10			872,117		
Oregon									888,013	886,093	6	3		1,428,559		
Pennsylvania					603,387	603,387	3		476,181	476,181	3			1,435,846		
Rhode Island					1,472	1,462			3,146,650	3,146,650	15			160,571		
South Dakota					9,420	9,420	2	2	2,193	2,193				2,506,663		
Tennessee									47,842	47,842	1	1		275,921		
Texas									515,821	499,871	9		3	975,666		
Utah					138,448	136,041	1	1	1,256,182	1,256,182	6	1	2	830,897		
Vermont									779,830	779,154	6			672,189		
Virginia									42,917	42,917	1	2		2,388,145		
Washington									324,331	294,552	2	1		362,966		
West Virginia									352,754	332,794	3	1		965,312		
Wisconsin									244,558	244,558	5			447,340		
Wyoming									1,041,532	834,123	4	2		324,320		
District of Columbia					172,050	172,050	3	1	381,214	380,470	3			1,671,665		
Hawaii					36,476	36,381			6,210	6,210				428,049		
Puerto Rico									285,934	285,934	2	1		91,161		
TOTALS	2,962,063	2,872,083	23	5	22	31,944,671	29,964,994	200	32	79	7,114,073	6,452,235	30	14	205	45,300,919

