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D. M. BEACH, Editor

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The reports of research published in this magazine are necessarily qualified by the conditions of the tests from which the data are obtained. Whenever it is deemed possible to do so, generalizations are drawn from the results of the tests; and, unless this is done, the conclusions formulated must be considered as specifically pertinent only to described conditions.

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STATE OF IMPROVEMENT OF RURAL ROADS IN RELATION TO TRAFFIC AND DWELLINGS SERVED

BY THE DIVISION OF HIGHWAY TRANSPORT, PUBLIC ROADS ADMINISTRATION

Reported by JOHN T. LYNCH, Highway Engineer-Economist and THOMAS B. DIMMICK, Associate Highway Engineer-Economist

A NIMPORTANT PART of the State-wide highway planning surveys has been the determination of the state of improvement of rural roads in relation to the volume of travel and to the location of rural dwellings. Such a determination is basic in any appraisal of highway service in relation to needs, and in the setting up of long-range construction programs and the making of financial provisions for them.

The summarization of the data has not yet been completed in all States, but the work is sufficiently far advanced to permit the release of preliminary figures that give a reasonably accurate national picture of our highway facilities, showing the extent to which traffic and rural dwellings are served by roads having surfaces of different types. A complete appraisal of the adequacy of highways for traffic needs would necessarily take into consideration surface width, alinement, grades, and sight distances, as well as surface type. The clearances, strength, and condition of bridges, and the hazards and delays at railroad grade crossings, should also be considered. Information concerning all of these factors has been obtained in the highway planning surveys, but is not yet ready for presentation on a national basis. This article deals only with the service rendered by roads of different surface types in those States for which the information is now available.

Road construction practices vary in different sections of the country because of differences in climatic conditions, subsoil conditions, and character of available local materials as well as differences in wealth and in public policy as carried out by the State and local highway organizations. There are, then, no standards of adequacy that are generally applicable throughout the country. In one section, climatic conditions may be such that unsurfaced roads will satisfactorily serve much higher volumes of traffic than in another section. Subsoil conditions in one locality may require a higher type of surface for traffic of a given volume and weight composition than that required in another locality. Differences in available funds and mileages of roads needing improvement have caused States to adopt different standards of improvement for roads of equal traffic importance. These facts should be borne in mind in appraising the relative degrees of surface improvement shown by the tabulations in this article.

ROADS CLASSIFIED INTO FIVE GROUPS ACCORDING TO SURFACE TYPE

Because of differences in construction practices and in current terminology, considerable difficulty was experienced in classifying surface types on a comparable basis in all States. Although every effort was made to attain uniformity, minor differences in classification undoubtedly exist. In preliminary tabulations 12 classifications were used but these are combined into five groups in the accompanying tables. These five general surface type groups are defined in commonly used terms, without attempting to make precise distinctions. The terms used to describe the individual types composing each group are not, in all cases, mutually exclusive, as two or more types may be nearly identical. The distinctions between the general surface type groups, however, are reasonably definite in all cases.

1. *Pavement* includes concrete, brick, stone block, wood block, asphalt block, sheet asphalt, rock asphalt, bituminous concrete, and bituminous penetration.

2. Other dustless surface includes plant mix without precise control, road mix or mixed-in-place, and bituminous surface treated gravel or stone.

3. Nondustless surface includes plain macadam, gravel, traffic-bound crushed stone, slag, chert, caliche, iron ore, chats, sand-clay, and topsoil.

4. Graded and drained includes roads of natural earth, alined and graded to permit reasonably convenient use by motor vehicles, with drainage systems sufficient to prevent serious impairment of the road by surface water.

5. Unimproved includes primitive roads and trails usable by four-wheel vehicles and also earth roads on which some blading may have been done but which do not conform in respect to alignment, grade, and drainage to the requirement for a graded and drained road.

Roads with surfaces falling in the first three groups are called surfaced roads; roads falling in the last two groups are called unsurfaced roads.

The initial road inventory and traffic surveys were started in a few States as early as the fall of 1935 and completed toward the end of 1936. The work was started later in most States and, in a few, only very recently. The effective dates of figures in the accompanying tables vary from 1936 to 1939 and the period covered by the traffic survey does not, in all cases, coincide with that covered by the road inventory. Under the continuing survey an effort is being made to keep traffic and road inventory information current so that it should soon be possible to present up-to-date tabulations of both road inventory and traffic data for a specific year.

In interpreting the figures given in the tables and text of this report, it should be borne in mind that there are some differences in effective dates and that the data apply to different periods from 1936 to 1939. There have undoubtedly been changes since the effective dates of the data, because of the normal construction programs of Federal, State, and local agencies, and the surfacing of considerable mileages with the assistance of the Work Projects Administration in a number of States. On the whole, however, it is believed that changes have not been sufficiently large, relatively, to invalidate the general picture presented.

The total mileage of rural roads in the United States is estimated to be approximately 2,960,000 miles. Of this, approximately 1,200,000 miles, or about 40 percent, have surfaces permitting travel in all seasons of the

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Complete information on road mileage and suryear. face type as of various dates from August 1936 to December 1939 is available for 34 States. The total road mileage reported by these States was 2,219,723 miles, of which 840,129 miles, or 37.8 percent, were surfaced. Table 1 shows the mileage of each general surface type and the percentage of the total mileage for each of the 34 States.

The percentage of the total mileage that was surfaced ranged from 85.4 percent in Ohio, to 13.5 percent in Nevada. It will be noted that most of the surfaced mileage in Ohio consisted of gravel and other nondustless types, only 25.1 percent of the total mileage in this State having dustless surfaces. In New Jersey, on the other hand, in which 62.7 percent of the total mileage was surfaced, 49.4 percent had dustless surfaces. Nine States had less than 20 percent of their total road mileages surfaced. Figure 1 shows the percentage of the total mileage surfaced, the percentage with a dustless surface, and the percentage paved, for each of the 34 States, arranged in descending order of the percentage surfaced.

TRAFFIC ON PAVED ROADS 12 TIMES THE AVERAGE FOR ALL ROADS

Naturally, travel is generally heavier on roads having high type surfaces than it is on low type surfaces or on unsurfaced roads. In the first place, high traffic volume, actual or potential, was generally the cause of the construction of the higher type surfaces. In addition, traffic tends to gravitate to the more highly improved roads.

Combined traffic and road inventory information is available for 24 States. In these States the average daily traffic was 1,232 vehicles for pavements, 413 vehicles for other dustless surfaces, 77 vehicles for nondustless surfaces, 22 vehicles for graded and drained roads, and 13 vehicles for unimproved roads. The average for all types of road was 104 vehicles. Table 2 shows average daily traffic on each general surface

TABLE 2.—Average daily traffic on rural roads of each general surface type in each of 24 States

State	Pave- ment	Other dustless surface	Non- dustless surface	Graded and drained	Unim- proved	All roads
Arizona	946	522	83	50	13	. 84
California	1,485	266	83	40	17	236
Colorado	2,096	575	84	32	10	59
Florida	988	353	47	27	10	173
Idaho	1,896	551	79	25	12	70
Indiana	1,370	370	53	10	6	165
Iowa	1,014	386	87	25	17	94
Kansas	1, 239	590	98	.54	16	64
Louisiana	836	789	96	12	6	113
Maryland	1, 285	565	65	35	21	338
Michigan	1, 525	324	84	22	10	160
Missouri	1,303	377	60	14	4	76
Montana	505	293	49	26	9	38
Nevada	1, 535	233	38	22	5	33
North Dakota	1,641	484	67	11	2	17
Ohio	1,146	332	64	20	18	204
Oklahoma	1,356	638	153	24	6	78
Oregon	1,084	398	60	. 9	3	89
South Dakota	1,065	517	78	12	1	27
Texas	1, 199	641	95	78	25	122
Utah	1,418	416	57	17	5	72
Vermont	916	535	68	11	3	111
West Virginia	992	485	105	52	26	147
Wyoming	742	357	88	27	12	56
A verage	1, 232	413	77	22	13	104

TABLE 1.-Mileage of rural roads of each general surface type and percentage of total mileage of rural roads in each of 34 States

State	Date of in- ventory	Pavem	ent	Other dus surfac	stless	Nondust surfac	less	Total sur	faced	Graded draine	and d	Total impr	oved	Unimpro	ved 1	Total	
Arizona Arkansas California Colorado Florida	Dec. 1937 Jan. 1937 Dec. 1937 Aug. 1939 Dec. 1936	Miles 564. 5 1, 595. 4 10, 114. 7 493. 4 1, 835. 7	Per- cent 2.0 2.9 10.2 .7 6.4	Miles 2, 174. 4 624. 0 22, 183. 8 3, 218. 6 7, 599. 5	Per- cent 7.9 1.2 22.3 4.3 26.3	Miles 2, 317. 8 12, 528. 6 2 20, 333. 0 9, 307. 9 2, 435. 9	Per- cent 8.4 23.1 20.4 12.3 8.4	<i>Miles</i> 5, 056. 7 14, 748. 0 52, 631. 5 13, 019. 9 11, 871. 1	Per- cent 18.3 27.2 52.9 17.3 41.1	Miles 4, 218. 8 11, 648. 9 3, 810. 4 6, 658. 2 13, 828. 6	Per- cent 15.3 21.5 3.8 8.8 47.8	Miles 9, 275, 5 26, 396, 9 56, 441, 9 19, 678, 1 25, 699, 7	Per- cent 33.6 48.7 56.7 26.1 88.9	Miles 18, 271. 7 27, 889. 6 3 43, 118. 6 55, 576. 7 3, 212. 1	Per- cent 66.4 51.3 43.3 73.9 11.1	<i>Miles</i> 27, 547, 2 54, 286, 5 99, 560, 5 75, 254, 8 28, 911, 8	Per- cent 100.0 100.0 100.0 100.0
Idaho Illinois Indiana Iowa Kansas	Dec. 1936 Jan. 1937 Jan. 1937 Dec. 1937 Dec. 1936	$\begin{array}{r} 171. \ 9 \\ 11, \ 390. \ 9 \\ 5, \ 452. \ 4 \\ 4, \ 843. \ 5 \\ 1, \ 809. \ 8 \end{array}$.5 11.1 7.1 4.8 1.4	$\begin{array}{c} 1,940.2\\678.8\\6,390.8\\558.9\\3,341.7\end{array}$	5.8 .7 8.3 .5 2.6	$\begin{array}{c} 7,792.1\\ 47,899.5\\ 49,872.6\\ 35,077.3\\ 24,756.3\end{array}$	$\begin{array}{c} 23.\ 2\\ 46.\ 6\\ 65.\ 1\\ 33.\ 8\\ 19.\ 3\end{array}$	$\begin{array}{c} 9,904.2\\ 59,969.2\\ 61,715.8\\ 40,479.7\\ 29,907.8\end{array}$	$\begin{array}{c} 29.5 \\ 58.4 \\ 80.5 \\ 39.1 \\ 23.3 \end{array}$	$\begin{array}{r} 4,484.1\\ 37,747.5\\ 5,690.4\\ 40,794.9\\ 2,004.6\end{array}$	13. 436. 87. 440. 41. 6	$\begin{array}{c} 14,388.3\\97,716.7\\67,406.2\\81,274.6\\31,912.4\end{array}$	$\begin{array}{c} 42,9\\ 95,2\\ 87,9\\ 79,5\\ 24,9\end{array}$	19, 152. 2 4, 966. 9 9, 283. 9 4 20, 624. 3 96, 285. 4	57.1 4.8 12.1 20.5 75.1	$\begin{array}{c} 33,540.5\\ 102,683.6\\ 76,690.1\\ 101,898.9\\ 128,197.8\end{array}$	100.0 100.0 100.0 100.0 190.0
Kentucky Louisiana Maryland Michigan Missouri	July 1938 Dec. 1937 Jan, 1938 Dec. 1936 Dec. 1936	$\begin{array}{c} 1,910.\ 2\\ 3,405.\ 4\\ 2,447.\ 1\\ 5,464.\ 5\\ 3,998.\ 0 \end{array}$	3.4 8.8 15.3 5.9 3.4	$5,758.0 \\ 29.9 \\ 3,227.7 \\ 4,342.9 \\ 3,117.5$	$10.2 \\ .1 \\ 20.2 \\ 4.7 \\ 2.7$	$\begin{array}{c} 18,741.6\\ 13,198.0\\ 3,243.7\\ 48,991.9\\ 29,406.1 \end{array}$	34.0 34.1 20.3 53.1 25.2	$\begin{array}{c} 26,409.8\\ 16,633.3\\ 8,918.5\\ 58,799.3\\ 36,521.6 \end{array}$	$\begin{array}{r} 47.\ 6\\ 43.\ 0\\ 55.\ 8\\ 63.\ 7\\ 31.\ 3\end{array}$	$\begin{array}{r} 4,033.3\\15,701.0\\4,684.4\\21,837.3\\60,456.3\end{array}$	$\begin{array}{c} 7.2 \\ 40.5 \\ 29.3 \\ 23.8 \\ 51.8 \end{array}$	$\begin{array}{c} 30,443.1\\ 32,334.3\\ 13,602.9\\ 80,636.6\\ 96,977.9\end{array}$	54.8 83.5 85.1 87.5 83.1	$\begin{array}{c} 25,837.8\\ 6,387.2\\ 2,380.2\\ 11,509.5\\ 19,716.0 \end{array}$	$\begin{array}{c} 45,2\\ 16,5\\ 14,9\\ 12,5\\ 16,9\end{array}$	$\begin{array}{c} 56,280,9\\ 38,721,5\\ 15,983,1\\ 92,146,1\\ 116,693,9 \end{array}$	100.0 100.0 100.0 100.0 100.0
Montana Nebraska Nevada New Hampshire New Jersey	July 1937 Dec. 1936 Dec. 1937 Dec. 1937 July 1939	$1,466.4\\1,034.2\\51.5\\413.6\\4,852.0$	$2.2 \\ 1.0 \\ .2 \\ 3.4 \\ 26.2$	$\begin{array}{c} 2,965,7\\ 861,7\\ 2,338,6\\ 3,404,7\\ 4,292,2 \end{array}$	$\begin{array}{r} 4.5 \\ .9 \\ 10.0 \\ 28.0 \\ 23.2 \end{array}$	$\begin{array}{c} 6,248.7\\ 16,333.8\\ 761.2\\ 3,943.0\\ 2,475.3 \end{array}$	9.516.33.332.513.3	$\begin{array}{c} 10,680.8\\ 18,229.7\\ 3,151.3\\ 7,761.3\\ 11,619.5 \end{array}$	$\begin{array}{c} 16.2\\ 18.2\\ 13.5\\ 63.9\\ 62.7 \end{array}$	$\begin{array}{c} 3,389.9\\ 9,315.3\\ 865.0\\ 2,725.2\\ 342,2 \end{array}$	5.2 9.3 3.7 22.4 1.9	$\begin{array}{c} 14,070.7\\ 27,545.0\\ 4,016.3\\ 10,486.5\\ 11,961.7 \end{array}$	$\begin{array}{c} 21.\ 4\\ 27.\ 5\\ 17.\ 2\\ 86.\ 3\\ 64.\ 6\end{array}$	$51, 659, 4 \\72, 770, 0 \\19, 257, 6 \\1, 656, 4 \\6, 542, 1$	78.672.582.813.735.4	$\begin{array}{c} 65,730.1\\ 100,315.0\\ 23,273.9\\ 12,142.9\\ 18,503.8 \end{array}$	100.0 100.0 100.0 100.0 100.0
North Carolina North Dakota Ohio Oklahoma Oregon	June 1938 Jan. 1938 Jan. 1937 Jan. 1937 Sept. 1936	$\begin{array}{r} 4,975.0\\ 24,5\\ 8,059.9\\ 2,696.3\\ 1,799.7\end{array}$	8.6 (⁵) 9.8 2.7 3.9	$\begin{array}{r} 4,419.0\\ 689.1\\ 12,653.4\\ 1,369.4\\ 2,907.9\end{array}$	$7.6 \\ .6 \\ 15.3 \\ 1.3 \\ 6.2$	$\begin{array}{c} 20,759.2\\ 16,702.2\\ 49,700.6\\ 11,161.8\\ 14,599.3\\ \end{array}$	$\begin{array}{c} 35.9\\ 15.2\\ 60.3\\ 11.0\\ 31.2 \end{array}$	30, 153. 2 17, 415. 8 70, 413. 9 15, 227. 5 19, 306. 9	$52.1 \\ 15.8 \\ 85.4 \\ 15.0 \\ 41.3$	$\begin{array}{c} 23,490,9\\ 19,261,5\\ 7,012,7\\ 65,648,5\\ 15,118,7\end{array}$	$\begin{array}{r} 40.\ 6\\ 17.\ 5\\ 8.\ 5\\ 64.\ 7\\ 32.\ 3\end{array}$	$53, 644. 1 \\ 36, 677. 3 \\ 77, 426. 6 \\ 80, 876. 0 \\ 34, 425. 6$	92.7 33.3 93.9 79.7 73.6	$\begin{array}{r} 4,162.7\\73,376.2\\5,022.4\\20,529.3\\12,340.8\end{array}$	7.366.76.120.326.4	$57,806.8\\110,053.5\\82,449.0\\101,405.3\\46,766.4$	100.0 100.0 100.0 100.0 100.0
South Carolina South Dakota Texas Utah Vermont	Dec. 1938. Jan. 1937 Sept. 1937 Jan. 1937 Dec. 1938	$\begin{array}{c} 2, 326, 2\\ 228, 7\\ 6, 579, 8\\ 364, 9\\ 895, 2 \end{array}$	$5.4 \\ .2 \\ 3.5 \\ 1.7 \\ 6.6$	$\begin{array}{r} 3,854.0\\895.2\\12,485.9\\1,481.6\\438.0\end{array}$	9.0 .9 6.7 6.9 3.3	$\begin{array}{c} 6,830.3\\ 18,629.1\\ 25,170.8\\ 5,357.6\\ 5,659.9 \end{array}$	$ \begin{array}{c} 16.0\\ 18.4\\ 13.6\\ 24.9\\ 42.0 \end{array} $	$\begin{array}{c} 13,010.5\\19,753.0\\44,236.5\\7,204.1\\6,993.1\end{array}$	30.4 19.5 23.8 33.5 51.9	$\begin{array}{c} 11,089.2\\ 40,920.3\\ 16,058.7\\ 3,640.6\\ 4,384.0 \end{array}$	$\begin{array}{c} 25.9\\ 40.5\\ 8.6\\ 17.0\\ 32.6 \end{array}$	$\begin{array}{c} 24,099.7\\ 60,673.3\\ 60,295.2\\ 10,844.7\\ 11,377.1 \end{array}$	56.360.032.450.584.5	$18, 676. 1 \\ 40, 471. 8 \\ 125, 565. 5 \\ 10, 633. 0 \\ 2, 092. 9$	$\begin{array}{r} 43.7\\ 40.0\\ 67.6\\ 49.5\\ 15.5\end{array}$	$\begin{array}{r} 42,775.8\\ 101,145.1\\ 185,860.7\\ 21,477.7\\ 13,470.0 \end{array}$	100.0 100.0 100.0 100.0 100.0
Washington West Virginia Wisconsin Wyoming	Dec. 1939 Jan. 1937 Dec. 1936 Dec. 1937	$\begin{array}{c} 1,972.7\\ 2,437.4\\ 4,927.4\\ 14.8\end{array}$	4.0 7.5 6.0 .1	$\begin{array}{c} 2,997.2\\ 2,053.8\\ 5,751.6\\ 2,826.1 \end{array}$	6.2 6.3 7.0 11.4	$\begin{array}{c} 20,034.7\\ 6,134.2\\ 48,035.8\\ 1,200.1 \end{array}$	$\begin{array}{c} 41.2 \\ 18.8 \\ 58.4 \\ 4.8 \end{array}$	$\begin{array}{c} 25,004.6\\ 10,625.4\\ 58,714.8\\ 4,041.0 \end{array}$	51.4 32.6 71.4 16.3	$\begin{array}{c} 13,258.7\\ 6,393.0\\ 19,624.7\\ 1,185.4 \end{array}$	$27.3 \\ 19.6 \\ 23.9 \\ 4.8$	$\begin{array}{r} 38, 263, 3\\ 17, 018, 4\\ 78, 339, 5\\ 5, 226, 4 \end{array}$	78.752.295.321.1	$\begin{array}{c} 10,377.9\\ 15,571.7\\ 3,853.9\\ 19,498.4 \end{array}$	$21.3 \\ 47.8 \\ 4.7 \\ 78.9$	48, 641. 2 32, 590. 1 82, 193. 4 24, 724. 8	100.0 100.0 100.0 100.0
Total		100, 617. 6	4.5	133, 871. 8	6.0	605, 639, 9	27.3	840, 129. 3	37.8	501, 323. 2	22.6	1, 341, 452. 5	60.4	878, 270. 2	39.6	⁶ 2,219,722.7	100.0

Includes trails

Includes 6,727.7 miles of oiled earth. Includes 22.4 miles, surface type unreported. Includes 274.1 miles, surface type unreported.

Less than 0.05 percent.
 States not listed estimate their total mileage at 739,154 miles, giving total for the country as 2,958,877 miles of rural highway.

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type for each of the 24 States. Maryland had an average daily traffic on all roads of 338 vehicles, the highest of any of the States listed; while North Dakota had an average daily traffic on all roads of but 17 vehicles, the lowest of any of the States listed. In all of the States the average daily traffic was successively higher for each successively higher type surface.

Though the average daily traffic on paved roads is much greater than that on roads with lower type surfaces and on unsufraced roads, it does not follow that all of the paved mileage is more heavily traveled than any of the unpaved mileage. Table 3 shows the mileage and percentage of each general surface type in different average daily traffic volume groups in 23 of the 24 States listed in table 2. Data for Indiana are not included in table 3 because tabulations showing mileages of each surface type in traffic volume groups in that State were not available.

Table 3 shows that, in the 23 States, almost 5,000 miles of paved roads carry less than 100 vehicles per day while more than 10,000 miles of unimproved roads and nearly 12,000 miles of graded and drained roads carry higher volumes of traffic. This may reflect on the judgment exercised, in some cases, in selecting roads for improvement, or may indicate that considerations other than traffic importance influenced the selection. There are valid reasons, however, why such conditions should exist with respect to a portion of the mileage. Some of the paved mileage was lightly traveled during



FIGURE 2.—PERCENTAGE OF RURAL ROAD MILEAGE COMPARED

IGURE 2.—PERCENTAGE OF RURAL ROAD MILEAGE COMPARED TO PERCENTAGE OF VECHICLE-MILES OF TRAVEL FOR EACH GENERAL SURFACE TYPE IN 24 STATES. STATES INCLUDED ARE: ARIZONA, CALIFORNIA, COLORADO, FLORIDA, IDAHO, INDIANA, IOWA, KANSAS, LOUISIANA, MARYLAND, MICHIGAN, MISSOURI, MONTANA, NEVADA, NORTH DAKOTA, OHIO, OKLAHOMA, OREGON, SOUTH DAKOTA, TEXAS, UTAH, VERMONT, WEST VIRGINIA, AND WYOMING.

the period of the survey because of construction work on other parts of the road, temporarily diverting to other roads part of the normal traffic. Likewise, some of the paved sections connected with sections that are not yet improved and large traffic increases may be expected when the road is completely improved from one population center to another. On the other hand, some of the heavily traveled graded and drained mileage may have been temporarily in that status, during the course of stage construction. The true extent of overdevelopment or underdevelopment of highway facilities cannot be determined from summary tables of this kind, but can be determined only by a study of each individual road section, taking into consideration all of the pertinent circumstances. Such an approach is being used in a number of States in setting up programs for future improvement and in determining priorities for improvement.

PAVED ROADS CARRIED NEARLY HALF OF THE TOTAL TRAVEL ON ALL ROADS

In the 24 States listed in table 2, the average daily travel on all rural roads amounted to about 169,523,000 vehicle-miles. This is equivalent to 1,695,230 vehicles traveling an average of 100 miles each per day, or to 6,780,920 vehicles traveling an average of 25 miles each per day. About 152,309,000 vehicle-miles, or 90 percent of the total travel, was on surfaced roads, and only about 17,214,000 vehicle-miles on unsurfaced The vehicle-mileages reported for each general roads. surface type by the 24 States were as follows:

	Venicle-miles
Pavement	80, 262, 267
Other dustless surface	40, 791, 208
Nondustless surface	31, 255, 837
Graded and drained	8, 258, 911
Unimproved	8, 955, 018
-	
Total	169, 523, 241

Table 4 shows for each of the 24 States, the percentage

distribution of the total vehicle-mileage of travel by surface types in comparison with the percentage distribution of the total road mileage by surface types. It shows that paved roads, constituting only 4.0 percent of the total rural road mileage in these States, carried 47.3 percent of the total vehicles-mileage of travel. Surfaced roads of all types, constituting 34.9 percent of the total rural road mileage, carried 89.8 percent of the total vehicle-mileage. Unimproved roads constituted 42.7 percent of the total road mileage but

TABLE 3.—Mileage of rural roads and percentage of total rural mileage of each general surface type in different average daily traffic volume groups in 23 States 1

Average daily traffic	Pavem	ent	Other du surfac	stless ce	Nondustles	s surface	Graded and	drained	Unimpr	oved	Tota	1
0-24 25-49 50-99 100-199 200-299	Miles 1, 494. 4 2, 189. 6 1, 285. 1 3, 135. 3 3, 111. 4	Percent 2, 51 3, 67 2, 16 5, 26 5, 22	Miles 9, 562. 4 4, 372. 3 9, 451. 6 15, 061. 4 12, 258. 6	Percent 10.11 4.62 9.99 15.93 12.96	Miles 119, 650, 2 75, 522, 6 74, 497, 2 53, 045, 8 17, 217, 5	Percent 33.64 21.24 20.95 14.92 4.84	Miles 277, 841. 9 48, 730. 1 22, 526. 4 8, 171. 9 2, 012. 5	Percent 76.94 13.50 6.24 2.26 .56	Miles 606, 179. 6 51, 418. 5 24, 099. 8 7, 749. 3 1, 688. 6	Percent 87.60 7.43 3.48 1.12 .24	Miles 1, 014, 728, 5 182, 233, 1 131, 860, 1 87, 163, 7 36, 288, 6	Percent 64. 92 11. 66 8. 44 5. 58 2. 32
300-399 400-499 500-569 600-699 700-799	3, 135, 3 3, 244, 7 3, 410, 5 3, 434, 1 3, 493, 0	5.26 5.44 5.72 5.76 5.86	9, 506. 1 7, 993. 0 6, 009. 4 4, 374. 6 3, 702. 0	$\begin{array}{c} 10.05\\ 8.45\\ 6.35\\ 4.63\\ 3.92 \end{array}$	$\begin{array}{c} 7,783.4\\ 3,604.6\\ 1,900.6\\ 858.3\\ 575.4\end{array}$	$2.19 \\ 1.01 \\ .54 \\ .24 \\ .16$	$\begin{array}{c} 723.\ 4\\ 395.\ 8\\ 235.\ 5\\ 109.\ 4\\ 77.\ 1\end{array}$	$ \begin{array}{r} 20 \\ .11 \\ .07 \\ .03 \\ .02 \end{array} $	$\begin{array}{r} 477.8\\173.5\\119.4\\62.3\\28.3\end{array}$	$ \begin{array}{c} .07 \\ .03 \\ .02 \\ .01 \\ (2) \end{array} $	$\begin{array}{c} 21,626.0\\ 15,411.6\\ 11,675.4\\ 8,838.7\\ 7,875.8\end{array}$	1.38 .99 .75 .57 .50
800-899 900-999 1,000-1,249. 1,250-1,499. 1,500-1,999.	$\begin{array}{c} 3,092.\ 7\\ 2,682.\ 2\\ 5,980.\ 0\\ 4,474.\ 5\\ 5,941.\ 8\end{array}$	5.19 4.50 10.03 7.50 9.96	$\begin{array}{c} 2,572,1\\ 1,726,3\\ 2,966,5\\ 1,940,8\\ 1,730,5\end{array}$	$\begin{array}{c} 2.72 \\ 1.83 \\ 3.14 \\ 2.05 \\ 1.83 \end{array}$	$\begin{array}{c} 334.\ 7\\ 163.\ 8\\ 229.\ 7\\ 127.\ 1\\ 63.\ 1\end{array}$.09 .05 .06 .04 .02	57. 739. 240. 321. 134. 9	$02 \\ 01 \\ 01 \\ 01 \\ 01 \\ 01$	$\begin{array}{c} 30.8 \\ 14.2 \\ 16.0 \\ 8.7 \\ 15.9 \end{array}$	(2) (2) (2) (2) (2) (2)	$\begin{array}{c} 6,088.0\\ 4,625.7\\ 9,232.5\\ 6,572.2\\ 7,786.2 \end{array}$. 39 . 30 . 59 . 42 . 50
2,000-2,999 3,000-3,999 4,000-4,999 5,000-5,999 6,000-6,999	$5, 243. 6 \\1, 862. 9 \\1, 008. 7 \\664. 0 \\282. 2$	8.79 3.12 1.69 1.11 .47	$928. \ 4 \\ 240. \ 9 \\ 111. \ 3 \\ 24. \ 8 \\ 21. \ 8 \\$.98 .25 .12 .03 .02	51.2 17.9 4.6 11.4	.01 (²) (²) (²)	10.9 24.7	(2) 01	8.2	(2)	$\begin{array}{c} 6,242,3\\2,146,4\\1,124,6\\700,2\\304,0\end{array}$. 40 . 14 . 07 . 04 . 02
7,000-7,999 8,000-8,999 9,000-9,999 10,000-12,499 12,500-14,999	$208. 1 \\ 83. 3 \\ 44. 1 \\ 66. 9 \\ 29. 0$	$ \begin{array}{r} .35\\ .14\\ .07\\ .11\\ .05 .05 $	$\begin{array}{c} 4.9\\ 1.0\\ .3\\ 3.1\\ 5.2 \end{array}$.01 (2) (2) (2) (2) .01							213. 084. 344. 470. 034. 2	$\begin{array}{c} . 01 \\ . 01 \\ (^2) \\ (^2) \\ (^2) \\ (^2) \end{array}$
15,000-19,999 20,000-24,999 25,000-29,999 30,000 and over	30.7 3.5 2.3	.05 .01 (²)									30. 7 3. 5 2. 3	(2) (2) (2)
Total. Percentage of total mileage	59, 633, 9 3, 82	100.00	94, 569. 3 6. 05	100.00	355, 659. 1 22, 75	100.00	361, 052. 8 23. 10	100.00	692, 090. 9 44. 28	100.00	1, 563, 006. 0 100. 00	100.00

¹ The States included are: Arizona, California, Colorado, Florida, Idaho, Iowa, Kansas, Louisiana, Maryland, Michigan, Missouri, Montana, Nevada, North Dakota, Ohio, Oklahoma, Oregon, South Dakota, Texas, Utah, Vermont, West Virginia, and Wyoming. ² Less than 0.005 percent.

TABLE 4.—Mileage of rural roads of each general surface type and vehicle-miles of travel on each, expressed as percentages of totals for all types in each of 24 States

	Pave	ment	Other of sur	lustless face	Nond	ustless face	Total s	urfaced	Grade drai	d and ned	Total in	nproved	Unim	proved	To	tal
State	Per- centage of mile- age	Per- centage of vehicle- miles	Per- centage of mile- age	Per- centage of vehicle- miles	Per- centage of mile- age	Per- centage of vehicle- miles	Per- centage of mile- age	Per- centage of vehicle- miles	Per- centage of mile- age	Per- centage of vehicle- miles	Per- centage of mile- age	Per- centage of vehicle- miles	Per- centage of mile- age	Per- centage of vehicle- miles	Per- centage of mile- age	Per- centage of vehicle- miles
Arizona California Colorado Florida Idaho	2.0 10.2 .7 6.4 .5	$23.1 \\ 63.9 \\ 23.1 \\ 36.2 \\ 13.9$	7.922.34.326.35.8	$\begin{array}{r} 49.0\\ 25.1\\ 41.3\\ 53.5\\ 45.6\end{array}$	8.420.412.38.423.2	8.3 7.2 17.9 • 2.3 26.3	$18.3 \\ 52.9 \\ 17.3 \\ 41.1 \\ 29.5$	80. 4 96. 2 82. 3 92. 0 85. 8	15.3 3.8 8.8 47.8 13.4	$9.1 \\ .6 \\ 4.8 \\ 7.4 \\ 4.7$	33. 656. 726. 188. 942. 9	89.5 96.8 87.1 99.4 90.5	$\begin{array}{r} 66.4\\ 43.3\\ 73.9\\ 11.1\\ 57.1 \end{array}$	$ \begin{array}{r} 10.5 \\ 3.2 \\ 12.9 \\ .6 \\ 9.5 \end{array} $	100.0 100.0 100.0 100.0 100.0 100.0	$ \begin{array}{r} 100.0 \\ 100.0 \\ 100.0 \\ 100.0 \\ 100.0 \end{array} $
Indiana. Iowa Kansas Louisiana. Maryland	$7.1 \\ 4.8 \\ 1.4 \\ 8.8 \\ 15.3$	$59.3 \\ 51.4 \\ 27.2 \\ 65.8 \\ 58.3$	$8.3 \\ .5 \\ 2.6 \\ .1 \\ 20.2$	$18.7 \\ 2.2 \\ 23.9 \\ .3 \\ 33.8$	$\begin{array}{c} 65.1\\ 33.8\\ 19.3\\ 34.1\\ 20.3 \end{array}$	21. 232. 129. 428. 73. 9	$\begin{array}{c} 80.\ 5\\ 39.\ 1\\ 23.\ 3\\ 43.\ 0\\ 55.\ 8\end{array}$	$\begin{array}{c} 99.\ 2\\ 85.\ 7\\ 80.\ 5\\ 94.\ 8\\ 96.\ 0\end{array}$	7.440.41.640.529.3	$\begin{array}{r} & 4 \\ 10.\ 6 \\ 1.\ 3 \\ 4.\ 3 \\ 3.\ 0 \end{array}$	$\begin{array}{r} 87.9 \\ 79.5 \\ 24.9 \\ 83.5 \\ 85.1 \end{array}$	99.696.381.899.199.0	12. 120. 575. 116. 514. 9	.4 3.7 18.2 .9 1.0	$100. 0 \\ 100. 0 \\ 100. 0 \\ 100. 0 \\ 100. 0 \\ 100. 0$	$ \begin{array}{c} 100. \\ 0\\ 100. \\ 0\\ 100. \\ 0\\ 100. \\ 0 \end{array} $
Michigan Missouri Montana Nevada North Dakota	5.9 3.4 2.2 .2 (1)	$58.2 \\ 57.8 \\ 30.1 \\ 10.2 \\ 2.2$	$\begin{array}{r} 4.7\\ 2.7\\ 4.5\\ 10.0\\ .6\end{array}$	$9.7 \\ 12.3 \\ 35.2 \\ 70.2 \\ 17.9$	53.1 25.2 9.5 3.3 15.2	$28.1 \\19.7 \\12.4 \\3.8 \\59.6$	$\begin{array}{c} 63.7\\ 31.3\\ 16.2\\ 13.5\\ 15.8\end{array}$	96. 0 89. 8 77. 7 84. 2 79. 7	$23.8 \\ 51.8 \\ 5.2 \\ 3.7 \\ 17.5$	3.39.33.62.511.0	$\begin{array}{r} 87.5\\83.1\\21.4\\17.2\\33.3\end{array}$	$\begin{array}{c} 99.3\\ 99.1\\ 81.3\\ 86.7\\ 90.7 \end{array}$	$12.5 \\ 16.9 \\ 78.6 \\ 82.8 \\ 66.7$.7 .9 18.7 13.3 9.3	$ \begin{array}{c} 100. \ 0 \\ 100. \ 0 \\ 100. \ 0 \\ 100. \ 0 \\ 100. \ 0 \end{array} $	$ \begin{array}{c} 100. \ 0 \\ 100. \ 0 \\ 100. \ 0 \\ 100. \ 0 \\ 100. \ 0 \end{array} $
Ohio Oklahoma Oregon South Dakota Texas	9.8 2.7 3.9 .2 3.5	54.8 45.9 47.0 9.0 34.7	$15.3 \\ 1.3 \\ 6.2 \\ .9 \\ 6.7$	$\begin{array}{c} 24.9\\ 11.0\\ 27.9\\ 16.9\\ 35.2 \end{array}$	$\begin{array}{c} 60.\ 3\\ 11.\ 0\\ 31.\ 2\\ 18.\ 4\\ 13.\ 6\end{array}$	$\begin{array}{c} 18.9\\ 21.5\\ 20.9\\ 53.6\\ 10.5 \end{array}$	$\begin{array}{c} 85.4\\ 15.0\\ 41.3\\ 19.5\\ 23.8\end{array}$	$\begin{array}{c} 98.\ 6\\ 78.\ 4\\ 95.\ 8\\ 79.\ 5\\ 80.\ 4\end{array}$	$\begin{array}{r} 8.5 \\ 64.7 \\ 32.3 \\ 40.5 \\ 8.6 \end{array}$	$\begin{array}{r} .8\\ 19.9\\ 3.2\\ 18.3\\ 5.5\end{array}$	$\begin{array}{c} 93.\ 9\\ 79.\ 7\\ 73.\ 6\\ 60.\ 0\\ 32.\ 4\end{array}$	99. 4 98. 3 99. 0 97. 8 85. 9	$\begin{array}{c} 6.1 \\ 20.3 \\ 26.4 \\ 40.0 \\ 67.6 \end{array}$	$\begin{array}{r} .6\\ 1.7\\ 1.0\\ 2.2\\ 14.1\end{array}$	$\begin{array}{c} 100.\ 0\\ 100.\ 0\\ 100.\ 0\\ 100.\ 0\\ 100.\ 0 \end{array}$	100.0 100.0 100.0 100.0 100.0
Utah Vermont West Virginia Wyoming	$ \begin{array}{r} 1.7 \\ 6.6 \\ 7.5 \\ .1 \\ \end{array} $	$33.4 \\ 55.0 \\ 50.4 \\ .8$	$ \begin{array}{r} 6.9 \\ 3.3 \\ 6.3 \\ 11.4 \end{array} $	$39.9 \\ 15.7 \\ 20.9 \\ 72.7$	$24.9 \\ 42.0 \\ 18.8 \\ 4.8 $	$19.7 \\ 25.7 \\ 13.5 \\ 7.6$	$33.5 \\ 51.9 \\ 32.6 \\ 16.3$	$93.0 \\ 96.4 \\ 84.8 \\ 81.1$	$17. 0 \\ 32. 6 \\ 19. 6 \\ 4. 8$	3.9 3.2 6.8 2.3	50.5 84.5 52.2 21.1	$\begin{array}{c} 96.9\\ 99.6\\ 91.6\\ 83.4 \end{array}$	49.5 15.5 47.8 78.9	3.1 .4 8.4 16.6	$100.0 \\ 100.0 \\ 100.0 \\ 100.0 \\ 100.0$	100. 0 100. 0 160. 0 100. 0
Average	4.0	47.3	6.1	24.1	24.8	18.4	34.9	89.8	22.4	4.9	57.3	94.7	42.7	5.3	100.0	100.0

¹ Less than 0.05 percent.

carried only 5.3 percent of the total vehicle-mileage. This shows that the improvement of roads now unimproved would mean, in general, the construction of relatively large mileages to serve a relatively small portion of the vehicle-miles of travel. These relationships are presented graphically in figure 2.

Not only do the roads of higher type surface serve greater traffic densities than those of lower type surface, but they also, in general, pass through more densely populated rural areas. Table 5 shows the number per mile of farm units and other rural dwellings along roads with different type surfaces in each of 32 States. The States are grouped into four regions from east to west. The greatest density of dwellings along all rural roads is in the eastern region; the next greatest is in the western region; and the density is lower for the two intermediate regions. For all 32 States, there are, on the average, 2.7 rural dwellings per mile of rural road. The dwelling density along paved roads is 7.1 per mile and is lower for each successively lower general surface type, ranging down to 1.5 per mile for unimproved roads. In individual States, however, the roads of higher surface type do not always have the greater dwelling densities. In Vermont, Michigan, Illinois, Maryland, Missouri, Texas, and Louisiana for example, there are fewer houses per mile along paved roads than along roads having other dustless surfaces, and in North Dakota, Nebraska, Montana, Nevada, and Arizona there are fewer houses per mile along roads having dustless surfaces other than pavement, than along roads having nondustless surfaces. These minor varia-

 TABLE 5.—Number of rural dwellings per mile along roads with different general surface types by States (grouped by regions)

Region and State	Pave- ment	Other dustless surface	Nondust- less sur- face	Graded and drained	Unim- proved	All types
Design to						
Region 1: Now Hampshire	10.0	7.8	3.6	2.6	12	43
Vermont	7.4	82	4.0	2.5	1.4	3.5
Michigan	6.3	7.2	4.3	2.8	1.1	3.8
Illinois	4.6	9.8	3.2	2.4	1.7	3.1
Ohio	10.0	7.2	4.2	2.3	2.1	5.0
Maryland	10.0	10.4	6.5	4.3	3.9	6.7
West Virginia	13.9	11.2	6.4	4.0	3.3	5.4
Kentucky	9.2	8.3	5.1	4.5	4.0	5.0
North Carolina	10.8	8.1	0.4	0.0	0.4	0.8
Florida	7 2	4.6	31	2.6	2.0	3.4
r iorida		1.0		4.0		
Average	8.1	7.4	4.3	3.4	3.2	4.5
Region 2:		-				
North Dakota	1.6	1.0	1.1	. 9	. 6	.7
South Dakota	1.9	1.6	1.3	9	.4	. 8
Wisconsin	5.3	4.2	3.2	1.9	.9	3.0
Iowa	2.7	2.6	2.6	2.1	1.9	2.3
Minooumi	2.0	1.2	2.0	1.0	1.1	1.3
Kaness	6.4	0.0	2.2	2.0	1.0	1.5
Arkansas	5.8	4 7	4.0	3 2	3.1	3.4
Oklahoma	3.5	3.1	2.8	2.2	1.5	2.2
Texas	4.0	4.5	3.8	2.1	2.4	2.7
. Louisiana	7.5	9.7	5.9	3.2	2.9	4.4
Average	4.5	4.0	2.9	2.0	1.5	2.1
Region 3:						
Montana	1.9	1.6	1.8	1.0	.7	. 9
Idaho	8.2	2.9	2.8	1.4	1.0	1.6
Wyoming	4.1	1.4	1.3	. 6	.8	. 9
Colorado	4.2	2.3	2.2	1.2	. 9	1.2
Utan.	9.8	3.6	2.0	1.1	.5	1.3
Arizono	4.8	. 9	1.0	.8	1 4	.0
Allouia	15.0	0.4	0.4	4. 1	1. 9	
A verage	5.6	2.1	2.3	1.4	. 8	1.2
Region 4:						
Washington	9.7	4.1	3.8	1.3	.8	2.8
Oregon	6.6	3.9	3.4	.7	.4	1.9
California	10.8	6.8	2.5	2.0	1.0	3.7
Average	10.1	6.1	3.4	1, 1	. 9	3.0
represented	7 1	5.6	3 5	2.2	1.5	2.7
. of the owned at a start of the	1.1	0.0	0.0	2.0	1.0	2.1

DWELLINGS SERVED BY SURFACED ROADS (Asteriore) VEHICLE-MILES SERVED BY SURFACED ROADS 1777 OHIO CALIFORNIA OREGON MICHIGAN MARYLAND VERMONT UTAH LOUISIANA FLORIDA W. VIRGINIA IDAHO IOWA ARIZONA MISSOURI KANSAS TEXAS S. DAKOTA MONTANA COLORADO NEVADA WYOMING N. DAKOTA OKLAHOMA AVERAGE 40 60 80



tions in the general trend are caused by the building of high-type roads, important to through traffic, across relatively sparsely settled areas.

In table 6, and in figure 3, a comparison is made of the percentage of the rural road mileage surfaced, the percentage of the vehicle-miles of travel served by surfaced roads, and the percentage of the rural dwellings directly served by surfaced roads, in each of 23 States. For these States, 37.8 percent of the total rural road mileage is surfaced, and this surfaced mileage serves directly 57.2 percent of the rural dwellings and accommodates 89.8 percent of the vehicle-miles of travel on rural roads. In Ohio, 93.8 percent of the rural dwellings are directly served by surfaced roads, whereas in

MILEAGE OF SURFACED ROADS

PUBLIC ROADS

NON-DUSTLESS SURFACE DUSTLESS SURFACE FTA GRADED AND DRAINED UNIMPROVED 2017 WISCONSIN ILLINOIS MICHIGAN 1 N CAROLINA NEW HAMPSHIRE WASHINGTON VERMONT FLORIDA OREGON MARYLAND MISSOURI LOUISIANA CALIFORNIA OKLAHOMA IOWA UTAH S. DAKOTA W. VIRGINIA IDAHO KENTUCKY S. CAROLINA ARIZONA ARKANSAS N. DAKOTA TEXAS COLORADO KANSAS NEBRASKA MONTANA 8077 NEVADA 8277 WYOMING AVERAGE 20 80 100 60 40

PERCENTAGE OF ALL RURAL DWELLINGS IN EACH STATE

FIGURE 4.—PERCENTAGE OF ALL RURAL DWELLINGS DIRECTLY SERVED BY ROADS WITH DIFFERENT GENERAL SURFACE TYPES IN EACH OF 32 STATES.

Oklahoma only 20.1 percent are so served. The percentages of rural dwellings served by surfaced roads in other States range between these two extremes.

The percentages of all rural dwellings directly served by roads with different general surface types are shown for each of 32 States in table 7 and in figure 4. By omitting vehicle-mileage data, it was possible to include 9 more States in this table than in table 6. In these 32 States, 57.2 percent of all rural dwellings were directly served by surfaced roads, which is exactly the same percentage as that shown in table 6 for 23 States. Only 22.6 percent of the rural dwellings were located on unimproved roads. For individual States, the percentage of rural dwellings located on unimproved roads varied from 71.2 percent in Wyoming down to 1.5 percent in Wisconsin.

TWO-THIRDS OF RURAL DWELLINGS IN 10 STATES LOCATED WITHIN 1 MILE OF SURFACED ROAD

Many rural dwellings which do not front directly on improved roads are located close to them so that the occupants need travel only a short distance to get to an improved highway. Under such conditions, the actual mileage which need be driven on unimproved roads is very small in relation to the total mileage driven on an average trip. In 10 States, studies to determine the number of rural dwellings located within different travel distances of improved roads have beep completed.

Table 8 and figure 5 show the percentages of all rural

 TABLE 6.—Percentage of rural road mileage surfaced, and percentage of all rural travel and cf all rural dwellings directly served by surfaced roads in each of 23 States

State	Percentage of total rural road mileage surfaced	Percentage of total rural vehicle-mile- age served by surfaced roads	Percentage of all rural dwellings di- dectly served by surfaced roads
Arizona	$ \begin{array}{c} 18.3\\52.9\\17.3\\41.1\\29.5\end{array} $	80. 4	41.7
California		96. 2	85.7
Colorado		82. 3	31.5
Florida		92. 0	56.7
Idaho		85. 8	53.5
Iowa	$\begin{array}{c} 39.\ 1\\ 23.\ 3\\ 43.\ 0\\ 55.\ 8\\ 63.\ 7\end{array}$	85. 7	45.8
Kansas		80. 5	40.2
Louisiana		94. 8	61.2
Maryland		96. 0	76.2
Michigan		96. 0	79.8
Missouri.	31.3	89.8	40. 9
Montana	16.2	77.7	32. 4
Nevada	13.5	84.2	29. 8
North Dakota	15.8	79.7	24. 2
Ohio.	85.4	98.6	93. 8
Oklahoma.	15. 0	78. 4	20. 1
Oregon.	41. 3	95. 8	81. 5
South Dakota.	19. 5	79. 5	32. 8
Texas.	23. 8	80. 4	35. 2
Utah.	33. 5	93. 0	67. 4
Vermont	51.9	96. 4	70. 3
West Virginia	32.6	84. 8	56. 1
Wyoming	16.3	81. 1	25. 4
Average	37.8	89.8	57.2

 TABLE 7.—Percentage of all rural dwellings directly served by roads

 with different general surface types, in each of 32 States

State	Pave- ment	Other dustless surfaces	Non- dustless surfaces	All sur- faced roads	Graded and drained roads	All im- proved roads	Unim- proved roads
Arizona Arkansas California Colorado Florida	$12.6 \\ 5.0 \\ 29.8 \\ 2.2 \\ 13.4$	$11.7 \\ 1.6 \\ 40.3 \\ 5.3 \\ 35.3$	17.427.115.624.08.0	41. 7 33. 7 85. 7 31. 5 56. 7	16.419.72.19.936.9	58.1 53.4 87.8 41.4 93.6	$\begin{array}{c} 41.9\\ 46.6\\ 12.2\\ 58.6\\ 6.4 \end{array}$
Idaho. Illinois. Iowa. Kansas. Kentucky	$2.6 \\ 16.7 \\ 5.7 \\ 6.0 \\ 6.3$	$ \begin{array}{r} 10.2 \\ 2.1 \\ .6 \\ 4.2 \\ 17.0 \\ \end{array} $	$\begin{array}{r} 40.\ 7\\ 49.\ 7\\ 39.\ 5\\ 30.\ 0\\ 34.\ 6\end{array}$	53.5 68.5 45.8 40.2 57.9	11.528.737.41.16.4	$\begin{array}{r} 65.\ 0\\ 97.\ 2\\ 83.\ 2\\ 41.\ 3\\ 64.\ 3\end{array}$	35.0 2.8 16.8 58.7 35.7
Louisiana Maryland Michigan Missouri Montana	$ \begin{array}{r} 16.3 \\ 24.3 \\ 9.9 \\ 5.2 \\ 4.8 \\ \end{array} $	$\begin{array}{r} .1\\ 33.6\\ 8.9\\ 5.6\\ 8.1\end{array}$	$\begin{array}{r} 44.8 \\ 18.3 \\ 61.0 \\ 30.1 \\ 19.5 \end{array}$	$\begin{array}{c} 61.\ 2\\ 76.\ 2\\ 79.\ 8\\ 40.\ 9\\ 32.\ 4\end{array}$	$28.3 \\ 16.4 \\ 16.7 \\ 48.8 \\ 5.8 \\ 5.8 \\$	89.5 92.6 96.5 89.7 38.2	10.5 7.4 3.5 10.3 61.8
Nebraska Nevada New Hampshire North Carolina North Dakota	2.0 2.1 7.6 15.7 .1	.8 17.3 48.9 10.4 .8	$\begin{array}{c} 25.1 \\ 10.4 \\ 25.9 \\ 32.9 \\ 23.3 \end{array}$	$\begin{array}{c} 27.9\\ 29.8\\ 82.4\\ 59.0\\ 24.2 \end{array}$	$10.7 \\ 5.6 \\ 12.9 \\ 36.9 \\ 22.7$	38.6 35.4 95.3 95.9 46.9	61. 4 64. 6 4. 7 4. 1 53. 1
Ohio Oklahoma Oregon South Carolina South Dakota	$20.0 \\ 4.3 \\ 13.4 \\ 8.9 \\ .6$	$22.3 \\ 1.9 \\ 12.7 \\ 11.7 \\ 1.8$	51.5 13.9 55.4 16.6 30.4	$93.8 \\ 20.1 \\ 81.5 \\ 37.2 \\ 32.8$	3.8 65.5 11.9 27.1 47.2	$97. \ 6 \\ 85. \ 6 \\ 93. \ 4 \\ 64. \ 3 \\ 80. \ 0$	$\begin{array}{c} 2.4\\ 14.4\\ 6.6\\ 35.7\\ 20.0 \end{array}$
Texas. Utah Vermont Washington West_Virginia	5.2 10.0 14.2 14.2 19.6	$ \begin{array}{r} 11.0 \\ 19.9 \\ 7.7 \\ 9.3 \\ 13.4 \end{array} $	$19.0 \\ 37.5 \\ 48.4 \\ 57.4 \\ 23.1$	$\begin{array}{c} 35.2 \\ 67.4 \\ 70.3 \\ 80.9 \\ 56.1 \end{array}$	$\begin{array}{r} 6.5 \\ 14.2 \\ 23.6 \\ 13.2 \\ 14.8 \end{array}$	$\begin{array}{r} 41.\ 7\\ 81.\ 6\\ 93.\ 9\\ 94.\ 1\\ 70.\ 9\end{array}$	58, 3 18, 4 6, 1 5, 9 29, 1
Wisconsin Wyoming	10.7 .3	10.0 17.7	62.6 7.4	83. 3 25. 4	15.2 3.4	98.5 28.8	1.5 71.2
Average	11.3	12.2	33.7	57.2	20.2	77.4	22.6

dwellings within various travel distances of surfaced roads, in the 10 States in which this information is available. In these States 65.0 percent of the rural dwellings were within 1 mile of a surfaced road, and 77.5 percent were within 2 miles of a surfaced road.

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GRAPHICAL ANALYSES OF THE STABILITY OF SOIL

BY THE DIVISION OF TESTS, PUBLIC ROADS ADMINISTRATION

Reported by E. S. BARBER, Junior Highway Engineer, and C. E. MERSHON, Junior Engineer

TO FURTHER the development of rational procedures for use in the design and construction of highways during the past two decades, the Public Roads Administration has made comprehensive studies of published material and conducted supplementary laboratory research on the stability of soil. Results of this work, published in PUBLIC ROADS and in the Proceedings of the Highway Research Board, include the interpretation of test data, the evaluation of pressure against retaining walls, the design of cuts and embankment cross sections, and the estimation of the supporting value of undersoil. In an effort to expedite a general use of the theories as a basis of correlation with experience, this report

In an effort to expedite a general use of the theories as a basis of correlation with experience, this report presents methods of analyses in which charts are used to facilitate computations and thus greatly reduce the time and labor required in the application of the formulas. A summary of the development of the formulas and a brief discussion of the assumptions on which the theories are based are first presented. An explanation of the construction and use of the charts then follows.

STABILITY OF SOIL DEPENDS UPON ITS SHEARING RESISTANCE

The stability of a soil is assumed to depend upon its shearing resistance which, according to Coulomb's classical theory published in 1773 (1),¹ is expressed by the relation $s=c+n \tan \phi$(1)

 $s = c + \frac{1}{2}$

in which

where

s = unit shearing resistance,

c =unit cohesion,

n =stress normal to the plane of shear, and

 ϕ = angle of internal friction.

Cohesion is defined as that component of shearing resistance which is independent of the stress normal to the plane of shear. (See fig. 1.) Internal friction is defined as that component of shearing resistance which is directly proportional to the stress normal to the plane of shear.

A factor of safety with respect to total strength may be applied by dividing c and $\tan \phi$ by the desired factor (2). For a deformation less than that at failure, the corresponding c and ϕ (3) may be used by assuming a hypothetical soil with these ultimate values.

Compressive strength.—The relation between the unit compressive strength, v_0 , of an unconfined cylindrical soil sample, its cohesion, and its angle of internal friction, was discussed in the Proceedings of the Nineteenth Annual Meeting of the Highway Research Board (4). The formula is

 $v_0 = 2c \tan \alpha_{-----}(2)$

$$\alpha = 45^{\circ} + \frac{\phi}{2}$$







 $U = 2 c TAN \alpha + l TAN^2 \alpha$

FIGURE 2.—RELATION OF PRINCIPAL STRESSES AT FAILURE OF ANY POINT IN A STRESSED EARTH MASS.

If a unit lateral pressure, l, is applied to a sample as in figure 2, the expression for its unit compressive strength, v, as published in PUBLIC ROADS, December 1938 (5), becomes

$$v = 2c \tan \alpha + l \tan^2 \alpha \dots (3)$$

¹ Italic figures in parentheses refer to bibliography, page 155.

Tangent functions of α and its complement β are given in table 1.

The relation between the maximum height, H, at which an unrestrained embankment will stand vertically, the unit weight of the soil, w, and its shearing resistance, was discussed in PUBLIC ROADS, December 1929 (6). The expression is

$$H = \frac{v_0}{w} = \frac{2c}{w} \tan \alpha$$
(4)

TABLE	17	angent	functions	of α	and β
-------	----	--------	-----------	-------------	-------------

a derroes	fan d	$\alpha = 1$	$5^\circ + \frac{\phi}{2}$	$\beta = 15^{\circ} - \frac{\phi}{2}$					
φ, αυ ₃ του	0	$\tan \alpha = \cot \beta$	$\tan^2 \alpha = \\ \cot^2 \beta$	$\tan \beta = \cot \alpha$	$\tan^2\beta = \cot^2\alpha$				
0 1 2 3 4 5	$\begin{array}{c} 0 \\ 0.017 \\ .035 \\ .052 \\ .070 \\ .087 \end{array}$	1. 000 1. 018 1. 036 1. 054 1. 072 1. 091	$\begin{array}{c} 1.\ 000\\ 1.\ 036\\ 1.\ 072\\ 1.\ 110\\ 1.\ 150\\ 1.\ 191 \end{array}$	$\begin{array}{c} 1.\ 000\\ .\ 983\\ .\ 966\\ .\ 949\\ .\ 933\\ .\ 916\end{array}$	1. 000 . 966 . 933 . 901 . 870 . 840				
6 7 8 9 10	. 105 . 123 141 . 158 . 176	1. 111 1. 130 1. 150 1. 171 1. 192	$\begin{array}{c} 1.\ 233\\ 1.\ 278\\ 1.\ 323\\ 1.\ 371\\ 1.\ 420 \end{array}$. 900 . 885 . 869 . 854 . 839	. 811 . 783 . 756 . 729 . 704				
11 12 13 14 15	. 194 . 213 . 231 . 249 . 268	1. 213 1. 235 1. 257 1. 280 1. 303	$\begin{array}{c} 1.\ 472\\ 1.\ 525\\ 1.\ 580\\ 1.\ 638\\ 1.\ 698 \end{array}$. 824 . 810 . 795 . 781 . 767	. 680 . 656 . 633 . 610 . 589				
16 17 18 19 20	287 306 325 344 364	$\begin{array}{c} 1.\ 327\\ 1.\ 351\\ 1.\ 376\\ 1.\ 402\\ 1.\ 428 \end{array}$	$\begin{array}{c} 1.\ 761 \\ 1.\ 826 \\ 1.\ 894 \\ 1.\ 965 \\ 2.\ 040 \end{array}$. 754 . 740 . 727 . 713 . 700	.568 .548 .528 .509 .490				
21 22 23 24 25	. 384 . 404 . 424 . 445 . 466	$\begin{array}{c} 1.\ 455\\ 1.\ 483\\ 1.\ 511\\ 1.\ 540\\ 1.\ 570 \end{array}$	$\begin{array}{c} 2.\ 117\\ 2.\ 198\\ 2.\ 283\\ 2.\ 371\\ 2.\ 464 \end{array}$.687 .675 .662 .649 .637	$\begin{array}{c} .\ 472\\ .\ 455\\ .\ 438\\ .\ 422\\ .\ 406\end{array}$				
26 27 28 29 30	. 488 . 510 . 532 . 554 . 577	$\begin{array}{c} 1.\ 600\\ 1.\ 632\\ 1.\ 664\\ 1.\ 698\\ 1.\ 732 \end{array}$	$\begin{array}{c} 2.\ 561 \\ 2.\ 663 \\ 2.\ 770 \\ 2.\ 882 \\ 3.\ 000 \end{array}$.625 .613 .601 .589 .577	$ \begin{array}{r} 390 \\ 376 \\ 361 \\ 347 \\ 333 \end{array} $				
31 32 33 34 35	. 601 . 625 . 649 . 675 . 700	1, 767 1, 804 1, 842 1, 881 1, 921	$\begin{array}{c} 3.\ 124\\ 3.\ 255\\ 3.\ 392\\ 3.\ 537\\ 3.\ 690 \end{array}$. 566 . 554 . 543 . 532 . 521	. 320 . 307 . 295 . 283 . 271				
$ 36 \\ 37 \\ 38 \\ 39 \\ 40 $. 727 . 754 . 781 . 810 . 839	1. 963 2. 006 2. 050 2. 097 2. 145	3. \$52 4. 023 4. 204 4. 395 4. 599	. 510 . 499 . 488 . 477 . 466	. 260 . 249 . 238 . 228 . 217				
$ \begin{array}{r} 41 \\ 42 \\ 43 \\ 44 \\ 45 \end{array} $. 869 . 900 . 933 . 966 1. 000	2. 194 2. 246 2. 300 2. 358 2. 414	4, 815 5, 045 5, 289 5, 550 5, 828	. 456 . 445 . 435 . 424 . 414	. 208 . 198 . 189 . 180 . 172				

Active and passive pressures.—Formulas for finding the lateral pressures of soils against retaining walls were published in PUBLIC ROADS, December 1938 (δ). For the simplest case of a cohesive soil with level backfill, vertical back of wall, no surcharge, and swelling phenomena neglected, the total active horizontal pressure, L, per unit length of wall is obtained from the expression

in which

h =height of backfill, and

 $L = h \left(\frac{wh}{2} \tan^2 \beta - 2c \tan \beta \right)$ (5)

$$\beta = 45^{\circ} - \frac{9}{2}$$



FIGURE 3.—SURFACES OF SLIP BEHIND WALLS.

For the same conditions, the total passive earth pressure, P, per unit length of wall is given by the formula

The significance of the terms "active" and "passive" earth pressure has been described in PUBLIC ROADS (3) as follows:

In the design of retaining walls, three types of earth pressure may be considered.

Without movement of the earth, pressures against the walls, figures 3-A and 3-B, become the "earth pressures at rest" which depend upon the coefficient K, expressed by the relation: K=l/v. (K depends on the soil's elasticity.)

When depend upon the content K, expressed by the relation. K=l/v. (K depends on the soil's elasticity.) However, soil must deform to fail. •The pressures it produces at maximum deformation without failure are termed active or passive, depending on the directions of the applied forces responsible.

Wedges assumed in the design of retaining walls, figure 3, have lower boundaries, D–D, on which the soil slips when it shears. Weight of the earth in figure 3–A produces the active earth pressure which forces walls outward and causes D–D to incline at an angle α with the horizontal and β with the vertical. Forcing walls backward as in figure 3–B produces the passive earth pressure which causes D–D to incline at an angle β with the horizontal and α with the vertical.

A cable anchorage would exert passive pressure on the soil in front of it. The surface shear test apparatus developed by Burggraf (7) measures a similar passive resistance.

FORMULAS GIVEN FOR BEARING CAPACITY AND DISTORTION OF SOILS

Bearing capacity under strip load.—It will be noted that equations 5 and 6 have two parts. The first depends on the weight of the earth in the wedge and the second on the cohesion. As shown in figure 4-A, the bearing capacity, q, of soil under a long, uniform, strip load depends on an active wedge being held in equilibrium by a passive wedge. Since the passive pressure, P, is always greater than the active pressure, L, an additional pressure, q, can be supported at the surface of the active wedge. Then at equilibrium

$$q = \left(\frac{P-L}{h}\right) \tan^2 \alpha \tag{7}$$

By substituting equations 5 and 6 for L and P in equation 7 there is obtained

$$q = w_f H = c \frac{2 \tan \alpha}{\cos^2 \alpha} + w_u B \frac{\tan^4 \alpha - 1}{2 \cot \alpha} \dots (8)$$

in which

 w_f =unit weight of embankment material, H=critical height of embankment, w_u =unit weight of undersoil, and B=width of active wedge at the subgrade surface, figure 4-A.



FIGURE 4.—SURFACES OF SLIP UNDER EMBANKMENTS ($\phi = 10^\circ$).

If a surcharge of thickness T is applied to the surface of the passive wedge, figure 5-A, the load on the active wedge may be increased by $w_s T \tan^4 \alpha$, where w_s is the unit weight of the surcharge material. The total bearing capacity (8) then becomes

$$q = w_f H = c \frac{2 \tan \alpha}{\cos^2 \alpha} + w_u B \frac{\tan^4 \alpha - 1}{2 \cot \alpha} + w_s T \tan^4 \alpha_-(9)$$

For zero cohesion, the first term is zero; for zero friction, the second term is zero; and for zero surcharge, the third term is zero. If B is taken as zero in equation 9, the bearing capacity becomes equivalent to

$$q=2c (\tan^3 \alpha + \tan \alpha) + w_s T \tan^4 \alpha_{----}(10)$$

which gives the maximum allowable vertical pressure under the edge of a footing (\mathcal{G}) . Equation 9 has been suggested for use in estimating the supporting value of a homogeneous subgrade under a symmetrical strip load which divides in the center as it fails.

To apply the formula to a long fill, the cross section of the fill must be modified. With reference to figure 5, it should be noted that B is the width of the active wedge whereas b is one-half the top width of the fill. One method of solution is to assume a rectangular cross section of width 2B and an area equal to the area of the fill as shown by figure 5–B. This solution considers no surcharge. Another method is to assume a rectangle of width 2B with equal surcharges on each side as shown by figure 5–C. The area of the rectangle plus the area of the surcharges is equal to the actual area of the fill cross section.

If the embankment can be considered rigid enough to settle as a unit and the undersoil moves out on one side only as in figure 4–B, the full width, 2B, is used in place of B and equation 9 becomes

$$q = c \, \frac{2 \, \tan \, \alpha}{\cos^2 \, \alpha} + w_u B \, \frac{\tan^4 \, \alpha - 1}{\cot \, \alpha} + w_s T \, \tan^4 \, \alpha_{----} (11)$$

A different problem is presented by a fill, figure 4–C, which is rigid enough to settle vertically without tilting and without breaking in the middle, forcing the undersoil out on both sides. Prandtl's formula with a term $\frac{261154-40-2}{2}$



FIGURE 5.--- MODIFIED EMBANKMENT CROSS SECTIONS.

added to include the weight of the supporting soil (10) may be used to find the supporting power of the undersoil under these circumstances. The formula considers no surcharge and is

$$q = (c \cot \phi + w_u B \tan \alpha) (\tan^2 \alpha \times e^{\pi t a n \phi} - 1) (12)$$

Distortion of soil.—If the amount of distortion in the soil behind a retaining wall were relatively the same as in a shear sample and the wedge of soil behind a rotating wall deformed in pure shear parallel to the plane of failure, figure 6, the average movement of the top of the soil wedge h feet high as the wall rotates about its base (4) would be:

For walls moving out (active pressure), figure $6 - \Lambda$.

$$d_{l} = \frac{mh}{200} \sin^{2} \beta \text{ feet} = 0.03mh(1 - \sin \phi) \text{ inches}_{--}(13)$$
$$d_{v} = \frac{mh}{200} \sin \beta \cos \beta \text{ feet} = 0.03mh \cos \phi \text{ inches}_{--}(14)$$

For walls moving in (passive pressure), figure 6–B.

$$d_{l} = \frac{mh}{200} \cos^{2} \beta \text{ feet} = 0.03mh(1 + \sin \phi) \text{ inches}_{--} (15)$$

$$d_v = \frac{mm}{200} \sin\beta\cos\beta \text{ feet} = 0.03mh\cos\phi \text{ inches}_{--}(16)$$

where

 d_i = average lateral soil movement, d_i = average vertical soil movement, and

$$m = \text{shear strain in percent} = \frac{\text{stangential instance}}{\text{thickness}} \times 100$$

In a direct shear test, the shear strain may be taken as the shear movement divided by the effective thickness of the sample. For constant volume and small strains the maximum shear strain in a uniformly stressed cylinder is 1.5 times the vertical strain (11). The average



FIGURE 6.- DISTORTION OF SOIL BEHIND WALL.

settlement due to lateral movement of the soil supporting a long nonrigid embankment may be estimated by using B, the half width of the fill, in place of h in equation 15.

Bearing capacity of thin layer.—For a plastic material squeezed between two parallel rigid plates, the theory of plasticity indicates that the bearing capacity varies directly as the shearing resistance and as the ratio of the width of the plates to the distance between them if this ratio is at least four (12, 13). For the case of a triangular load of relatively firm material resting on a soft layer which is underlaid by relatively firm material, figure 7–A, the expression for the unit bearing capacity, q, of the soft undersoil is

$$q = W_f H_0 = s \frac{x}{\overline{D}} \tag{17}$$

where

 $w_f =$ unit weight of fill material,

 H_0 = height of triangular fill,

s = unit shearing resistance of soft layer,

x = width of fill at base, and

D =depth of soft layer.

By assuming a triangular embankment, figure 7–A, and substituting $c + \frac{w_u D}{2} \tan \phi$ (that is, $c + n \tan \phi$ of

the soft undersoil) for s and $2H_0S_0$ for x, a formula was derived for calculating the critical slope, S_0 , of the embankment. The expression thus obtained is

$$S_0 = \frac{Dw_f}{2c + Dw_u \tan \phi}$$
(18)

in which

 $w_u =$ unit weight of soft undersoil.

The critical slope of a trapezoidal fill may be estimated from equation 18 by considering a triangle, figure 7– Λ , with the same base and slope as the embankment. In the case of low, wide fills, the results obtained by this procedure may be too conservative. Another method (13) is to consider a triangle with an area equal to the trapezoidal area of the fill as in figure 7–B. For this assumption, the relation between the slope, S, of the



FIGURE 7.-FILL ON SOFT LAYER.

fill and the slope, S_0 , of the triangle is

$$S_0 = S + \frac{\left(\frac{b}{H}\right)^2}{2\frac{b}{H} + S}$$
(19)

where

b = top half-width of fill, andH = height of fill.

FORMULA FOR GREATEST SHEARING STRESS UNDER FILL PRESENTED

Critical height of slopes.—A graphical method for determining the critical height of slopes was published in PUBLIC ROADS, December 1929 (6). It assumes a circular surface of sliding as shown in figure 8 and compares the moment of the shear resistance along this surface with the moment of the weight of soil bounded by the surface. Moments are taken about the center of curvature. The most dangerous circle is determined by trial. Various analyses of the critical height, H, of cuts and embankments of homogenous material with level tops have been compared and tabulated by Taylor

(14). His tables give the dimensionless ratio $\frac{c}{wH}$ for various values of ϕ and slope angle *i*. For a vertical slope, the critical height becomes approximately $\frac{3.83c}{2}$

tan α which is greater than that given by equation 4, based on different assumptions.

Greatest shear stress under fill.—Applying the theory of elasticity to a semi-infinite, homogeneous, isotropic material, it is found that the greatest shear stress, s_{σ} , under a symmetrical, trapezoidal strip load, figure 9, is on the centerline and is expressed by the formula (15, 5)

where



FIGURE 8.—SLIDING SURFACE IN HOMOGENEOUS SLOPE.



FIGURE 9 .- TRAPEZOIDAL LOAD ON ELASTIC MATERIAL.

- z =depth below the surface, chosen to make s_{g} a maximum,
- p =pressure on centerline at the surface of the supporting material,
- a = width of one side slope of trapezoidal fill,
- b = half width of fill at top, and

$$B=b+\frac{a}{2}=z$$
 (approximately).

For the special case of a rectangular strip load, a=0, the shear stress is maximum and equal to p/π at all points on the circumference of a semicircle passing through the edge of the load. If there is a rigid layer at some depth below the load, then according to D. L. Holl (16), the greatest shear stress is nearer the surface and of greater magnitude than for a homogeneous supporting material of infinite depth.

The load producing a greatest shearing stress equal to the cohesion of the supporting soil is less than the



FIGURE 10.—GREATEST STRESSES DUE TO UNIFORM CIRCULAR LOAD ON A SEMI-INFINITE SOLID FOR DIFFERENT POISSON'S RATIOS.

ultimate bearing capacity, computed for conditions of failure. That is, the load which causes failure at a single point or localized region is less than the load which will cause total failure throughout the supporting soil. Design based on stresses causing failure at restricted regions under a finite area is illustrated by considering the stresses under a circular load.

Stresses under loaded circular areas.-A complete analysis of the stresses below a uniformly loaded circular area, using the theory of elasticity, has been presented by Love (17, 18) and includes a tribulation of stresses for Poisson's ratio equal to one-four h. This analysis was discussed in PUBLIC ROADS (19) Figure 10 shows the location and magnitude of the greatest shear stress and the corresponding major principal stress at any level in the undersoil. The influence of Poisson's ratio on these stresses is also shown. The greatest shear stress anywhere under a uniformly loaded circular area is at the surface, just beneath the perimeter. For a Poisson's ratio of one-half, the greatest shearing stress is on the axis for all depths where z/r is greater than 0.7. Broken lines in figure 10 were interpolated. Figure 11 shows the effect upon the greatest shearing stress at any level of varying the applied load when the total load or the area of the circle or the unit pressure is kept



FIGURE 11 .- GREATEST SHEAR STRESSES DUE TO UNIFORM CIRCULAR LOAD ON A SEMI-INFINITE SOLID.



FIGURE 12.-Relation of Strength to Stresses at a Point.

constant. For ϕ equal to zero and Poisson's ratio equal to one-half, a point at the edge of the load is overstressed if the unit load exceeds 3.14c (see fig. 10) whereas, according to H. Hencky (12), the ultimate bearing capacity under a rigid circular load is not reached until the average unit pressure is 5.64c.

To determine the cohesion required to prevent overstress at any point at a given level below a uniformly loaded circular area in a material wherein ϕ is greater than zero, account must be taken of the weight, w, of the material and the normal stresses produced by the load (20). Mohr's diagram, figure 12, shows the cohesion required to prevent overstressing at any point and was used in deriving the formula,

where

$$F = \frac{s_{\text{max}}}{n \cos \phi} - \left(\frac{p_1 - s_{\text{max}}}{n}\right) \tan \phi$$

in which s_{\max} and p_1 are the stresses s_q and p_1 shown in

 $c = nF - wz \tan \phi$

figure 10 except for certain values of ϕ for which stresses

at points off the axis required the greatest cohesion. This analysis may be useful as a qualitative indica-tion of the shearing strength required in flexible pavements and subgrades under pneumatic tire loads. However, as in other soil stability problems, due consideration must be given to such factors as wetting and drying, freezing and thawing, swelling and consolidation, distortion, and nonuniformity.

CHARTS USED TO FACILITATE COMPUTATIONS

The charts used in the solution of the foregoing formulas are of the simplest types and, in general, permit the determination of any one variable if the others are given (21). Supplementing the method of constructing each chart is an illustrative example which demonstrates its use.

Principal stresses at failure.—If equal lateral and vertical pressures are applied to a right circular cylinder and the vertical pressure then increased to failure, these stresses are related to c and ϕ by equation 3. This formula may be used to determine either the major principal stress, v, or the minor principal stress, l. To construct the graph, each term was evaluated separately. Thus, in the left chart of figure 13, $\tan \alpha$ is plotted on the vertical axis, marked with the corresponding values of ϕ , and is multiplied by the sloping lines for various values of c to determine $2c \tan \alpha$ on the horizontal axis. On the right chart $\tan^2 \alpha$ is plotted on the vertical scale. An illustrative problem in which c, ϕ , and l are given is shown on the figure.



(21)

FIGURE 13 .- PRINCIPAL STRESSES AT FAILURE.



STRAIGHT BROKEN LINES ARE FOR EXAMPLE :- BY PUBLIC ROADS FORMULA FOR $\oint =10^{\circ}$ (same for each q) and c=500 LB. PER SQ.FT., () Gives $q_c = 2,900$ LB. PER SQ.FT. FOR B = 40 FT. AND $w_{it} = 100$ LB. PER CU.FT., $w_{it}B = 4,000$ LB. PER SQ.FT., (2) GIVES $q_{jt} = 2,400$ LB. PER SQ.FT. FOR T = 5 FT. AND $w_{it} = 120$ LB. PER CU.FT., $w_{it}B = 4,000$ LB. PER SQ.FT., (3) GIVES $q_{jt} = 1,200$ LB. PER SQ.FT. THEN $q = q_c + q_B + q_T = 6,500$ LB. PER SQ.FT. $w_f H$ (EQUILIBRIUM); FOR $w_f = 130$ LB. PER CU.FT., (2) H = 50 FT.



To solve for v, start at $\phi = 20^{\circ}$ and follow the dotted lines to obtain the two terms of equation 3 which are added to determine v. The necessary additions or subtractions as well as the reading of the charts may be done with the aid of temporary marks on a straightedge.

Equation 5 may be rewritten in the form

$$\frac{wh}{2} = 2c \tan \alpha + \frac{L}{h} \tan^2 \alpha$$

The active horizontal thrust, L, against a retaining wall may then be determined from figure 13 by using $\frac{wh}{2}$ for v and $\frac{L}{h}$ for l.

As an example, find L for a wall 20 feet high; given w=100 pounds per cubic foot, c=200 pounds per square foot, $\phi=10^{\circ}, \frac{wh}{2} = \frac{100 \times 20}{2} = 1,000$ pounds, per square foot.

From the left chart, $2c \tan \alpha = 480$ pounds per square foot. Then $\frac{L}{h} \tan^2 \alpha = 1,000 - 480 = 520$ pounds per square foot.

From the right-hand chart for $\tan^2 \alpha = 520$ and $\phi = 10^{\circ}, \frac{L}{h} = 370$ pounds per square foot.

Thus L=7.400 pounds per foot of wall.

Similarly, the passive pressure, P, back of a retaining wall as expressed by equation 6, may be obtained from figure 13 by substituting P/h for v and wh/2 for l. A nomograph of the general formula for pressures on a wall with a cohesionless backfill has been published by Taylor (22).

Bearing capacity of soil under long fill.—Figure 14 may used to compute the bearing capacity of a homogeneous subgrade under a symmetrical strip load The figure solves equations 9, 11, and 12 by dividing the total bearing capacity, q, into components— q_c involving c; q_B involving the pressure in the supporting soil, $w_u B_i$; and q_T involving the surcharge pressure $w_s T$. The total bearing capacity is the sum of the components. The lower-left chart gives the value of a function of ϕ and the upper-left chart multiplies this value by the appropriate values of c, $w_u B$, or $w_s T$. The values of w_u , w_s , and w_f which are selected should represent the most unfavorable conditions to be anticipated.

The illustrative example shown on figure 14 considers a nonrigid fill with surcharges for which c, ϕ, w_u, w_s , T, w_f , and B are given. In the problem H is solved for by means of equation 9.

Values for a multiplying factor not on the chart such as c equals 50 may be determined by using the line for c

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FIGURE 15.-SOIL MOVEMENT BEHIND WALL.

equals x times 50 and dividing the resulting q_c by x. This same device may also be used for $w_u B$, $w_s T$, H, w_f , or similar factors on other charts.

Soil movement behind wall .-- The movement of the faces of the wedge of soil behind a rotating wall as given by equations 13 to 16 may be determined from figure 15. The middle chart multiplies m by h, and the left and right charts multiply this product by the appropriate functions of ϕ . The arrows and broken lines indicate the use of the chart in an example to solve for the average lateral and vertical movements of the soil of height h for a given shear strain or deformation and the corresponding ϕ .

Critical slope of fill on soft layer.—Figure 16 may be used in solving equations 18 and 19. The three terms of equation 18 as rearranged on figure 16 are solved separately and must be added or subtracted as required. The lower-left chart is used for obtaining w_f/S_0 and $w_u \tan \phi$. The right-hand chart divides 2c by D. The small chart in the upper left is used for obtaining S_0 , the slope of the triangle, when the equal area method is used.

In the illustrative example, b, H, S, w_f , w_u , ϕ , and D are given and the c required for equilibrium is to be found. Using method I, enter the upper-left chart with b/H and S to get S_0 . The lower chart divides w_f by S_0 and multiplies w_u by $\tan \phi$. Next enter the right-hand chart with 2c/D (which equals $w_f/S_0 - w_u \tan \phi$), and with the given D find the required c. One may solve for S, if the other factors are known, by going through the about in the reverse direction. If the closer through the chart in the reverse direction. If the slopes of the fill are assumed to be continued to form a triangle as in method II, $S = S_0$ and the upper-left chart may be disregarded.

Critical height of slopes.—The lower-left chart of figure 17 was constructed from Taylor's table of values of $\frac{c}{wH}$ for various values of slope angle and ϕ . The abscissas in the lower-left chart are $\frac{wH}{c}$ and the ordinates of the upper charts are wH. The curved lines in the lower-left chart are for circles through the toe of the slope when a more dangerous circle exists which passes below the toe.

In the first example, find H when S is $1\frac{1}{2}$: 1, $\phi = 7.6^{\circ}$, c=100 pounds per square foot, and w=100 pounds per cubic foot. By following the broken lines marked 1, H is found to be 10 feet. If H is 20 feet, one can go through the chart in reverse direction and find that the critical slope is 3.4:1.

The critical height of cohesionless materials is, according to this analysis, zero for slopes greater than ϕ and unlimited for slopes less than ϕ .

Greatest shear stress under long fill.- Equation 20 was solved for variously proportioned trapezoids and figure 18 was constructed for calculating the greatest shear stress produced by a given fill pressure. The

left chart determines $\frac{s_{\sigma}}{p}$ for given ratios of b/B. The

right-hand chart multiples $\frac{s_c}{p}$ by p. Cohesion required under uniform circular load.— Figure 19 may be used to determine F in equation 21. Values of s_G/p and p_1/p for substitution in the expression for F were taken from figure 10 for Poisson's ratio equals one-half. Broken lines on figure 19 represent values of F which were interpolated between values on the axis and values at the surface.

As an example of the use of figure 19, take p=8,000pounds per square foot, r=8 inches, w=100 pounds per cubic foot, and $\phi=10^{\circ}$. Then, to determine the required c at a depth of 16 inches, enter the chart at



obtain $c=8,000\times0.11-100\times16\times0.015=880-24=856$ Tan ϕ is used to showed

pounds per square foot. $\frac{1}{12}$ is used to change z from inches to feet.

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FIGURE 17 .- CHART FOR CRITICAL HEIGHT OF SLOPES.

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FIGURE 19.-GRAPH OF FACTOR FOR DETERMINING COHESION REQUIRED IN SOLID SUPPORTING A UNIFORM CIRCULAR LOAD.



FIGURE 5.—PERCENTAGE OF ALL RURAL DWELLINGS WITHIN VARIOUS TRAVEL DISTANCES OF SURFACED ROADS IN EACH OF 10 STATES.

The percentage of all rural dwellings within 1 mile of a surfaced road ranged from 94.9 percent in Maryland, down to 42.2 percent in North Dakota.

Table 9 and figure 6 show, for the same 10 States, the percentage of all rural dwellings within various travel distances of either a surfaced road or a graded and drained road or, in other words, of an improved road. In these States, 80.9 percent of the rural dwellings were within 1 mile of an improved road. In Maryland, 99.1 percent of the rural dwellings were within 1 mile of an improved road and all of them were within 2 miles of an improved road. In Texas, on the other hand, the percentages of the rural dwellings within 1 mile and within 2 miles of improved roads were 62.0 percent and 73.9 percent, respectively. In all of the States for which the information was obtained, only a very small percentage of the rural residents need travel more than 1 or 2 miles from their homes to reach an improved road.

 TABLE 8.—Percentage of all rural dwellings within various travel
 distances of surfaced roads, in each of 10 States

State	Within	Within	Within	Within	Within
	1 mile	2 miles	3 miles	4 miles	5 miles
Arizona.	$77.1 \\72.6 \\81.1 \\94.9 \\70.7$	80. 4	82. 1	83. 1	84. 2
Florida.		80. 5	85. 6	89. 1	91. 4
Idaho		87. 3	90. 2	92. 3	93. 8
Maryland.		98. 9	99. 7	99. 9	100. 0
Missouri		85. 4	92. 7	96. 6	98. 4
Nebraska North Dakota South Dakota. Texas Utah	$54.0 \\ 42.2 \\ 57.8 \\ 54.6 \\ 85.7$	74. 264. 375. 066. 689. 9	$\begin{array}{c} 83.\ 4\\ 77.\ 7\\ 81.\ 4\\ 75.\ 0\\ 92.\ 1\end{array}$	88.5 85.3 84.9 81.2 93.3	91.5 90.1 87.0 85.8 94.4
A verage	65.0	77.5	84.4	88.7	91.5

In this report, all of the rural roads within a State have been considered as constituting a single system of



FIGURE 6.—PERCENTAGE OF ALL RURAL DWELLINGS WITHIN VARIOUS TRAVEL DISTANCES OF IMPROVED ROADS IN EACH OF 10 STATES.

roads. This is the concept of the average motorist since he generally does not know or care what administrative system or systems of roads he travels over in driving from one place to another. To those charged with the responsibility of financing, building, and maintaining roads, however, distinction between administrative systems is of the utmost importance. In nearly all States one group of public officials is responsible for State roads and other groups for county roads or township roads. The Federal Government has assumed responsibilities with respect to the 7 percent Federal-aid system, the 10 percent Federal-aid secondary system, and several systems or groups of roads serving national reservations, such as national forest highways, national forest development roads, national park roads, Indian roads, roads through public lands, and recently, roads of major importance from the standpoint of national defense. The sources and amounts of funds made available for each administrative system are responsibilities of legislative bodies. The segregation of road mileages into administrative systems is complicated because of overlapping juris-

 TABLE 9.—Percentage of all rural dwellings within various travel distances of improved roads, in each of 10 States 1

State	Within 1 mile	Within 2 miles	Within 3 miles	Within 4 miles	Within 5 miles
Arizona Florida. Idaho. Maryland Missouri	86.7 99.1 89.1 99.1 99.1 96.9	89.5 99.6 92.8 100.0 99.1	90. 9 99. 7 94. 5 100. 0 99. 6	91. 8 99. 8 95. 5 100. 0 99. 8	92. 4 99. 8 96. 3 100. 0
Nebraska North Dakota	66. 9 72. 8 94. 3 62. 0 94. 3	83. 9 87. 9 97. 8 73. 9 96. 3	90. 6 93. 8 98. 8 81. 9 97. 2	93. 8 96. 4 99. 2 87. 5 97. 6	95, 5 97, 8 99, 4 91, 3 98, 0
Average	80. 9	88.2	92.1	94.5	96.1

¹ Improved roads include graded and drained roads and surfaced roads.

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dictions. Also, significant comparisons of systems between States are difficult because of differences in the extent of the responsibilities of different jurisdictions. For example, in several States the State government assumes responsibility for all rural public roads, in others the responsibility is divided between the State and the counties, and in still others it is divided between the State, counties, and townships.

A complete and detailed analysis of roads by administrative system is being made in each State. For the

REGULAR FEDERAL-AID FUNDS AUTHORIZED FOR 1942 AND 1943

The Federal Highway Act of 1940, which authorizes regular Federal-aid funds for highways, secondary or feeder roads, and grade crossings for the fiscal years 1942 and 1943, was approved on September 5, 1940. The act is in conformity with the congressional policy of authorizing in advance of the period for which they are available the Federal-aid funds for 2 years, enabling the various State legislatures, many of which meet biennially, to plan their highway budgets with foreknowledge of their approximate Federal-aid apportionments. Federal funds for other classes of road work are also provided by the act, the amounts provided for each fiscal year being as follows:

Item	Amount for each fiscal year
Federal-aid system	\$100, 000, 000 17, 500, 000 20, 000, 000 7, 000, 000 3, 000, 000 4, 000, 000 7, 500, 000 1, 500, 000 3, 000, 000

As in previous years, the Federal-aid highway and secondary road funds must be matched with State funds, and the grade crossing funds are outright grants to the States. Funds for these three classes of work for the fiscal year 1942 are required by law to be apportioned to the States, the District of Columbia, Hawaii, and Puerto Rico, by the Federal Works Administrator before next January 1. Formulas for reasons cited, such an analysis does not lend itself well to presentation on a national basis. Significant nationwide comparisons can be made, however, for the 7 percent Federal-aid system, for a group of the most important roads in each State designated as State highways in some and as primary State highways in others, and for all other rural roads regardless of jurisdiction which are mainly local roads in the sense that interest in them is not State-wide. Such comparisons will be made in a subsequent article.

apportioning the funds among the States remain unchanged.

Section 12 of the act specifically authorizes the Reconstruction Finance Corporation "to cooperate with States to finance, or to aid in financing, the acquisition of real property or interests in property * * * necessary or desirable for road projects eligible for Federal aid under the Federal Highway Act * * *." This provision will enable the long-term financing of highway rights-of-way through cities, thereby facilitating the early completion of necessary improvements that heretofore have not been undertaken because of the lack of sufficient current funds to pay both right-ofway and construction costs. High right-of-way costs, in many cases amounting to several times the actual construction costs, have retarded improvements to main routes through cities needed to eliminate traffic congestion and attendant danger and delay.

Section 19 of the act provides that: "In approving Federal-aid highway projects to be carried out with any unobligated funds apportioned to any State, the Commissioner of Public Roads may give priority of approval to, and expedite the construction of, projects that are recommended by the appropriate Federal defense agency as important to the national defense."

Under this provision of the law it should be possible to make an immediate beginning on the strategic highway program. A system of 75,000 miles of main highways has been selected by military and naval authorities as highly important for definite strategic reasons. Many sections of the system are already in satisfactory condition but there are also numerous substandard sections. Replacing weak bridges and widening and strengthening road surfaces and shoulders will be important parts of the work. The program is aimed at the elimination of critical weaknesses and restrictions on main highways.

	02	STATUS C	DF FEL	ERAL-AI	D HIGHV	VAY PI	ROJECTS			
		A	S OF	SEPTEM	BER 30,	1940				
STATE	COMPLETED DU	JRING CURRENT FISCA	L YEAR	IGNU	ER CONSTRUCTION		APPROVEI	D FOR CONSTRUCTIO	z	BALANCE OF FUNDS AVAIL- ABLE FOR PRO-
	Estimated Total Cost	Federal Aid	Miles	Estimated Total Cost	Federal Aid	Miles	Estimated Total Cost	Federal Aid	Miles	GRAMMED PROJ- ECTS
lab ama rizona rkunsas	\$ 1,705,324 550,885 2,503,775	* 847,917 390,645 1,121,535	49.1 29.9	* 4,642,001 1,147,222 2,681,494	* 2, 308.053 712, 144	164.0 141.2 128.0	* 1.837.290 507.140 381.484	* 914,440 275,016 189,448	76.1 32.0 22.1	* 2,166,993 1,225,492
alifornia olorado onnecticut	3,129,803 1,171,028 15,000	1,630,057 658,169 7,500	108.0	8,035,542 2,227,256 2,706,4448	4,150.579 1.237.574	128.0 114.6	2.085.955 513.621 261 250	1,138,550 289,477 120,573	43.9 77.6 1.7	1,855,907 2,173,268
elaware lorida eorgia	52,661 732,053 875,168	26, 330 366, 026 437, 584	-9 	1,969,981 3,232,131 7,396,295	984, 211 1,605,786 3,682,230	31.6 36.8 316.4	155,407 840,971 3,727,435	51,044 1,20,485 1,864,217	17.14 167.6	1,048,125 2,313,581 4,685,263
labo linois idiana	518,918 2,041,318 2,242,510	317,895 999,099 1.121,255	52.6 16.1 58.9	1, 393, 325 9, 425, 200 6, 537, 321	856,177 4,711,705 3,118,504	122.3 196.9 112.8	378,325 1,869,392 3,461,808	162,646 873,696 1,698,749	17.7 62.7 52.1	1,581,336 2,700,572 884,241
wa ansas entucky	1,985,091 601,220 1,233,766	879, 440 300, 610 615, 327	53.5 38.4	5,463,365 7,516,257 3,948,134	2.508.062 3.757.784 1.974.067	195.4 149.6 108.0	2,130,594 2,866,854 661,649	1,000,950 1,433,426 330,825	71.2 256.3 53.5	401,368 3,236,143 2,893,632
ouisiana laine laryland	643,730 750,620 345,000	316,311 374,022 172,500	15.8 10.1	12,720,814 1,068,408 3,665,830	3,405,451 534,203 1,824,197	64.7 29.5 40.2	1,170,213 243,710 489,303	577, 418 121,855 244,651	27.3 5.0	2,863,497 604,532 1,237,233
lassachusetts lichigan innesota	553,156 1,085,862 2,067,822	276, 153 542, 931 983, 419	37-9	3,672,471 11,593,150 7,522,270	1,827,381 5,708,474 3,748,500	31.0	177,871 1,623,316 803,941	88.935 794.758 401.971	50.1 29.5	2,836,471 288,080 3,155,788
lississippi lissouri ontana	763,732 1,880,717 2,658,568	305, 416 940, 354 1, 506, 079	2075 2015 2017	7,013,274 7,596,672 2,460,575	3, 141, 143 3, 477, 199 1, 390, 039	335.3 206.1 149.2	1,943,060 3,269,330 880,862	948,730 1,202,672 483,552	112.5 112.8 148.8	1, 454, 294 3,939,547 3,470,304
ebraska levada ew Hampshire	2, 464, 333 1, 196, 352 487, 278	1,232,166 1,029,857 238,155	298.2 60.9	5,384,664 1,374,094 1,361,162	2,604,315 1,194,076 668,546	619.3 62.1 32.9	1,236,985	618,492 4,881 7,004	150.9	2, 362, 913 759, 186 899, 329
iew Jersey iew Mexico iew York	1,089,800 1,133,944 2,008,626	5 ⁴⁴ , 670 700, 925 992, 285	8.9 89.6	4,416,148 1,480,152 16,742,726	2,208,074 900,375 8,187,377	32.9 88.3 242.3	1.295,710 622,420 4.096,017	647.855 375.195 1.519.684	46.0 46.0	1,586,676 1,124,863 925,195
orth Carolina orth Dakota hio	2,782,105 1,055,750 1,805,773	1,390,055 565,838 902,886	130.5 101.5 23.8	3.891.121 2.684,408 13.637,102	1,946,132 1,509,122 6,794,222	187.3 211.0 124.4	2, 251,000 3,169,464 2,953,710	1,102,970 1,621,950 1,476,580	104.1 269.1 24.9	1,321,066 3,197,318 4,281,168
klahoma regon ennsylvania	866, 292 1,513, 189 1,977, 624	459,550 909,630 961,473	50.0 108.9 26.8	3,486,033 2,724,902 12,841,490	1,843,453 1,619,495 6,377,960	112.9 56.4 127.4	1,395,085 1,311,660 3,564,299	686,614 602,750 1,774,179	50.9 30.2 29.2	4,092,767 871,464 1,868,526
hode Island outh Carolina outh Dakota	292,212 518,801 1,557,814	145,890 241,980 881,564	2.5 36.5 235.7	1,457,124 2,605,266 4,338,480	727,160 1,261,281 2,567,480	15.1 176.5 582.6	300,411 933,689 1,239,140	150,180 1,16,210 749,990	1.9 69.7 225.2	852,533 2,139,230 2,448,861
ennessee exas tah	524,608 3,321,834 412,082	262, 304 1, 636, 523 300, 075	231.2 32.5	3,646,358 7,788,195 1,237,235	1,823,179 3,861,187 918,263	106.6 364.4	1.547.042 3.965.998 677.430	1,878,790 276,330	61.6 172.2 26.3	3.708,269 6.033,154 733,481
ermont irginia fashington	214,830 1,288,200 1,678,909	107,415 597,453 884,294	11.1 41.1	1.499,414 2.556,897 2.990,562	748,058 1,236,647 1,582,235	40.7 62.5 69.1	320,628 2,117,568 776,697	1.040.995 1.040.995 413.400	5.7 27.1 12.4	263,842 1,204,406 754,323
/est Virginia /isconsin /yoming	507.150 3.142.906 1.290.783	252,395 1,533,610 814,104	142.2	3,531,880 3,791,451 1,003,016	1,757,741 1,883,380 638,987	107.3 158.3 108.1	666, 530 325, 553 536, 608	333, 265 153, 950 3422, 993	12.9 14.4 65.8	1.671.200 3.407.486 5.649.481
istrict of Columbia awaii uerto Rico	154,300 29,184 20,338	77,150	1.2	383,942 354,190 1.597,615	191,971 179,072 790,710	0.5.7 62	250,100 546,461 106,769	117,016 273,046 52,580	9.5 9.5	325,209 1,498,907 717,500
TOTALS	63.442.744	32,819,811	2,997.3	234,440,463	115,338,611	7,697.3	68,520,362	33,202,818	2.803.4	98,159,682

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STATUS OF FEDERAL-AID SECONDARY OR FEEDER ROAD PROJECTS

AS OF SEPTEMBER 30, 1940

COMPLETED DU	RING CURRENT FISCA	AL YEAR	IND	DER CONSTRUCTION		APPROVE	ED FOR CONSTRUCTIO	N	BALANCE OF
Estimated	Federal Aid	Miles	Estimated Total Cost	Federal Aid	Miles	Estimated	Federal Aid	Miles	FUNDS AVAIL- ABLE FOR PRO- GRAMMED PROJ-
LOIL COST			1 OCBI COSE			1 Otal Cost			FCIS
# 175.144	* 87.363	4.6	* 1,065,679	* 531.120	148.6	* 257,167	\$ 128,233	12.2	\$ 354,133
58,228	21,068	1.4	282,532	203,973	13.9				234,685
+C0,100	174,440		200 100	140.041	8.12	628.101	08.370	1.0	72, 539
600 1+6	006.001	6.01	100 1100	390,814	4.10	108, 742	109.04	1.	808,868
52.818	25.299	80.	317.216	154.114		102 386	104.04	0.1	165 662
69.537	37.768	7.8	98,995	41.350	7.7		n/++ n		268,125
12,030	6,015		108,106	204,053	F	537.863	243.458	19.8	175.237
10/ 25	19, 50	2.2	\$33,812 \$6	301.906	2.42	1900 881	1#, 565	1.24	1.039.418
204.12	36. 64	ne n	· 106.000	+) C + C	0.01	+00° 00+	106.00	0.01	424,001
Th6 670	121 021	010	105 102	00, 101		920 10	The own	20.02 11 11	016 007
1 170 075	561 025	201 1	1 050 522	712 001	1 - 1 - 1 - C	012.12	006 CT	0.910	160 646
C12, 211	200 000	1.1.7	300 000	010.664	1.0 21	1001,100	Jec, 000	10.01	100,601
EA6 671	110,990		C16.071	00(*)00	0.04	1420,080	C4C, (89	2.0.	101 100 101 1
110,000	013 VG	C 2	266 202	120 001	0.00	-11 * 00C	161+66	2.01	100 201
115 010	210,02	1.0	TRO ZIL	gu pha	0 1	20,000	1h Em	1 1	TICU X
010*611	011.00		177 006	03° C-0	-0.0	000°63	mc*+1		120.00
110 536	FLL RED	2.7	521 271	258 386	10.7				LOK 612
126 Elle	120 10	1 66	1 1156 220	721 551	0 211	oli ono	W1 661	10 5	511 1/11
100 545	12.00	19.7	753 335	375 951	118.6	56g 703	281 152	2.01	SUE OFO
172 962	se her	10.6	671 150	900 022	21 12		105 665	10.8	GET HOH
206.701	101 201	0.00	Enn zilo	210 700	1.7.0	oilo ilzil	120 011		666 220
LES 768	PEO LESK	61.7	Pho 708	110 060	0.01	55 560	12 566	2.1	644 158
328.695	164.188	53.7	660.960	324, 496	89.0	147.879	71.940	24.4	231.413
151.328	130.622	33.6	126.614	104.928	12.9	10.501	35.310	2.1	55.950
48.387	23.241	2.2	97.444	46.738	2.1				167.812
319.500	159.750	10.6	264,090	131.965	0.6	61.333	30,660	2.4	6111 661
101.564	63.386	13.1	396.987	244.848	14.7	273.612	100.232	14.1	85.190
981.201	190.601	10.8	2.065.875	093.860	54.2	366.640	149.143	11.4	72.269
L12.550	236.275	48.0	627.203	314.568	1.0H	23.680	10.500	8.	317.257
227 02	91 120	2	146 161	70 200		27 620	14 750	1 0	1 OIL FRE
L13 957	206.913	14.6	2.808.570	1.403.025	8.16	271.880	135.940	7.1	741.852
536.288	234.638	37.9	254.865	134.374	19.2	112.680	35.269	3.1	945.470
276.538	147.840	43.6	281.370	125.830	23.0	101.801	58.990	11.3	207.154
755.242	371.516	30.6	1.572.441	785.312	39.3	42,000	21.000		214.836
107.556	53.749	1.8	130.339	65,143	1.8				91,158
92,800	43.750	8.9	633, 123	236,366	72.2	188,647	80,250	50.0	193.595
			3,624	3,624	11				1,250,913
105,038	52.519	6-1	48,612	24,306	3.0	186,188	160.56	7.2	909,350
983,983	1483,876	138.4	665,963	328,010	20.6	392,750	181,475	26.5	845,653
3.767	2,500		240,082	151.040	25.9	31,200	23,000	5.7	121,152
139.900	48,305	5.4	366,408	103.750	14.0				22,500
168.540	80.212	0.6	1100.311	211.971	25.1	112,476	146,810	1.2	286,029
159.951	83.988	5.8	462.792	245.539	25.2	277.321	144,800	25.0	119.534
154.950	76.950	9.6	155,669	17.834	7.1	79.308	39.904	7.7	1116,556
180.214	89.751	2.5	758.070	378.910	27.7				649.380
349. 348	229.898	39.0	114,117	72,162	6-1				117,160
			.130,684	64,842	1.1				22,150
170.361	85.037	P.4	302,225	147.640	14.0	55.188	27.140	2.1	158.77
10 (CO 707	C 707 010	0 000 1	שני עני עבר	120 × 100		- 1.00 1.00			00 1.40 (a.
12,650,395	542°505°9	1,104.0	20,802,930	15,214,124	1,668.0	62th 96th 8	3.921.402	0*+11	50,481,6
	Future rest rest rest rest rest rest rest re	Factor of the contract for the contract of the contract	Fundament Federal Ald Milla Fundament Federal Ald Milla * 175, 1144 * 87, 355 9.4 \$31,054 Federal Ald Milla \$31,054 Federal Ald Milla \$31,054 153,938 9.4 \$31,054 153,938 10.5 \$25,818 25,299 .6 \$31,054 19,185 10.5 \$22,818 25,599 .6 \$25,597 4,1768 7.8 \$25,592 19,385 5.6 \$21,402 110,3997 5.6 \$22,592 144,8835 10.5 \$21,992 110,3997 5.7 \$21,992 110,3997 5.7 \$21,952 144,8355 10.5 \$11,0,517 250,164 7.6 \$21,952 144,8355 10.5 \$21,952 144,8355 10.5 \$21,952 144,637 20.14 \$22,952 144,8355 10.5 <	Total Line Faderal Ala Mate Faderal Ala Mate Fundamenta Faderal Ala Mate Faderal Ala Mate \$5,222 \$1,75,1144 \$5,1365 9,4 \$1,065,617 266,507 \$3,10,954 \$15,1365 9,4 \$1,106,617 266,507 269,507 \$3,10,954 \$15,1365 9,4 \$1,106,617 266,507 269,507 \$3,11,2615 \$16,14 \$1,65,14 \$11,219 266,507 269,505 \$2,616 \$16,14 \$1,6,14 \$1,17,216 269,507 269,505 \$2,616 \$15,16 \$2,17 \$26,507 269,507 269,507 \$2,616 \$2,17 \$26,517 \$26,517 199,105 269,517 \$2,616 \$2,166 \$2,17 \$26,517 199,102 2114,217 \$26,517 \$26,517 \$26,517 \$26,517 256,516 256,516 \$26,517 \$26,517 \$26,517 \$26,517 199,5165 256,516 \$26,517 \$26,517 <td>Torrelation Name Construction Name Construction 175, 114, 313, 522 Fallentian Name Fallentian Fallentian Fallentian 313, 522 21, 056 15, 114, 133, 054 15, 155 9, 4, 133, 054 15, 155 9, 14, 105 15, 11, 100 313, 522 25, 253 15, 955 16, 115 139, 126 14, 114 313, 525 15, 956 15, 955 15, 955 146, 114 15, 116 314, 570 11, 5, 101 5, 116 15, 55 146, 105 14, 114 115, 010 5, 116 11, 5 10, 55 11, 95, 102 11, 55 115, 010 5, 116 12, 112 115, 113 195, 102 11, 15 116, 556 11, 5, 11 166, 123 11, 55 11, 15 146, 105 115, 010 5, 116 17, 51 195, 102 11, 15 11, 55 115, 010 115, 114 115, 114 115, 116 115, 116 115, 010 115, 116 116, 114 116, 116</td> <td>Tenter is the intervence Anna Entimeter is the intervence Anna Anna</td> <td>Name Contract relation Name Contract relation Name Contract relation Name Name</td> <td>Construct Description <thdescription< th=""> <thdescription< th=""> <</thdescription<></thdescription<></td> <td>Manualization Name Nam Name Name</td>	Torrelation Name Construction Name Construction 175, 114, 313, 522 Fallentian Name Fallentian Fallentian Fallentian 313, 522 21, 056 15, 114, 133, 054 15, 155 9, 4, 133, 054 15, 155 9, 14, 105 15, 11, 100 313, 522 25, 253 15, 955 16, 115 139, 126 14, 114 313, 525 15, 956 15, 955 15, 955 146, 114 15, 116 314, 570 11, 5, 101 5, 116 15, 55 146, 105 14, 114 115, 010 5, 116 11, 5 10, 55 11, 95, 102 11, 55 115, 010 5, 116 12, 112 115, 113 195, 102 11, 15 116, 556 11, 5, 11 166, 123 11, 55 11, 15 146, 105 115, 010 5, 116 17, 51 195, 102 11, 15 11, 55 115, 010 115, 114 115, 114 115, 116 115, 116 115, 010 115, 116 116, 114 116, 116	Tenter is the intervence Anna Entimeter is the intervence Anna Anna	Name Contract relation Name Contract relation Name Contract relation Name Name	Construct Description Description <thdescription< th=""> <thdescription< th=""> <</thdescription<></thdescription<>	Manualization Name Nam Name Name

		DAT ANY OF	FUNDS AVAIL ABLE FOR PROGRAMMED PROJECTS	* 943,965 242,091	1,688,531 919,512 110,111	1, 290, 976	2,028,650	1, 217, 969	251,016 271,815	2,024,008 822,730 1.010,984	741, 349 973, 697 410, 875	115,071	1.207.397 482,206	852,601 741,485 2 307 021	1,890,452 357,935 3,359,718	95,913 927,058 740,832	1,715,788 2,006,230 276,632	223, 562 601, 096 1419, 464	1,141,717 1,358,033 193,577	150,010 292,509 414,657	47,933,356
	-		Grade Crossings Protect- ed by Signals or Other- wise	5000	51 -	- 54	292	2 ⁴		a 23 – 5	m-0	- 02 -	at te	20000	5 °	33	27	- 10 LC	ms	N	458
		UMBER	Grade Crossing Struc- tures Re- construct- ed	-		- 14	1	-		- m	N		- a	N	Q		N	~			23
	UCTION	NOTION I	Grade Crossings Elimitaaled by Separa- tion or Relocation	u		* *		- m	=-	-	- t	2 N	n n		n- σ	9	- 0	m-			79
CCTS	TONED FOR CONSTRA	OVED FOR CONSTR	Federal Aid	* 16,800 6,006	26,464 3,401	13.839 127.334 607 118	85,358 200,862 86,166	90, 456 205, 254	570, 153 128, 350	150.1 197.698	166,500 829,073 96,681	132,930 42,138 81,538	75,490	94,050 108,473 62% 380	197.357 5.790	174, 748 146, 585	159,203 70,450 76,260	34,546 608,649 135,862	8,220	464.6	8,935,593
G PROJI	dady	VILL	Estimated Total Cost	* 16,800 6,006	26, 464 3, 401	127.859	235, 161 235, 161	96,576 205,254	627,885 128,350 15,600	1034	1,057,603 99,948	132,930	75,490 189,836 283,405	94,050 132,270 664,157	200,773 5,790	175.150	159,203 70,450 76,367	34,546 608,649 135,862	8,220	464.6	9,366,242
SIN 940	-		Grade Grade Crossings Protect- ed by Signals or Other- wise			~ -	33	4-	- 0	00 10		50		995	n cu	50 EU	911		ñ		223
FEDERAL-AID GRADE CROS AS OF SEPTEMBER 30, 1		UMBER	Grade Crossing Struc- tures Re- construct- ed	-	-	Ľ		-	- 0	mt.	- m	-	×.	n w			-	∾ -	N		58
	2	N	Grade Crotings Eliminated by Separa- tion or Relocatios	4 6			טוס די	+	~ ~	- 2001	~~	1 10	5-0	no c	272	mr	- 21	ma	amr	101	246
	NDEP CONSTRUCTI		Federal Aid	8 697.827 7.592 1 086 848	707,664 283,222 628,437	137,499 37,626 765,788	2,171,855 892,262	316,258 531,467	291,627 26,841 1440,316	326,168 1,466,427 1,472,970	494.634 1,555.174 85.232	737,924 115,732 82,452	772,219 110,443	1,007,103 103,750	593,095 315,518 1,553,894	365, 705	1.354.563 1.354.563	209, 428 215, 038 165, 173	229,982 512,062 540,923	59,061 194,028 579,336	32,165,519
	T		Estimated Total Cost	* 717,904 7.592	2856,145 283,222 633,988	137,499 37,626 765,788	2,374,331 892,262	394,280 531,945 1.042,369	345,122 26,841 472,109	336,591 1,466,427 1,472,970	1,843,554 154,873	137,924 115,732 82,452	772, 219 110, 443	1.007.343 103.750 2.462.415	594, 442 594, 442 462, 847 1, 563, 370	191,039 365,705 281,762	1,367,684 40,813	209,428 216,368 166,673	229,982 541,103 540,923	59,061 194,036 584,007	33,437,487
	-	T	Grade Grade Crossings Protect- ed by Signals or Other- wise	- u	m	0-1	16	800	us tt.	50	ຸດ	2	9	~ = 0	55	90	ma	- m=+	7		222
	VEAR	UMBER	Grade Crossing Struc- tures Re- construct- ed		-				-		N	-	-	-		~ ~	N	•	ເນ		23
IO S	DURING CURRENT FISCAL Y	Z	Grade Crossings Eliminated by Separa- tion or Relocation	4-	N	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	- M-	- 10.7		**	5	~	- 0	- 10.3	N	cu =	- 00	- cu	9		85
STATUS			Foderal Aid	* 4,100 356,072 77,452	181,075 148,343	45, 759 198, 849 98, 753	377,451	151,600 126,425 161,777	95,496 149,214 180,993	15,710 682,519 507,509	23,760 150,832 276,720	195,478	140.504	112,535 132,190 347,336	244,771 528,871	3,750 120,394 72,600	204,265 957,100 9,358	2,969 80,765 199,253	728,501		9,439,913
	COMPLETED		Estimated Total Cost	* 4,100 363,787 77 1452	181,075	45, 759 203, 348 98, 753	104, 842 431, 062 237, 674	160,688 1126,125 161,777	95,496 149,214 180.996	15,710 682,519 515,366	23,760 150,832 276,720	100.989	140,504	112,535 1432,190 369,098	245, 798	3,831 120,394 72,600	204,265 959,152 9,569	2,969 81,101 199,253	743.581		9,570,435
			STATE	Alabama Arizona Arkansas	California Colorado Connecticut	Delaware Florida Georgia	Idaho Illinois Indiana	lowa Kansas Kentucky	Louisiana Maine Maryland	Massachusetts Michigan Minnesota	Mississippi Missouri Montana	Nebraska Nevada New Hampshire	New Jersey New Mexico New York	North Carolina North Dakota Ohio	Oklahoma Oregon Pennsylvania	Rhode Island South Carolina South Dakota	Tennessee Texas Utah	Vermont Virginia Washington	West Virginia Wisconsin Wyoming	District of Columbia Hawaii Puerto Rico	TOTALS

