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The reports of research published in this magazine are necessarily qualified by the conditions of the tests from which the data are obtained. Whenever it is deemed possible to do so, generalizations are drawn from the results of the tests; and, unless this is done, the conclusions formulated must be considered as specifically pertinent only to described conditions.

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PRELIMINARY RESULTS OF ROAD-USE STUDIES

BY DIVISION OF CONTROL, BUREAU OF PUBLIC ROADS

Reported by ROBERT H. PADDOCK and ROE P. RODGERS, Associate Highway Engineer Economists

TOTOR VEHICLES annually travel approximately MOTOR VEHICLES and any draver approach high-250 billion vehicle-miles over the streets and high-ways of the United States. The benefits derived from such travel may be considered one measure of the returns received on the large capital investment in highway facilities. To finance the facilities necessary for the effective handling of such a great volume of travel, a large portion of the needed revenues is collected from vehicles owners largely on the basis of motor-vehicle use. However, highways also furnish other benefits than those received directly by motorists, and highwayuser revenues are supplemented to a limited extent by revenues from other sources.

In planning highway programs an important problem is determination of where highway-user revenues should be spent to benefit the greatest number of motorists and to provide for the most essential needs on all classes of roads and streets. It is evident that a properly considered highway program should be intended, insofar as possible, to provide facilities for various classes of motorists so that the maximum benefits to the public will be realized.

Determinations of the type and extent of highway use as obtained from road-use studies will assist in formulating such a program. These studies, which are integral parts of the current State-wide highway planning surveys under way in 46 States, will provide factual bases for answering important questions concerning the nature of highway traffic. They will make possible an understanding of the relationships between highway expenditures and the travel of those who pay a large share of the transportation bill. The studies will also show the variations between the motoring habits of rural and urban residents and between the traffic of different types of motor vehicles.

The data presented in this summary are presented without the complementary material which will be available from other phases of the planning surveys, and which are essential in formulating integrated highwaydevelopment programs. However, a study of road-use data will assist in an understanding of highway-transportation problems.

ANALYSIS MADE OF INTERVIEWS FROM 17 REPRESENTATIVE STATES

Road-use information was obtained by means of a large number of personal interviews with motor-vehicle owners and drivers. These interviews were carefully selected to insure a proper representation of each geographical division of a State, of each group of governmental jurisdictions within similar population ranges, of various occupations, and of vehicles according to types and ages in operation. Information obtained from vehicle owners by survey interviewers made it possible to determine the extent of the owners' travel during the preceding year, and the routes of such travel for each trip. Experience has demonstrated that the year's driving of an individual can be accounted for 142758-39-1

reliably because of the numerous habitual trips, frequent local recreational trips, and unusual long trips that can be easily recalled.

By summarizing the data and expanding to the total State registration for each vehicle type-taking into account all known factors affecting the amount and kind of driving—information is obtained from which it is possible to estimate-

1. The total amount of travel on the various highway systems in a given area or in the State, and

2. The amount of travel performed on the various highway systems in the State by vehicle owners residing in the several governmental jurisdictions.

The two special analyses presented in this report are largely based upon preliminary road-use data obtained in the 17 States of Colorado, Florida, Iowa, Louisiana, Michigan, Minnesota, Missouri, Montana, New York, Ohio, Oklahoma, Oregon, Pennsylvania, South Dakota, Utah, Washington, and Wisconsin. Interviews cover-ing a total of 198,809 passenger cars and 71,941 trucks were taken in these States during different periods, most of them during 1936, but some at an earlier date. All figures were adjusted to the year 1936 in proportion to the motor-vehicle registrations for the particular vears under consideration. The 17-State sample was then expanded to obtain figures representing the entire United States by applying road-use data for a particular State to those surrounding or similar States for which data were not yet available.

Factors taken into consideration in these calculations included motor-vehicle registrations, the distribution of population by population groups (see table 1), motor-vehicle ownership per capita for various population groups, and existing mileages of the several highway systems in each State. A number of additional corrective factors were omitted in this preliminary analysis, but it is believed that the results are reliable.

The 17 States which formed the basis for this analysis represent:

- 45.4 percent of the estimated population of the United States in 1936.
- 47.8 percent of motor-vehicle registrations in the United
- States in 1936. 41.0 percent of the State primary road mileage in the United States in 1936.

Results of road-use studies indicate that these States were responsible for:

- 46.3 percent of estimated travel on all roads and streets in 1936.
- 44.9 percent of estimated travel on State-administered highways in 1936.

The close agreement of these figures indicates that for the purpose of this study, the 17 States were representative of the country as a whole.

That the estimate of total annual travel amounting to almost 250 billion vehicle-miles for all passenger cars, trucks, and busses in the United States is reasonable, can be demonstrated by comparison with the total

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 TABLE 1.—Approximate distribution of population and of motorvehicle registration in the United States in 1936 by population groups of residence

Population group	Populat	ion 1	Motor-vehicle regis- tration ²			
Unincorporated areas	Number 44, 636, 770	Percent 36.4	Number 8, 617, 876	Percent 30.6		
1,000 or less	4,362,746 4,820,707 10,614,746	3.6 3.9	1, 491, 044 1, 544, 370 2, 061, 970	5.3 5.5		
10,001 to 25,000 25,001 to 100,000	9,097,200 12,917,141	7.4 10.5	2, 444, 929 3, 419, 713	8.7 12.1		
Total	122, 775, 046	100.0	28, 165, 550	100.0		

¹ Population data from 1930 census. Total midyear population for 1936 estimated by United States Census Bureau at 128,429,000. ² Includes passenger cars, trucks, and busses.

quantity of gasoline consumed in street and highway travel. The total estimated travel of 249,778,990,000 vehicle-miles in 1936, divided by the 17,855,454,000 gallons of gasoline used on highways in 1936, gives an average of 14 miles per gallon for all types of motor vehicles. This result is in close agreement with other estimates of average gasoline consumption per vehicle made in recent years. Both this figure and the estimated average annual travel of 8,870 miles for all types of motor vehicles also compare favorably with similar values determined from other highway planning survey data in a number of States.

OVER HALF OF ALL TRAVEL PERFORMED ON PRIMARY STATE HIGHWAYS

The highway systems over which total travel was distributed are classified as (1) primary rural highways and transcity connections, (2) secondary highways and local rural roads, and (3) city streets. Primary rural highways under State control consisted of 339,000 miles which, with the urban extensions and connecting links through municipalities of 20,000 miles, totaled 359,000 miles in the United States in 1936.

The secondary and local rural road classification includes 178,000 miles of rural highways under State control other than primary State highways, as well as an estimated 2,440,000 miles of county and township roads or a total for this classification of 2,618,000 miles for the United States.

City street mileage comprised 215,000 miles, of which 20,000 miles was urban extensions and connecting links of the primary systems and 195,000 miles was the estimated total of other streets in all incorporated places in 1936.

In determining the distribution of travel to these various systems all travel on streets of incorporated places incurred in going to or coming from rural portions of the primary highway system was summarized separately and for this particular presentation has been credited to the primary system. Similarly, travel on city streets incurred in going to or coming from rural portions of the secondary system was credited to the secondary system. Purely local city travel originating inside a municipality and not extending beyond the city limits was credited to the local street classification, even though some of that travel occurred on the urban extensions or connecting links of the primary system within the city.

Table 2 shows the distribution of estimated annual motor-vehicle travel in the United States in 1936 on the various highway systems, as performed by motorvehicle owners resident in different population groups. Table 3 shows for each population group of residence or vehicle ownership the percentage of total annual travel performed on each of the highway systems. The composition of the total annual motor-vehicle travel occurring on each highway system according to the various population groups in which the travel originated appears in table 4.

Average annual travel figures for each highway system by motor-vehicle owners resident in each population group appear in table 5.

 TABLE 2.—Estimated motor-vehicle travel on various highway systems in the United States in 1936¹

	Total travel on—								
Travel by motor-vehicle owners resident in—	Primary rural high- ways and transcity connections	Secondary highways and local rural roads	City streets	All systems					
Unincorporated areas Incorporated places having a population of	Million ve- hicle-miles 40, 846. 6	Million ve- hicle-miles 19, 453. 7	Million ve- hicle-miles 3, 333.0	Million ve- hicle-miles 63, 633. 3					
1,000 or less 1,001 to 2,500 2,501 to 10,000 10,001 to 25,000 25,001 to 100,000 100,001 or more	$\begin{array}{c}9,869.0\\10,368.9\\19,800.8\\15,127.6\\18,632.8\\26,328.0\end{array}$	$\begin{array}{c} 2,942.7\\ 2,063.6\\ 2,909.3\\ 1,906.8\\ 2,044.5\\ 2,113.4\end{array}$	$\begin{array}{c} 760.\ 4\\ 1,\ 826.\ 8\\ 6,\ 284.\ 5\\ 6,\ 869.\ 8\\ 12,\ 710.\ 2\\ 43,\ 586.\ 5\end{array}$	$\begin{array}{c} 13,572.1\\ 14,259.3\\ 28,994.6\\ 23,904.2\\ 33,387.5\\ 72,027.9\end{array}$					
Total	140, 973. 7	33, 434. 0	75, 371. 2	249, 778. 9					

¹ Based on preliminary data from road-use surveys in 17 representative States.

 TABLE 3.—Percentage of estimated motor-vehicle travel on the various highway systems in the United States in 1936

	Total travel on-								
Travel by motor-vehicle owners resident in—	Primary rural high- ways and transcity connections	Secondary highways and local rural roads	City streets	All sys• tems					
Unincorporated areas Incorporated places having a popula- tion of-	Percent 64.2	Percent 30.6	Percent 5.2	Percent 100.0					
1,000 or less	72.7	21.7	5.6	100.0					
1,001 to 2,500	72.7	14.5	12.8	100.0					
2,501 to 10,000	68.3	10.0	21.7	100,0					
10,001 to 25,000	63.3	8.0	28.7	100.0					
25,001 to 100,000	55.8	0.2	38.0	100.0					
100,001 of more	30.6	2.9	60.5	100, 0					
Total	56.4	13.4	30.2	100.0					

TABLE 4.—Percentage of estimated motor-vehicle travel on eachhighway system by population groups of residence in whichtravel originated in the United States in 1936

	Total travel on—								
Travel by motor-vehicle owners resident in—	Primary rural high- ways and transcity connections	Secondary highways and local rural roads	City streets	All systems					
Unincorporated areas. Incorporated places having a popu- lation of—	Percent 29.0	Percent 58.2	Percent 4.4	Percent 25.5					
1,000 or less 1,001 to 2,500 2,501 to 10,000 10,001 to 25,000	7.0 7.4 14.0 10.7	8.8 6.2 8.7 5.7	$ \begin{array}{r} 1.0\\ 2.4\\ 8.3\\ 9.1 \end{array} $	5.4 5.7 11.6 9.6					
25,001 to 100,000 100,001 or more	13.2 18.7		$ 16.9 \\ 57.9 $	13.4 28.8					
Total	100.0	100.0	100.0	100.0					

The data presented in tables 2 and 3 indicate that of the nearly 250 billion vehicle-miles traveled in 1936 by passenger cars, trucks, and busses in the United States, 56.4 percent was travel on the primary rural highways and transcity connections, 13.4 percent on the secondary highways and local rural roads, and 30.2 percent on city streets. These figures may be more easily visualized by reference to table 5, which shows that the average motor vehicle traveled 8,870 miles during 1936, and that the division of this travel among the three classes of highways was 5,000, 1,190, and 2,680 miles, respectively.

TABLE	5E	Istimated	average	travel	per	motor	vehicle o	on the
	various	highway	systems	of the U	nited	l States	in 1936	

	Average travel on-								
Travel by motor-vehicle owners resident in—	Primary rural high- ways and transcity connections	Secondary highways and local rural roads	City streets	All systems					
Unincorporated areas. Incorporated places having a popu-	Vehicle- miles 4, 740	Vehicle- miles 2, 250	Vehicle- miles 390	Vehicle- miles 7, 380					
1,000 or less 1,000 or less 2,501 to 2,500 10,001 to 25,000 25,001 to 100,000 100,001 to 100,000 100,001 or more	$\begin{array}{c} 6, 620 \\ 6, 710 \\ 6, 470 \\ 6, 190 \\ 5, 450 \\ 3, 470 \end{array}$	1,970 1,340 950 780 600 280	$510 \\ 1, 180 \\ 2, 050 \\ 2, 810 \\ 3, 710 \\ 5, 740 \\ $	9, 100 9, 230 9, 470 9, 780 9, 760 9, 490					
Total	5,000	1, 190	2, 680	8, 870					

Because the total average annual travel for motor vehicles registered in each population group was relatively uniform with the exception of those owned in unincorporated areas (table 5), the percentage of total annual travel on all highways and streets corresponded very closely to the percentage distribution of vehicle registrations within each population group. This fact is apparent from comparison of the figures in the last columns of tables 1 and 4.

MAJOR USE OF PRIMARY HIGHWAYS WAS BY CITY CAR OWNERS

There was considerable difference, however, in the relative use of the various highway systems by vehicles registered in the several population groups. These differences are indicated in tables 3 and 5. Vehicles owned in unincorporated areas performed 64.2 percent and 30.6 percent of their travel in 1936 on the primary highways and the secondary and local rural roads, respectively, and used city streets for only 5.2 percent of their total travel.

The use of the various highway systems by vehicles owned in the smaller incorporated places was somewhat similar to that for rural-owned vehicles. However, it is interesting to note the extent of the change in use of other highway systems with increase in the size of the place of vehicle ownership. Vehicles owned in the group of smallest incorporated places used the primary highways and the secondary and local rural roads for 72.7 percent and 21.7 percent, respectively, of their total annual driving, while vehicles owned in cities having populations over 100,000 used these same systems to the extent of 36.6 percent and 2.9 percent, respectively. Vehicles owned in the smallest incorporated places were used on city streets for only 5.6 percent of their total annual travel, but those owned in the largest cities performed 60.5 percent of their annual travel on streets of incorporated places. (See table 3.)



FIGURE 1.—DISTRIBUTION OF ESTIMATED ANNUAL MOTOR-Vehicle Travel in the United States in 1936 by Residents of Unincorporated Areas and Incorporated Places.

This increase in the use of city streets by vehicles owned in the larger incorporated places was, of course, accompanied by a corresponding decrease in the use of other highway systems. It should be noted, however (table 3), that even for vehicles owned in the largest cities the primary rural highways and transcity connections were used for over one-third of the total annual travel. This use was sufficiently high to account for 18.7 percent (table 4) of the annual travel by all motor vehicles on the primary highway system.

As shown in table 4 and figure 1, the largest proportion of travel on the primary highway system was that of town and city residents. Motorists living in incorporated places accounted for 71 percent of the total travel on this system.

The importance of the primary highways to the city motorist is apparent. Though his use of the rural highway system decreased (see table 3) as the size of the place in which the motorist lived increased, the percentage of his travel on such highways was considerable. Only for vehicle owners resident in cities over 100,000 population did their travel on primary rural highways fall below 50 percent of their total travel.

Considering only residents of cities having populations of more than 10,000, table 2 shows that they accounted for more than 60 billion vehicle-miles of the 140,973,700,000 vehicle-miles traveled on primary highways in 1936. Residents of unincorporated areas accounted for only slightly more than 40 billion vehiclemiles of the primary highway travel, or less than that provided by vehicle owners from cities having more than 25,000 population.

In considering travel by residents of incorporated places having 10,000 population or less, it is significant that the percentage of their travel on primary highways as shown in table 3 was greater than that of any other

group, even the residents of unincorporated areas. Practically all such incorporated places are located on or within a very short distance from primary roads. Highway development in this country has been such that generally it has been expected that all but the very smallest places would be given consideration in the location of primary routes. Vehicle owners living within these cities are located close to primary highways; they are much closer than many rural residents who live on township or county roads; and they are generally closer than residents of the larger cities who frequently must travel a considerable distance to reach primary routes.

It is apparent from this discussion and from the data shown in the accompanying tables that the major use of the primary highways is by city motor-vehicle owners, and that in spite of their large use of local city streets, the use of primary highways by all city-owned vehicles is greater than their use of all other highway facilities. It therefore follows that the provision of adequate rural highway facilities today is of major importance to the city motorist and that the required improvements in those facilities are largely occasioned by the city motorists' demands on the primary system.

TRIP-LENGTH DATA OBTAINED IN 11 STATES

Table 4 shows that only 4.4 percent of the total travel on city streets was contributed by motorists living in unincorporated areas, and that most of the remaining 95.6 percent of travel performed by residents of incorporated places represented the operations of residents in the larger cities. Of all travel on city streets, 57.9 percent was performed by residents of cities having populations of over 100,000, and drivers living in cities with over 10,000 inhabitants accounted for 83.9 percent of the total travel on local city streets.

Concerning motor-vehicle use on all streets and highways, approximately one-fourth was by residents of unincorporated areas, while twice that amount, or 51.8 percent of all travel, represents the driving of those living in cities having over 10,000 inhabitants. The largest percentage of vehicle travel accounted for by residents of any one group of governmental units as shown in table 4 was that originating in cities having populations over 100,000. Residents of these cities contributed 28.8 percent of all travel on all roads and streets.

These data on vehicle travel have also been expressed in terms of average 24-hour traffic volumes for each class of road and street. Table 6 shows that for the

TABLE 6.--Approximate mileage of each highway system and average 24-hour traffic volume on each highway system in the United States in 1936

Highway system	Approxi- mate mileage	Estimated total an- nual motor- vehicle travel	A verage 24-hour traffic volume
Primary rural highways and transcity con- nections. Secondary highways and local rural roads City streets. All systems.	Miles 359,000 ¹ 2,618,000 ² 215,000 ³ 3,172,000	Million vehisle-miles 140, 973, 7 33, 434, 0 75, 371, 2 249, 778, 9	Vehicles 1,076 35 960 216

¹ Based on latest available estimates.
 ² Estimate includes 20,000 miles of transcity connections which are also included with primary system mileage, because exclusively local city travel includes travel over such connections.
 ³ Excludes duplication of 20,000 miles of trans-city connections.

country as a whole, primary rural highways and their transcity connections carried an average daily volume of 1,076 vehicles, which was slightly higher than the 960 vehicles computed as the average for city streets. These volumes were about 30 times greater than the average daily volume on secondary and local rural roads combined. Average 24-hour traffic volume for the more than 3 million miles of roads and streets in the United States was estimated at 216 vehicles.

Another special study of considerable value was also made from road-use data concerning the radii of operation of motor vehicles. It was sought by this investigation to determine the length of vehicle trips that extend beyond the limits of cities; that is, of trips that are either partly or wholly on rural roads. Thus all trips by residents of unincorporated areas were included; but for motorists living in incorporated places, only those trips were counted that extended beyond the limits of the town or city in which the driver resided.

This special study was made in the 11 States of Florida, Kansas, Louisiana, Minnesota, New Hamp-shire, Pennsylvania, South Dakota, Utah, Vermont, Washington, and Wisconsin. In 1936 there were 4,862,541 passenger cars and 880,432 trucks registered in these 11 States, or a combined registration of 5,742,-973. These figures are presented in table 7, together with information concerning the number of interviews taken in each State. The number of interviews totaled 129,407, and consisted of 94,167 for passenger cars and 35,240 for trucks. Trip-length information was not expanded to represent data for the entire country, but only to represent total registrations in each of these States.

For purposes of this analysis, all trips have been classified as one-way trips. If a motor-vehicle owner left his home and drove to some other point 10 miles distant, requiring a total travel of 20 miles from point of starting until return to that point, such a trip could be classified as two one-way trips of 10 miles each. The one-way trip classification has been used for all tabulations in this discussion.

TABLE 7.—1936 motor-vehicle registrations and number of road use interviews used for basis of analysis of total number of one-way trips outside city limits in 11 States

	193	6 registrat	ion	Number of interviews				
State	Passenger cars	Trucks	Total	Passenger cars	Trucks	Total		
Florida Kansas Jouisiana Minnesota New Hampshire Pennsylvania South Dakota Utah Vermont Washington Wisconsin	$\begin{array}{c} 321,467\\ 490,793\\ 228,361\\ 668,915\\ 97,361\\ 1,615,955\\ 158,192\\ 96,768\\ 75,195\\ 419,493\\ 690,041 \end{array}$	$\begin{array}{c} 63,885\\{}^{1}87,113\\73,628\\114,448\\{}^{1}24,875\\235,834\\{}^{2}28,216\\19,397\\8,845\\79,538\\144,653\end{array}$	$\begin{array}{c} 385, 352\\ 577, 906\\ 301, 989\\ 783, 363\\ 122, 236\\ 1, 851, 789\\ 186, 408\\ 116, 165\\ 84, 040\\ 499, 031\\ 834, 694 \end{array}$	$\begin{array}{c} 7,015\\ 8,663\\ 3,891\\ 13,059\\ 1,936\\ 23,783\\ 3,608\\ 2,148\\ 1,472\\ 14,027\\ 14,565\end{array}$	$\begin{array}{c} 3,010\\ 2,813\\ 1,623\\ 5,649\\ 914\\ 10,567\\ 1,533\\ 1,097\\ 850\\ 1,313\\ 5,871\\ \end{array}$	$\begin{array}{c} 10,025\\ 11,476\\ 5,514\\ 18,708\\ 2,850\\ 34,350\\ 5,141\\ 3,245\\ 2,322\\ 15,340\\ 20,436\end{array}$		
Total	4, 862, 541	880, 432	5, 742, 973	94, 167	35, 240	129, 40		

¹ Includes busses.

PASSENGER-CAR AND TRUCK TRIPS PREDOMINATELY OF SHORT LENGTH

Tables 8 and 9 contain analyses of the length of oneway trips partially or wholly traveled on roads in unin-corporated areas. The numbers of these trips within designated length classifications are shown graphically in figure 2 for passenger cars and trucks combined.

The short length of travel of a large part of motor-



FIGURE 2.—PERCENTAGE OF ALL PASSENGER-CAR AND TRUCK TRIPS EXCEEDING VARIOUS LENGTHS.

vehicle operation is readily apparent. For passenger cars, trips of less than 5 miles constituted 38.4 percent of the number of all one-way trips traveled partly or wholly on highways in unincorporated areas. Trips of over 5 but less than 10 miles comprised 26.5 percent of the total. Of all the one-way trips tabulated, therefore, 64.9 percent of the total number were less than 10 miles long. Trips less than 20 miles long accounted for 85 percent of all passenger-car trips. Accordingly, only 15 percent of all trips extending beyond city limits or traveled entirely on rural roads were greater than 20 miles long. Passenger cars went over 100 miles from their starting point on only 1.5 percent of all their trips.

Analysis of truck movements gave fairly similar results, 34 percent of all trips being less than 5 miles long, 59.5 percent less than 10 miles, and 80.3 percent less than 20 miles. Trips over 100 miles were 2.0 percent of the total number of all trips, and truck trips above 50 miles and less than 250 miles long constituted 6.2 percent of the total number as compared with 4 percent for passenger cars.

Considering passenger cars and trucks combined, 37.5 percent of the number of all one-way trips involving travel on roads in unincorporated areas extended less than 5 miles from the point of origin. The fact that the many short trips made wholly within incorporated areas have been omitted from these trip-length data emphasizes still further the preponderant use of motor vehicles for short trips.

Tables 10 and 11 show the States of destination of one-way trips over 100 miles long made by passenger cars and by trucks registered in the 11 States. These data are summarized in table 12 to show the percentage of such trips having destinations in the State of origin,

TABLE 8.—Frequency distribution of the length of all one-way trips made by passenger cars that extended outside city limits in 11 States 1 TOTAL NUMBER OF TRIPS

				Length of	one-way tr	ips from po	oint of orig	in in miles				m
State	$\underset{5}{\overset{\text{Less than}}{5}}$	5 to 9.9	10 to 19.9	20 to 29.9	30 to 39.9	40 to 49.9	50 to 99.9	100 to 249.9	250 to 499.9	500 to 999.9	1,000 and over	Total all trips
Florida Kansas Louisiana Minnesota New Hampshire Pennsylvania South Dakota Utah Vermont Washington Wisconsin	$\begin{array}{c} 1,000\\ trips\\ 45,189\\ 124,109\\ 43,984\\ 97,533\\ 10,782\\ 296,153\\ 16,704\\ 17,019\\ 14,763\\ 73,201\\ 95,622 \end{array}$	$\begin{array}{c} 1,000\\ trips\\ 40,584\\ 69,011\\ 25,005\\ 62,426\\ 12,941\\ 214,362\\ 11,760\\ 9,198\\ 10,650\\ 42,913\\ 77,445 \end{array}$	$\begin{array}{c} 1,000\\ trips\\ 31,803\\ 54,600\\ 18,019\\ 51,591\\ 10,975\\ 154,277\\ 11,880\\ 6,838\\ 6,178\\ 33,562\\ 58,499 \end{array}$	$\begin{array}{c} 1,000\\ trips\\ 11,069\\ 17,267\\ 6,970\\ 16,477\\ 3,046\\ 47,626\\ 4,894\\ 2,626\\ 2,079\\ 11,020\\ 20,691 \end{array}$	$\begin{array}{c} 1,000\\ trips\\ 4,055\\ 6,526\\ 2,986\\ 6,477\\ 1,393\\ 21,246\\ 1,631\\ 1,631\\ 1,631\\ 718\\ 5,700\\ 9,538 \end{array}$	$\begin{array}{c} 1,000\\ trips\\ 1,373\\ 2,282\\ 1,335\\ 3,680\\ 900\\ 9,180\\ 1,233\\ 908\\ 562\\ 2,208\\ 4,123\end{array}$	$\begin{array}{c} 1,000\\ trips\\ 3,776\\ 6,634\\ 3,842\\ 8,224\\ 1,536\\ 19,254\\ 1,950\\ 1,053\\ 757\\ 4,882\\ 9,324 \end{array}$	$\begin{array}{c} 1,000\\trips\\1,497\\2,591\\946\\5,045\\326\\8,016\\734\\477\\223\\2,062\\3,826\end{array}$	$\begin{array}{c} 1,000\\ trips\\ 400\\ 458\\ 182\\ 556\\ 27\\ 763\\ 180\\ 134\\ 28\\ 359\\ 541\end{array}$	$\begin{array}{c} 1,000 \\ trips \\ 85 \\ 154 \\ 35 \\ 89 \\ 8 \\ 221 \\ 41 \\ 60 \\ 1 \\ 49 \\ 82 \end{array}$	$\begin{array}{c} 1,000\\ trips\\ 76\\ 89\\ 10\\ 45\\ 4\\ 91\\ 24\\ 41\\ 1\\ 59\\ 49\end{array}$	$\begin{array}{c} 1,000\\ trips\\ 139,90\\ 283,72\\ 103,31\\ 252,14\\ 41,93\\ 771,18\\ 51,03\\ 40,00\\ 05,96\\ 176,01\\ 279,74 \end{array}$
Total	835, 059	576, 295	438, 222	143, 765	61, 923	27, 784	61, 232	25, 743	3, 628	825	489	2, 174, 96
		PERCE	NTAGE (OF TOTA	L NUMB	ER OF T	RIPS					
Florida	Percent 32.3 43.8 42.6 38.7 25.7 38.4 32.7 42.6 41.0 41.6 34,2	Percent 29.0 24.3 24.2 24.8 30.9 27.8 23.0 29.6 24.4 27.7	Percent 22. 7 19. 2 17. 4 26. 1 20. 0 23. 3 17. 1 17. 2 19. 0 20. 9	Percent 7.9 6.1 6.8 6.5 7.3 9.6 6.5 5.8 6.3 7.4	Percent 2.9 2.3 2.9 2.6 3.3 2.7 3.2 4.1 2.0 3.2 3.4	$\begin{array}{c} Percent \\ 1.0 \\ .8 \\ 1.3 \\ 1.5 \\ 2.2 \\ 1.2 \\ 2.4 \\ 2.3 \\ 1.6 \\ 1.3 \\ 1.5 \end{array}$	Percent 2.7 2.3 3.7 3.2 3.6 3.6 3.6 3.6 3.6 2.5 3.8 2.6 2.1 2.8 3.3	$\begin{array}{c} Percent \\ 1.1 \\ .9 \\ .9 \\ 2.0 \\ .8 \\ 1.0 \\ 1.4 \\ 1.2 \\ .6 \\ 1.1 \\ 1.4 \end{array}$	Percent 0.3 .2 .2 .1 .1 .4 .3 .1 .2 .2 .2 .2 .2 .2 .2 .2 .2 .2	Percent 0.1 (²) .1 (³) .1 .1 .1 .2 (²) .1 (²)	Percent (2) (3) (3) (4) (2) (2) (2) (2) (2) (2) (2) (2)	Percent 100 100 100 100 100 100 100 100 100 10
Total	38.4	26.5	20.1	6.6	2.8	1.3	2.8	1.2	. 2	.1	(2)	10
	CUMU	LATIVE	PERCEN	TAGE O	F TOTAL	NUMBE	ER OF TI	RIPS				
Florida Kansas Louisiana Minnesota New Hampshire Pennsylvania South Dakota Utah Vermont Washington Wisconsin Total	Percent 32, 3 43, 8 42, 6 38, 7 25, 7 38, 4 32, 7 42, 6 41, 0 41, 6 34, 2 38, 4	Percent 61.3 68.1 66.8 63.5 56.6 66.2 55.7 65.6 70.6 65.6 70.6 66.0 61.9 64.9	Percent 84.0 87.3 84.2 83.9 82.7 86.2 79.0 82.7 87.8 85.0 82.8 85.0	Percent 91.9 93.4 91.0 90.4 90.0 92.4 88.6 89.2 93.6 91.3 90.2 91.6	Percent 94.8 95.7 93.9 93.0 93.3 95.1 91.8 93.3 95.6 94.5 93.6 94.5 93.6	Percent 95.8 96.5 95.2 94.5 95.5 96.3 94.2 95.6 97.2 95.6 97.2 95.8 95.1 95.7	Percent 98.5 98.8 98.9 97.7 99.1 98.8 98.0 98.2 99.3 98.6 98.4	Percent 99.6 99.7 99.8 99.7 99.9 99.8 99.4 99.4 99.9 99.7 99.8 99.7	Percent 99.9 100.0 99.9 100.0 99.9 99.8 99.7 100.0 99.9 100.0 99.9	Percent 100.0 100.0 100.0 100.0 100.0 99.9 99.9 100.0 100.0 100.0	Percent 100 100 100 100 100 100 100 10	Percent

¹ Based on analysis of 42,407,204 one-way trips performed by 94,167 passenger cars in these States. ² Less than 0.1 percent. TABLE 9.-Frequency distribution of the length of all one-way trips made by trucks that extended outside city limits in 11 States 1 TOTAL NUMBER OF TRIPS

				Length of	one-way tr	ips from po	oint of origi	n in miles				00111
State	Less than 5	5 to 9.9	10 to 19.9	20 to 29.9	30 to 39.9	40 to 49.9	50 to 99.9	100 to 249.9	250 to 499.9	500 to 999.9	1,000 and over	trips
Florida Kansas Louisiana Minnesota New Hampshire Pennsylvania South Dakota Utah Vermont Washington Wisconsin	$\begin{array}{c} 1,000\\ trips\\ 11,145\\ 17,622\\ 14,769\\ 20,591\\ 2,708\\ 53,717\\ 2,824\\ 4,579\\ 3,159\\ 17,515\\ 36,323\end{array}$	$\begin{array}{c} 1,000\\ trips\\ 11,433\\ 12,722\\ 11,787\\ 20,750\\ 3,504\\ 33,446\\ 2.528\\ 1,960\\ 2,817\\ 9,438\\ 28,531\end{array}$	$\begin{array}{c} 1,000\\ trips\\ 8,504\\ 10,389\\ 12,645\\ 14,942\\ 2,584\\ 24,704\\ 3,104\\ 1,616\\ 1.969\\ 9,724\\ 23,340\end{array}$	$\begin{array}{c} 1,000\\ trips\\ 3,616\\ 3,777\\ 4,803\\ 5,534\\ 863\\ 7,291\\ 1,239\\ 659\\ 564\\ 4,734\\ 8,775\end{array}$	$\begin{array}{c} 1,000\\ trips\\ 1,456\\ 1,540\\ 2,539\\ 2,169\\ 414\\ 3,789\\ 516\\ 395\\ 247\\ 2,768\\ 4,197\end{array}$	$\begin{array}{c} 1,000\\ trips\\ 514\\ 741\\ 962\\ 1,302\\ 195\\ 1,897\\ 542\\ 247\\ 197\\ 903\\ 2,759\end{array}$	$\begin{array}{c} 1,000\\ trips\\ 1,358\\ 1,946\\ 3,909\\ 3,767\\ 602\\ 4,077\\ 1,181\\ 462\\ 258\\ 1,966\\ 4,382\end{array}$	$\begin{array}{c} 1,000\\ trips\\ 842\\ 792\\ 1,325\\ 2,511\\ 75\\ 1,130\\ 505\\ 314\\ 511\\ 755\\ 1,611\end{array}$	$\begin{array}{c} 1,000\\ trips\\ 179\\ 111\\ 97\\ 206\\ 3\\ 59\\ 77\\ 45\\ 33\\ 72\\ 152\end{array}$	1,000 trips 19 23 8 11 (2) 20 6 5 2 6 6 10	1,000 trips 14 16 (³) 3 (²) (²) 2 2 (²) 2 2 (²) 8 (⁷)	$\begin{array}{c} 1,000\\ trips\\ 39,080\\ 49,679\\ 52,844\\ 71,786\\ 10,948\\ 130,130\\ 12,524\\ 10,284\\ 9,207\\ 47,892\\ 110,080\end{array}$
Total	184, 952	138, 916	113, 521	41,855	20, 030	10, 262	23, 908	9, 911	1,034	110	45	544, 544
		PERCE	NTAGE	OF TOTA	L NUME	EROFI	RIPS					
Florida Kansas Louisiana Minnesota New Hampshire Pennsylvania South Dakota Utah Vermont	$\begin{array}{c} Percent \\ 28,5 \\ 35,5 \\ 27,9 \\ 28,7 \\ 41,3 \\ 22,5 \\ 44,5 \\ 34,0 \\ 36,6 \\ 33,0 \end{array}$	Percent 29, 2 25, 6 22, 3 28, 9 32, 0 25, 7 20, 2 19, 1 30, 3 19, 7 25, 9	Percent 21. 8 20. 9 23. 9 20. 8 23. 6 19. 0 24. 8 15. 7 21. 2 20. 3 21. 2	$\begin{array}{ c c } Percent \\ 9,3 \\ 7,6 \\ 9,1 \\ 7,7 \\ 7,9 \\ 5,6 \\ 9,9 \\ 6,4 \\ 6,0 \\ 9,9 \\ 8,0 \\ \end{array}$	Percent 3.7 3.1 4.8 3.0 3.8 2.9 4.1 3.8 2.7 5.7 3.8	Percent 1.3 1.5 1.9 1.8 1.4 4.3 2.4 2.1 1.9 2.5	$\begin{array}{c} Percent \\ 3.5 \\ 3.9 \\ 7.4 \\ 5.2 \\ 5.5 \\ 3.1 \\ 9.4 \\ 4.5 \\ 2.8 \\ 4.1 \\ 4.0 \end{array}$	$\begin{array}{c} Percent \\ 2.1 \\ 1.6 \\ 2.5 \\ 3.5 \\ .7 \\ .9 \\ 4.0 \\ 3.1 \\ .5 \\ 1.6 \\ 1.5 \end{array}$	Percent 0.5 .2 .3 (³) .1 .6 .4 .4 .2 .1 .1 .1 .2 .2 .2 .3 .3 .3 .3 .1 .4 .2 .2 .3 .3 .3 .3 .3 .4 .3 .3 .4 .3 .3 .3 .3 .4 .3 .3 .3 .3 .4 .4 .5 .5 .5 .5 .5 .5 .5 .5 .5 .5	Percent 0. 1 (3) . 1 (5) . 1 (3) (3) (3) (3)	Percent (3) (3) (3) (3) (3) (3) (3) (3)	$\begin{array}{c} Percent \\ 100 \\ 100 \\ 100 \\ 100 \\ 100 \\ 100 \\ 100 \\ 100 \\ 100 \\ 100 \\ 100 \end{array}$
Total	34.0	25.5	20.8	7.7	3.7	1.9	4,4	1.8	. 2	(3)	(3)	100
	CUMU	LATIVE	PERCEN	TAGE O	F TOTAI	NUMBI	ER OF TI	RIPS	1	1		<u> </u>
Florida Kansas Louisiana Minnesota New Hampshire Pennsylvania South Dakota Utah Vermont Washington Wisconsin	Percent 28,5 35,5 27,9 28,7 24,7 41,3 22,5 34,0 36,6 33,0 34,0	Percent 57.7 61.1 50.2 57.6 56.7 67.0 42.9 63.6 64.3 56.3 56.3 58.9	Percent 79.5 82.0 74.1 78.4 80.3 86.0 67.5 79.3 85.5 76.6 80.1 80.3	Percent 88, 8 89, 6 83, 2 86, 1 88, 2 91, 6 77, 4 85, 7 91, 5 86, 5 88, 1 88, 0	Percent 92,5 92,7 88,0 94,5 81,5 89,5 94,2 92,2 91,9 91,7	Percent 93.8 94.2 89.9 90.9 93.8 95.9 85.8 91.9 96.3 94.1 94.4	Percent 97.3 98.1 97.3 99.0 99.0 95.2 96.4 99.1 98.2 98.4 98.0	Percent 99, 4 99, 7 99, 8 99, 6 100, 0 99, 0 99, 2 99, 5 99, 6 99, 8 99, 9	Percent 99.9 90.9 100.0 99.9 100.0 100.0 99.8 99.9 100.0 100.0 100.0	Percent 100.0 100.0 100.0 100.0 100.0 99.9 100.0 100.0 100.0 100.0 100.0	Percent 100 100 100 100 100 100 100 10	Percent

Based on analysis of 22,268,882 one-way trips performed by 35,240 trucks in these States.
 Less than 500 trips.
 Less than 0.1 percent.

TABLE 10.—Estimated total number of annual one-way trips over 100 miles long traveled by passenger cars registered in 11 States of origin, and classified by State of destination of individual trips

					Sta	ate of origin					
State of destination	Florida	Kansas	Louisiana	Minnesota	New Hampshire	Pennsyl- vania	South Dakota	Utah	Vermont	Washing- ton	Wisconsin
Alabama Arizona Arkansas California Colorado	38, 172 1, 368 870 1, 032	$\begin{array}{r} 418\\764\\38,180\\16,122\\144,456\end{array}$	$33,422 \\72 \\43,030 \\1,556 \\130$	546 726 8,994 1,434	84	960 108 594 3, 516 994	1,498430865,7105,902	90 3, 474 33, 084 21, 856			102 172 496 5, 716 1, 088
Connecticut Delaware Florida Georgia Idaho	$1,060 \\ 168 \\ 1,746,624 \\ 137,300$	2,048 688 1,204	130 8, 518 6, 096	2,156 110 216	9, 044 280 1, 514	$18,178 \\ 124,146 \\ 30,266 \\ 3,986$	86 456	115 159	7, 424 86 86	160 220 122 418	596 5,838 628 260
Illinois Indiana Iowa. Kansas	$\begin{array}{r} 4,496\\ 4,590\\ 610\\ 296\end{array}$	$\begin{array}{c} 23,858\\ 3,582\\ 20,996\\ 1,827,206\end{array}$	2,060 292 344 330	59, 168 2, 398 119, 550 842	288 126	29,806 12,360 1,906 950	$ \begin{array}{c} 1, 620 \\ 7, 314 \\ 1, 442 \\ 73, 950 \\ 3, 090 \\ \end{array} $			$ \begin{array}{c} 125,418\\ 2,040\\ 494\\ 466\\ 312 \end{array} $	536, 088 34, 820 67, 972 990
Kentucky Louisiana Maine Maryland Massachusetts	6,094 7,826 1,140 1,500 2,570	2,462 4,026 278	726 873, 324 324	436 612 110 152	84 47, 766 420	$\begin{array}{r} 6,564\\ 904\\ 10,444\\ 381,698\end{array}$	198 602	120	9, 366	126	2, 916 926 286 994
Michigan Minnesota Mississippi Missouri	2,370 3,814 424 6,482 1,156	9,594 14,926 1,504 505,022	584 88, 934 990	56,724 5,106,158 196 5,554	124, 144 428	36, 346 46, 672 742	1, 444 101, 304	150 758 76	79, 258	96 3, 376 1, 528 62 684	1,918 158,378 322,160 632 426
Montana Nebraska Nevada New Hampshire	134 136 538	1, 570 98, 148 424		5, 232 6, 592	100, 766	2, 010 360 916 7, 056	2, 502 3, 292 24, 628	400 21, 496 544 31, 824	22, 576	32, 826 392 844	6,430 1,222 4,402
New Jersey New Mexico	2,044	98 8, 696	924	76	856	1, 100, 928	100	956	1, 560	164	528

 TABLE 10.—Estimated total number of annual one-way trips over 100 miles long traveled by passengercars registered in 11 States of origin, and classified by State of destination of individual trips—Continued

					Sta	ate of origin					
State of destination	Florida	Kansas	Louisiana	Minnesota	New Hampshire	Pennsyl- vania	South Dakota	Utah	Vermont	Washing- ton	Wisconsin
New York North Carolina North Dakota Ohio Oklahoma Oregon Pennyslyania	$12.708 \\ 23,894 \\ 68 \\ 6.056 \\ 584 \\ 100 \\ 4.482 \\ 200 \\ 700 \\ 1$	$\begin{array}{r} 4,458\\ 274\\ 562\\ 4,124\\ 284,538\\ 2,042\\ 1,076\end{array}$	$1,606 \\ 2.396 \\ 126 \\ 396 \\ 1.812 \\ 256 \\ 162 $	2, 296 89, 242 1, 786 1, 214 640 720	14, 820 84 534 560	$1, 368, 620 \\ 14, 120 \\ 244 \\ 837, 310 \\ 1, 578 \\ 114 \\ 4, 419, 820 \\ 1$	$2,526 \\ 20,480 \\ 186 \\ 258 \\ 952$	902 492 152 4, 188	28, 172 116 232 1, 284	$1,620 \\ 160 \\ 1,066 \\ 204 \\ 382 \\ 343,286 \\ 160 \\$	$\begin{array}{c} 9, 522 \\ 2, 662 \\ 4, 032 \\ 8, 896 \\ 424 \\ 586 \\ 2, 256 \end{array}$
Knode Island South Carolina South Dakota Tennessee Texas		$70 \\ 98 \\ 3,048 \\ 1,872 \\ 41,380 \\ 962 \\ 962$	6, 868 93, 052	$660 \\ 62,812 \\ 1,048 \\ 2,308 \\ 196$	224 84	$\begin{array}{r} 4,126\\ 2,620\\ 228\\ 4,620\\ 1,682 \end{array}$	698, 396 184 796 272	90 570 314,246	3, 032	904 774 2,356	$ \begin{array}{r} 68\\ 11,084\\ 1,650\\ 2,268\\ 462 \end{array} $
Vermont	$\begin{array}{r} 340\\ 2, 980\\ 220\\ 618\\ 976\\ 168\\ 5, 042\\ 2, 594\\ 220\end{array}$	$\begin{array}{r} 380\\ 2, 196\\ 352\\ 1, 906\\ 9, 136\\ 1, 718\\ 2, 944\\ 2, 610\end{array}$	304 162 388 130 528 848 1,756	2, 158 286 157, 866 1, 748 1, 096 31, 142 1, 284	34, 402 252 	$\begin{array}{c} 5,206\\ 94,218\\ 142\\ 159,020\\ 3,694\\ 490\\ 294,168\\ 55,096\\ 366\end{array}$	86 2,450 5,870 10,748 2,336 794	3, 302 90 148, 118 210 8, 960 428	73, 884 402 26, 328	$ \begin{array}{r} 46 \\ 4,882,136 \\ 416 \\ 4,86 \\ 276 \\ 276 \\ 76,274 \\ 3,618 \\ 36 $	472 558 124 3, 277, 310 1, 534 1, 510 13, 678 748
Total	2, 056, 412	3,092,706	1, 172, 848	5, 737, 020	366, 038	9,090,492	981, 988	712,052	254,038	2, 528, 100	4, 497, 614

 TABLE 11.—Estimated total number of annual one-way trips over 100 miles long traveled by trucks registered in 11 States of origin, and classified by State of destination of individual trips

					Sta	ate of origin					
State of destination	Florida	Kansas	Louisiana	Minnesota	New Hampshire	Pennsyl- vania	South Dakota	Utah	Vermont	Washing- ton	Wisconsin
Alabama	38, 538		70	23							
Arizona								2,990			
Arkansas		2,134	34, 250								
California		52			44	40		5, 242		8, 296	176
Colorado		13, 748					10.704	7,958			
Connecticut					3, 576	1, 318			2,078		
Delaware						14, 238					
Florida	- 930, 621					190					
Georgia	- 49, 568			44		514		50.010		ro moe	
Illinois		0.204		10 00			. 05 1024	09, 912		02,780	261 201
Indiana	- 220	2, 504		12,009		2, 294	9.24			110	001,021
Inulana	- 141	2 510		0, 027		00	12 002				2,000
Kaneac		551 866		37, 100			40, 002	[110	00,102
Kontucky	137	652				A 99A	400			110	176
Louisiana	126	002	1 266 240			7, 247					110
Maine	140		1, 200, 210		11 850				72		
Maryland	5 321				,	71 294					176
Massachusetts	.,				38, 142	4.114			12,604		
Michigan	133	15.566		378	00,100	764			,	204	75.605
Minnesota		154		2, 489, 410			28, 570				201, 889
Mississippi	. 250		30, 856								
Missouri		233, 474	136	38, 565		132	306				176
Montana							8,020	598		17,472	
Nebraska		26, 368		2, 556			19,652			110	176
Nevada								11, 122			
New Hampshire					13,990				9,426		
New Jersey	. 45				220	34,420			4, 830		
New Mexico		716	66								
New York	2,504			56	2,156	371, 576			36, 822		170
North Carolina	4, 251		60			394	E 010				170
North Dakota		60		94, 500		104 100	5, 018				1 0~0
Oklahoma	. 180	D() E5 610		211		104, 188					1,010
Orogon		00, 015	00		*	40		20		80.058	
Panneylyania	2.077					407 144		20	4 0.96	00, 000	
Rhode Island	2,011				6 600	54			86		
South Carolina	3 599				0,000						
South Dakota		60		7 689			463, 604			110	176
Tennessee.	4, 921	00	546	1,000		32					
Texas	148	5, 690	95, 366					96			
Utah					2,260			235, 788			
Vermont						228			11,862		
Virginia	. 2,070	60				6,246					176
Washington							936			676, 740	
West Virginia.	- 47					32, 678					
Wisconsin				20, 907							1, 092, 571
Wyoming		364				10,000	7,752	43, 918		110	
District of Columbia	8, 913					13, 622				0 100	
Canada.				516	132	332			3,051	3, 132	
MICAICO								04		225	
Total	1, 053, 790	942, 434	1, 427, 650	2, 730, 606	78.970	1, 210, 192	590, 040	367, 720	85, 460	839, 466	1, 773, 096



FIGURE 3.—PERCENTAGE OF TOTAL VEHICLE-MILES BY PAS-SENGER CARS AND TRUCKS TRAVELED ON TRIPS EXCEEDING VARIOUS LENGTHS.

adjoining States, and other States, providing another indication of the dispersion of motor-vehicle travel. It should be noted that even on these longer trips a very high percentage of the destinations was in the State of origin or in an adjoining State.

TABLE	12.—Destination	of	motor	-vehi	cle t	ravel	in	11	States	on
	one-way	trip	os over	100	mile	s long	7			

PASSENGER CARS

		Destination	of trips in—	
State of origin	State of origin	Adjoining States	Other States	Total
Florida Kansas Louisiana Minnesota New Hampshire Pennsylvania South Dakota Utah Vermont Washington Wisconsin	Percent 84. 9 55. 5 74. 5 89. 0 27. 5 48. 6 71. 1 44. 1 29. 1 74. 4 72. 9	$\begin{array}{c} Percent \\ 8.5 \\ 37.4 \\ 19.2 \\ 8.0 \\ 60.6 \\ 43.7 \\ 23.9 \\ 45.0 \\ 61.5 \\ 21.5 \\ 24.1 \end{array}$	$\begin{array}{c} Percent \\ 6.6 \\ 7.1 \\ 6.3 \\ 3.0 \\ 11.9 \\ 7.7 \\ 5.0 \\ 10.9 \\ 9.4 \\ 4.1 \\ 3.0 \end{array}$	$\begin{array}{c} \hline Percent \\ 100 \\$
	TRUCK	8		
Florida Kansas Louisiana Minnesota New Hampshire Pennsylvania South Dakota Utah Vermont Washington Wisconsin	$\begin{array}{c} 88.3\\ 61.7\\ 88.7\\ 91.2\\ 17.7\\ 41.1\\ 78.6\\ 64.1\\ 13.9\\ 80.6\\ 61.6\end{array}$	$\begin{array}{c} 8,4\\ 35,0\\ 11,2\\ 6,6\\ 63,5\\ 56,1\\ 19,2\\ 34,2\\ 73,1\\ 16,2\\ 38,1\\ \end{array}$	$\begin{array}{c} 3.3\\ 3.3\\ .1\\ 2.2\\ 2.8\\ 2.8\\ 2.2\\ 1.7\\ 13.0\\ 3.2\\ .3\\ \end{array}$	$ \begin{array}{c} 100\\ 100\\ 100\\ 100\\ 100\\ 100\\ 100\\ 100$
PASSENGI	ER CARS A	ND TRUC	KS	
Florida Kansas Louisiana Minnesota New Hampshire Pennsylvania South Dakota Utah Vermont. Washington Wisconsin	$\begin{array}{c} 86.\ 0\\ 56.\ 9\\ 81.\ 7\\ 89.\ 7\\ 25.\ 8\\ 47.\ 7\\ 73.\ 9\\ 50.\ 1\\ 25.\ 3\\ 75.\ 7\\ 71.\ 9\end{array}$	$\begin{array}{c} 8.5\\ 37.1\\ 15.1\\ 7.6\\ 61.6\\ 45.1\\ 22.1\\ 41.8\\ 64.5\\ 20.4\\ 25.4\end{array}$	$5.5 \\ 6.0 \\ 3.2 \\ 2.7 \\ 12.6 \\ 7.2 \\ 4.0 \\ 8.1 \\ 10.2 \\ 3.9 \\ 2.7 $	100 100

In addition to the distribution of the number of trips in various mileage ranges, the total vehicle-miles involved in these trips have also been computed and are presented in table 13 and figure 3. Here another aspect of motor-vehicle use is shown. For passenger cars, while trips of less than 5 miles constituted 38.4 percent of the total number of trips, they accounted for but 6.6 percent of the total vehicle-miles of travel partly or wholly on rural roads. Trips of less than 20 miles, accounting for 85.0 percent of all trips, involved but 40.9 percent of the total vehicle-miles of travel. Trips classified in mileage groups from 20 miles upward were responsible for a much larger percentage of travel than of total trips. In the higher mileage brackets, trips in the range from 50 to 249.9 miles were only 4.0 percent of the total number of trips, but they accounted for 28.6 percent of vehicle-miles traveled outside city limits.

These characteristics were similar but less pronounced for trucks. Thirty-four percent of the total number of one-way truck trips was classified as extending less than 5 miles, accounting for but 4.9 percent of the total vehicle-miles of travel; and trips of less than 20 miles, or 80.3 percent of all trips, constituted 33.9 percent of the mileage traveled wholly or partially on rural roads.

TABLE 13.—Number of trips and vehicle-miles traveled by vehicles which went outside city limits in 11 States

PASSENGER CARS

Length of one-way trip from point of origin (miles)	Number	of trips	Trav	vel
0 to 4.9 5.0 to 9.9 10.0 to 19.9 20.0 to 29.9 40.0 to 39.9 40.0 to 49.9 50.0 to 99.9 100.0 to 249.9 250.0 to 499.9 250.0 to 999.9 1,000.0 and over	1,000 trips 835,059 576,295 438,222 143,765 61,923 27,784 61,232 25,743 3,628 825 489	Percent 38.4 26.5 20.1 6.6 2.8 1.3 2.8 1.3 2.8 1.2 .2 .1 (1)	Million vehicle-miles 2, 087, 6 4, 322, 2 6, 573, 3 3, 594, 1 2, 167, 3 1, 250, 3 4, 592, 4 4, 505, 0 1, 360, 5 618, 8 733, 5	Percent 6. 6 13. 6 20. 7 11. 3 6. 8 3. 9 14. 4 14. 2 4. 3 1. 9 2. 3
Total	2, 174, 965	100. 0	31, 805. 0	100.0
Т	TRUCKS	I	11	
0 to 4.9 5.0 to 9.9. 10.0 to 19.9. 20.0 to 29.9. 30.0 to 39.9. 40.0 to 49.9. 50.0 to 99.9. 100.0 to 249.9. 250.0 to 499.9. 250.0 to 499.9. 50.0 to 99.9. 1,000.0 and over.	$184,952\\138,916\\113,521\\41,855\\20,030\\10,262\\23,908\\9,911\\1,034\\110\\45$	34.0 25.5 20.8 7.7 3.7 1.9 4.4 1.8 .2 (1) (¹)	$\begin{array}{r} 462.\ 4\\ 1,\ 041.\ 9\\ 1,\ 702.\ 8\\ 1,\ 046.\ 4\\ 701.\ 0\\ 461.\ 8\\ 1,\ 793.\ 1\\ 1,\ 734.\ 4\\ 387.\ 8\\ 82.\ 5\\ 67.\ 5\end{array}$	4.9 11.0 18.0 11.0 7.4 4.9 18.8 18.3 4.1 .7
Total	544, 544	100.0	9, 481. 6	100.0
PASSENGER	CARS ANI) TRUCK	S	
0 to 4.9 5.0 to 9.9 10.0 to 19.9 20.0 to 29.9 30.0 to 39.9 40.0 to 49.9 50.0 to 99.9 100.0 to 249.9 250.0 to 499.9 500.0 to 999.9 1,000.0 and over Total.	1,020,011 715,211 551,743 185,620 81,953 38,046 85,140 35,654 4,662 935 534 	37.5 26.3 20.3 6.8 3.0 1.4 3.1 1.3 .2 .1 (!)	$\begin{array}{c} 2,550.0\\ 5,364.1\\ 8,276.1\\ 4,640.5\\ 2,868.3\\ 1,712.1\\ 6,385.5\\ 6,239.4\\ 1,748.3\\ 701.3\\ 801.0\\ \hline \end{array}$	$\begin{array}{c} 6,2\\ 13,1\\ 20,1\\ 11,2\\ 6,9\\ 4,1\\ 15,5\\ 15,1\\ 4,2\\ 1,7\\ 1,9\\ \hline \end{array}$

¹ Less than 0.1 percent.

AVERAGE TRIP LENGTH ONLY 15.2 MILES

One-way truck trips less than 50 miles long constituted 93.6 percent of all truck trips outside city limits and accounted for 57.2 percent of all truck travel performed wholly or partially on rural roads. Trips less than 100 miles long accounted for 98.0 percent of such truck trips and 76.0 percent of all truck travel on rural roads. Corresponding figures for trips less than 250 miles were 99.8 percent of the number of trips and 94.3 percent of travel. It may be noted, however, that for distances over 250 miles, the passenger car was used relatively more than the truck. Thus, passenger-car and truck trips of 250 miles or more were 0.3 and 0.2 percent, respectively, of total number of trips, while the travel generated was 8.5 percent of total passenger-car vehicle-miles, and but 5.7 percent of all vehicle-miles of travel by trucks performed wholly or partially on rural roads.

Computations have also been made in this study of the mean and median lengths of trips involving the use of roads in unincorporated areas by residents of various governmental jurisdictions. Results are given in table 14. For the purpose of this particular presentation, unincorporated areas and incorporated places with a population of 2,500 or less have been grouped together, because motor-vehicle owners resident in these two classifications were considered to have travel characteristics sufficiently similar to warrant their combination. For motorists of these smaller cities, rural roads, either primary or purely local, are approximately as easily accessible as such roads are to strictly rural motorists.

Figure 4 shows that for both passenger cars and trucks the mean and median lengths of one-way trips that extended outside city limits were greatest for the largest place of residence of the owners. Thus the mean length of trips made by passenger cars owned by residents of unincorporated areas and places of 2,500 or less inhabitants was 10.6 miles, while for residents of cities having in excess of 100,000 persons the mean length was 37.1 miles. Corresponding values for median trip lengths were 5.9 and 16.3 miles. Figures for trip lengths for trucks were somewhat higher for all places of origin except the largest cities.

The mean one-way trip length for combined passenger-car and truck travel for all governmental jurisdictions was 15.2 miles, and the median trip, 7.4 miles.

The relative effect of the size of cities on highway use is also strikingly illustrated in tables 15 and 16 and figure 5, which show the average number of trips made outside cities by motor-vehicle owners of cities of various sizes. As in previous tables, a single round trip starting inside the city and going to some place outside the city limits was considered as two one-way trips for purposes of mileage classification. Thus, the average passenger-car owner resident in cities having from 2,501 to 10,000 population went outside the city of residence for 75 round trips less than 10 miles long, or as it has been expressed in table 15, for 150 one-way trips less than 5 miles long.

Trips which extended for one-way distances of 50 miles or more were made approximately the same number of times during the year by the average passengercar operators resident in all sizes of cities. However, the average number of trips extending beyond city limits in the shorter trip-length ranges decreased rapidly with increased size of the city of residence. For example, table 15 shows that residents of cities having populations of over 100,000 made about one-half as many trips in the 20.0- to 29.9-mile trip-length range,

TABLE 14.—Length of trips traveled outside city limits by vehicles registered in the various population groups in 11 States PASSENGER CARS

				Len	gth of trips	s traveled b	y vehicles	registered	in '—					
	Unincorp areas a	oorated nd incor-		Ir	acorporated	l places hav	ving a pop	ulation of—			All inco	rporated		
State	porated having tion of 2	l places a popula- 2,500 or less	2,501 to	0 10,000	10,001 t	o 25,000	25,001 to	o 100,000	More the	in 100,000	popula more tl	tion of han 2,500	All p	laces
	Mean ²	Median ³	Mean	Median	Mean	Median	Mean	Median	Mean	Median	Mean	Median	Mean	Median
Florida Kansas Louisiana Minnesota New Hampshire Pennsylvania South Dakota. Utah Vermont Washington Wisconsin	Miles 11.4 9.6 9.9 11.4 13.0 9.8 15.9 10.8 9.3 11.5 10.9 10.6	Miles 6.3 5.0 5.5 6.1 8.3 5.9 8.6 4.8 5.7 5.8 6.4	Miles 20, 2 23, 2 17, 9 24, 9 13, 7 13, 3 25, 2 18, 0 20, 2 20, 2 20, 2 24, 5 17, 2	$\begin{array}{c} Miles \\ 9,9 \\ 11,6 \\ 7,8 \\ 11,6 \\ 8,6 \\ 7,1 \\ 8,3 \\ 8,2 \\ 9,5 \\ 8,1 \\ 12,6 \\ - \\ 8,3 \end{array}$	Miles 29, 7 22, 7 22, 1 27, 4 20, 1 15, 1 34, 2 38, 3 24, 5 30, 6 27, 9 20, 5	$\begin{array}{c} Miles \\ 14.6 \\ 15.6 \\ 11.2 \\ 12.3 \\ 11.1 \\ 8.4 \\ 10.0 \\ 15.4 \\ 11.4 \\ 13.8 \\ 13.9 \\ \hline 9.9 \end{array}$	Miles 30.5 34.6 26.6 27.2 19.7 60.9 38.8 26.2 33.2 24.4	Miles 14, 9 16, 0 13, 5 16, 7 9, 4 26, 9 18, 9 14, 7 8, 4 11, 7	Miles 22.8 41.0 74.2 54.3 30.8 52.9 40.8 48.2 37.1	Mites 10.0 18.5 57.5 19.7 13.6 24.0 20.0 25.8 16.3	Miles 23, 5 29, 6 28, 3 34, 7 18, 5 17, 5 30, 9 34, 2 21, 6 30, 6 31, 2 22, 9	$\begin{array}{c} Miles\\ 11,3\\ 14,6\\ 12,6\\ 15,1\\ 9,9\\ 8,5\\ 9,9\\ 14,3\\ 9,8\\ 14,1\\ 15,9\\ 10,0\\ \end{array}$	Miles 16, 1 13, 3 14, 2 16, 4 15, 5 13, 5 18, 7 17, 4 11, 7 14, 6 15, 9 	Miles 8, 1 6, 3 6, 5 7, 3 8, 9 7, 1 8, 7 6, 6 6, 5 6, 7 7, 9 7, 2
						TRUC	KS							
		·												
Florida Kansas Louisiana Minnesota New Hampshire Pennsylvania	$ \begin{array}{r} 15.3 \\ 10.9 \\ 12.0 \\ 15.1 \\ 12.1 \\ 11.9 \\ \end{array} $	7.7 6.2 7.4 7.5 8.2 6.5	$ 18.1 \\ 19.9 \\ 28.7 \\ 26.6 \\ 11.4 \\ 10.6 $	9,510,121,311,77,45,0	$\begin{array}{c} 32, \ 3\\ 29, \ 5\\ 42, \ 6\\ 38, \ 1\\ 24, \ 6\\ 12, \ 6\end{array}$	$ \begin{array}{r} 15.7 \\ 13.3 \\ 30.6 \\ 20.2 \\ 13.7 \\ 6.8 \\ \end{array} $	21. 8 47. 0 57. 6 32. 9 18. 1	$ \begin{array}{r} 13.2 \\ 30.0 \\ 36.8 \\ 19.3 \\ 8.4 \\ \end{array} $	$ \begin{array}{r} 20. \ 6 \\ 54. \ 4 \\ 52. \ 0 \\ 36. \ 1 \\ \hline 19. \ 9 \end{array} $	9,5 29,1 27,1 11,8 9,4	$25.8 \\ 31.7 \\ 44.8 \\ 33.8 \\ 20.6 \\ 14.2$	$ \begin{array}{c} 10. \\ 14. \\ 26. \\ 13. \\ 10. \\ 0 \\ 6. \\ 9 \end{array} $	$ \begin{array}{r} 19.4 \\ 17.0 \\ 21.5 \\ 21.1 \\ 16.1 \\ 13.0 \\ \end{array} $	8 7 7,8 9,9 8,7 8,9 6,7
South Dakota. ['tah	21.7 16.0 11.9 12.6	$ \begin{array}{c} 10.5 \\ 5.2 \\ 7.7 \\ 6.5 \\ 6.9 \end{array} $	37.3 14.4 14.9 18.3 21.2	$ \begin{array}{c c} 17.6 \\ 5.4 \\ 6.3 \\ 9.7 \\ 11.6 \end{array} $	$ \begin{array}{c} 64.8\\ 20.3\\ 45.0\\ 32.5\\ 31.0 \end{array} $	$\begin{array}{c} 44.7 \\ 12.0 \\ 13.5 \\ 21.3 \\ 16.1 \end{array}$	$ \begin{array}{r} 66.9 \\ 41.8 \\ 24.8 \\ 33.0 \end{array} $	45.3 22.5 15.4 16.5	58.4 45.4 35.9	20.0 31.3 19.0	$ \begin{array}{c} 52.5\\ 28.8\\ 21.2\\ 31.6\\ 29.2 \end{array} $	$ \begin{array}{c} 28.0 \\ 9.7 \\ 7.4 \\ 19.2 \\ 15.0 \end{array} $	$ 28.7 \\ 19.9 \\ 14.2 \\ 17.5 \\ 16.5 $	12.9 6.4 7.6 8.4 8.3
A verage	12.8	7.0	17.6	8.6	24. 8	11.2	29.7	13. 7	35.6	15.3	26.0	11.1	17.4	8.1
				РА	SSENGE	R CARS	AND TR	UCKS						
A verage	11.1	6. 1	17.3	8.4	21.3	10. 0	25. 4	12.0	36. 7	16.0	23. 6	10. 2	15.2	7.4

¹ This is the one-way distance of all trips. A trip from Washington, D. C., to Baltimore, Md., and return would be considered as 2 trips of 40 miles each. ² The mean shows the arithmetical average length of all trips.

³ The median indicates the length of that trip below and above which equal numbers of trips occur.

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FIGURE 4.—MEAN AND MEDIAN LENGTHS OF ONE-WAY TRIPS THAT WENT OUTSIDE OF CITY LIMITS BY VEHICLES REGIS-TERED IN VARIOUS POPULATION GROUPS.

TABLE 15.—Average number of one-way trips of various lengths traveled outside city limits by passenger cars registered in various population groups

Length of one-way trip in miles	Average nu passenger lations of-	mber of one cars register	e-way trips ed in cities h	traveled by aving popu-
from point of origin	2,501 to 10,000	10,001 to 25,000	25,001 to 100,000	More than 100,000
0 to 4.9	150	84	40	14
5.0 to 9.9.	126	84	56	28
10.0 to 19.9	96	92	60	34
20.0 to 29.9	- 38	32	30	16
30.0 to 39.9	18	18	. 14	10
40.0 to 49.9	8	10	6	4
50.0 and over	26	28	28	22
Total.	462	348	234	128

TABLE 16.—Percentage of trips of various lengths traveled outside city limits by passenger cars registered in various population groups

Length of one-way trip in miles	Percentage registere	of trips tra d in cities ha	veled by pa ving populat	ssenger cars lions of
from point of origin	2,591 to 10,000	10,001 to 25,000	25,001 to 100,000	More than 100,000
0 to 4.9 5 0 to 9.9. 10.0 to 19.9. 20.0 to 29.9. 30.0 to 39.9. 40.0 to 49.9. 50.0 and over.	$\begin{array}{c} 32.5\\ 27.3\\ 20.8\\ 8.2\\ 3.9\\ 1.7\\ 5.6\end{array}$	24. 124. 126. 49. 25. 22. 98. 1	17. 123. 925. 612. 86. 02. 612. 0	10. 9 21. 9 26. 6 12. 5 7. 8 3. 1 17. 2
Total	100.0	100.0	100.0	100. 0

This smaller number of short trips by vehicles owned in the larger cities is to be expected because of the greater area covered by the larger cities. Since the analysis involved only those trips that extended beyond city limits, a large number of the shorter trips made by residents of large cities did not extend beyond the city limits and are not included in those trips shown here. It is probable that vehicle owners resident in the larger cities make as many, or possibly more, individual trips per year as do residents of the smaller cities. Many of these trips, however, are confined within the rather extensive city limits.

DATA EXPLAIN TRAFFIC CONGESTION NEAR LARGE CITIES

These data should not be considered as evidence that the vehicle owner in smaller cities makes more trips per year than does the owner resident in the larger cities. Rather, the data are an indication that the rural highway is of greater interest to the vehicle owner of large cities for long trips than for short ones, and that the rural highways serve vehicle owners resident in the smaller cities for local travel purposes to a much greater extent proportionally than they do residents of large That is, for those trips extending to rural porcities. tions of the highway system there is proportionally a greater interest in longer trips by the residents of a large city than by the residents of small places. Table 16 illustrates this point. Passenger-car owners resident in cities of 2,501 to 10,000 population made 32.5 percent of their trips involving rural highways within the 0 to 4.9-mile trip-length range, while the residents of cities of over 100,000 population made only 10.9 percent of The their out-of-city trips within that travel range. percentages for trips of 50 miles or more one way were 5.6 and 17.2 percent, respectively.

These trip-length data indicate that much of the dense traffic often resulting in congestion on rural portions of highways near city limits is composed of a multitude of cars making short trips originating within the city. Heavily traveled sections of highway extend greater distances from the limits of large cities than from smaller cities because of the greater con entration of vehicles in the city and also because of the higher percentage of longer trips.

Facts derived from road-use data provide important guidance in outlining future highway policies, in regard to both physical and financial plans. The extent and location of the improvements made on the primary highway system are of considerable importance to all residents of the State. Except for those who live in the largest cities, all motorists in the State use the primary highway system more than any other class of roads. The condition of this sytem, therefore, is of comparable interest to all motorists except those residing in the largest cities. The latter do the greatest part of their traveling on city streets. On the other hand, it is significant that these motorists, resident in large cities, because of their large numbers, are responsible for a considerable amount of the total travel on primary highways. Therefore, their interest in such roads, although comparatively less per motorist than for other residents of the State, is still very large in the aggregate.

(Continued on page 62)

A NEW VIBRATORY MACHINE FOR DETERMINING THE COMPACTIBILITY OF AGGREGATES

BY THE DIVISION OF TESTS, BUREAU OF PUBLIC ROADS

Reported by J. T. PAULS, Senior Highway Engineer, and J. F. GOODE, Junior Highway Engineer

THE IMPORTANCE of compaction in highway construction has long been recognized. Recent laboratory and field investigations have repeatedly emphasized the value of thorough consolidation in both the base and surfacing courses. Thorough compaction is known to produce the following desirable results:

1. It increases interlocking of the aggregate particles, which is the primary factor in developing a high degree of stability.

2. It retards the entrance of moisture, thus preventing excessive loss of stability under adverse service conditions.

3. It reduces the flow of air and water through bituminous mixtures and is therefore an effective means of lessening damage from weathering and film stripping.

In order to obtain consistently a high degree of consolidation during construction, it is essential to know in advance the limits of compactibility of the materials used. Such tests as have been employed to determine the attainable density of materials, among which are dry rodding, shaking, and various molding tests involving tamping and direct compression, do not always give consistent results. Furthermore, as will be shown in this report, they fail to show the maximum compactibility limits of many aggregates.

The Bureau has been using for some time a small vibrator ¹ called the voids determinator for the determination of voids in sheet asphalt aggregates. This vibrator, however, does not give consistent results for mixtures containing high percentages of dust; and, since the testing cylinder has a capacity of only 25 cubic centimeters, it is not suitable for testing aggregates containing large fragments. Accordingly, a new machine has been developed that produces more consistent results and higher densities, and which appears to be equally satisfactory for all gradations of aggregates commonly used in both base and surface construction.

APPARATUS CONSISTS ESSENTIALLY OF A VIBRATING TABLE

The general appearance of the newly developed test apparatus is shown in the cover illustration. The principle of its operation is more clearly brought out in figure 1.

The machine consists essentially of a floating table that is made to move vertically in periodic motion by rotating eccentric masses rigidly connected to its lower surface. The table is a steel plate 13 by 24 inches in size and ¾ inch thick. It is supported at each corner by a helical spring through which there is a vertical guidepost on which the table slides.

On the lower surface of the table, mounted parallel to the long axis of the plate, are two shafts running in ball bearings and geared to rotate at the same speed but in opposite directions. Four steel blocks of equal size and weight are symmetrically mounted at the ends of the two shafts, one at each end of each shaft. The size of these blocks and the speed at which they are rotated determine the magnitude of the unbalanced force. Since the two shafts rotate in opposite directions only vertical accelerations are imparted to the system.

The weight shafts are rotated at speeds of 4,300, 2,500, or 1,500 revolutions per minute by a 3-horse-power electric motor with a 3-speed, V-belt drive.

By trial it was found that the best compaction was obtained with a total eccentric weight of 1,100 grams located $1_{16}^{\prime\prime}$ inches off center and rotating at 4,300 revolutions per minute. For these particular conditions the maximum centrifugal force developed by each of the four eccentric masses is theoretically about 338 pounds. In the extreme upper and lower positions these forces add to give a theoretical total vertical resultant of about 1,350 pounds while at the midpoint between these positions the forces developed by the weights on one shaft exactly balance those of the other shaft and the total horizontal resultant is 0 pound.

At a frequency of 4,300 cycles per minute a powerful vibration is developed in the entire mass.

The assembly for holding the aggregate to be tested is bolted to the top of the vibrating plate or table. It is shown in section in figure 2. Its essential parts are a base plate and bottom plunger bolted to the table, a cylinder fitting over the bottom plunger and resting on a rubber support, and a top plunger which rests on the test material in the cylinder.

A micrometer dial mounted on a suitable base is used in conjunction with a series of calibrated gage blocks to measure the thickness of the compacted specimen without removing it from the cylinder.

The top plunger imposes a dead load of 1.75 pounds per square inch on the sample to be compacted. This dead load generally provides sufficient confinement to flatten the top of the specimen and to prevent segregation of the particle sizes. Both the top and bottom plungers have just sufficient clearance within the cylinder to allow free vertical movement during vibration, and each is fitted with three bronze guide strips to maintain it in a position parallel to the axis of the cylinder. The loss of fine aggregate is held to a minimum by the insertion of close-fitting pasteboard gaskets or pads above and below the test specimen. A suitable correction is made in the measured height of the specimen to allow for the final thickness of the pads.

EQUIPMENT ADAPTABLE FOR TESTING DIFFERENT AGGREGATES

In making a test, the first step is to obtain an initial or zero reading with the micrometer dial on the combined height of the two plungers with the two pasteboard pads compressed between the plungers by vibration for a short period. For this zero reading a steel spacer gage of the approximate thickness of a compacted specimen is inserted under the dial so that its $\frac{3}{4}$ - or

⁴ Research on Bituminous Paving Mixtures, by W. J. Enmons. PUBLIC ROADS, vol. 7, No. 10, December 1926.



FIGURE 1.—ESSENTIAL ELEMENTS OF THE VIBRATORY COMPACTOR.

1-inch range of travel will not be exceeded when the specimen is in place.

For tests in the 4-inch cylinder, which is the one used for aggregates up to about 1-inch maximum size, sufficient aggregate is used to produce a compacted specimen approximately $1\frac{1}{2}$ to $1\frac{3}{4}$ inches high. This requires about 750 grams of aggregate.

If desired, a much smaller cylinder may be used when testing fine aggregates such as soil, sand, rock dust, or sheet asphalt aggregate, and the depth of the compacted specimen may be reduced to 1 inch or less and its weight to as little as 75 grams. For very large aggregates, a larger cylinder should be used and the thickness of the compacted specimen should be increased so that it is at least one-half to three-fourths inch more than the nominal diameter of the largest individual aggregate particle. The weight of the top plunger should be such that the dead load is approximately 1 pound per square inch per inch of depth of the compacted specimen.

It is essential that the loose aggregate be placed in the cylinder without segregation. When the aggregate to be tested has a large percentage retained on the No. 10 sieve it has been found that the addition of about 50 to 70 cubic centimeters of kerosene to 750 grams of aggregate aids greatly in preventing segregation and does not interfere with compaction. The most satisfactory amount of kerosene seems to be that which will just fill the voids in the compacted aggregate.

Materials such as fine soil, sand, clay, etc., are not particularly subject to segregation and, because of the greater difficulty with which air is forced out of them when wet, do not always compact as well with kerosene as without. They are therefore tested dry.



Figure 2.—Cylinder and Plunger Assembly With Measuring Device.

For determining whether or not to use kerosene it has been found that in general the following conditions will govern:

1. For aggregates having less than 35 percent passing the No. 10 sieve, use kerosene.

2. For aggregates having more than 50 percent passing the No. 10 sieve, test dry.

3. For aggregates having more than 35 percent and less than 50 percent passing the No. 10 sieve, test both with and without kerosene and report the higher density value obtained.

VIBRATION FOR 20 MINUTES ADOPTED AS STANDARD PROCEDURE

All aggregates should be oven-dried before testing, since very small amounts of water or other liquid, as distinguished from the relatively large amount of kerosene added in testing coarse materials, have a marked bulking effect which interferes with the obtaining of accurate test results. Drying is also necessary in order to obtain the true sample weights for use in calculating the density after vibration. Lumps or clods of clay in the aggregate impair the accuracy of the test and should be thoroughly broken down before placing the sample in the cylinder for compaction.

After the material is placed in the cylinder, with a pasteboard pad underneath and another on top, the upper plunger is inserted and the assembly is vibrated for a period of 20 minutes. The final reading is taken with the dial, and from this reading and the initial reading the over-all volume of the material in the cylinder is calculated. This volume, the dry weight, and the apparent specific gravity of the aggregate are used in calculating the density. In this report density is expressed as the percentage of aggregate volume per unit of total volume.

The method of determining this percentage is illustrated with a typical example:

Apparent specific gravity ² of aggregate	2.67
Weight of aggregate sample, grams	736
Volume of vibrator-compacted sample, cubic centimeters_	313.9
Unit weight of compacted sample (grams per cubic centi-	
meter) 736/313.9	2.35
Density of compacted sample, (percent) $\frac{2.35}{2.67} \times 100$	88. 0

The densities of a number of aggregates were determined for various periods of vibration up to a maximum of 60 minutes. The results of these tests are shown in figure 3. The asphaltic concrete aggregate, the sheet asphalt aggregate, and the fine sand showed practically no increase in density after 20 minutes of vibration. The sand-clay and the sand-clay-gravel each showed an apparent increase of 1.1 percent in density for the time increment from 20 to 60 minutes, and the micaceous soil showed an increase of 1.4 percent. It was found, however, that loss of dust, which became guite noticeable late in the test because of wear on the gaskets, accounted for most of the reduction in volume and consequent apparent increase in density after the initial 20 minutes of vibration. Vibration for a period of 20 minutes has, therefore, been adopted as regular procedure for the test.

The results of compaction tests on three different types of aggregate are shown in table 1 and demonstrate the ability of the apparatus to produce results that check. The maximum variation in results for these tests was slightly under 0.5 percent. However, for routine testing by various operators, this degree of accuracy probably could not be expected.

 TABLE 1.—Consistency of check tests using the vibratory compacting machine

Type of aggregate	Density (aggregate volume per unit of total volume)									
I ype of aggregate	Test No. 1	Test No. 2	Test No. 3	Test No. 4	Aver- _age					
Sand-clay Sand-clay-gravel Sheet asphalt (sand and dust)	Percent 79.4 86.7 76.8	Percent 79.6 86.9 76.6	Percent 79.6 86.7	Percent 79.6 87.1	Percent 79.6 86.9 76.7					

In the following tables and discussion, the results of a number of compaction tests using the vibratory machine and several other methods of compaction are shown. Table 2 shows the comparative effects of vi-

² Standard Definitions of Terms Relating to Specific Gravity, A. S. T. M. Designation E12-27.



FIGURE 3.—DENSITIES OF VARIOUS TYPES OF AGGREGATES AND THEIR RATES OF CONSOLIDATION.

bration and direct compression on the grading of the aggregate and demonstrates that little or no change in grading was produced by the vibratory method of compaction, whereas direct compression resulted in sufficient crushing to alter materially the grading of the aggregate samples.

HIGHER DENSITIES OBTAINED BY VIBRATORY METHOD THAN BY OTHER METHODS

A comparison of densities obtained by several methods of compaction on various types of aggregates is shown in table 3. In the upper section of the table dealing with the aggregates for base courses, the densities obtained from circular-track test sections built and compacted under the most favorable laboratory conditions agree closely with those obtained by the vibratory

TABLE 2.-Effect of compaction by compression and vibration on breakage of various aggregates

				Grading	total aggregate passing— No. 4 No. 10 No. 40 No. 100 No. 200 Sieve Sieve Sieve Sieve Sieve Sieve Percent Percent Percent Percent Percent Percent 100.0 99.7 82.5								
Type of aggregate	Method of compaction	1-inch sieve	¾-inch sieve	3%-inch sieve	No. 4 sieve	No. 10 sieve	No. 40 sieve	No. 100 sieve	No. 200 sieve				
Graded, fine—high dust content Do	None Vibration	Percent	Percent	Percent	Percent 100.0 100.0	Percent 99. 7 99. 7	Percent 82.5 82.2	Percent	Percent 15.5 14.7				
Graded, fine—low dust content Do	None				100. 0 100. 0	99.4 99.8	82.3 82.8		2.4 2.8				
Do Do	None Compression, 3,000 lb./sq. in		, 			100. 0 100. 0	75. 2 76. 9	18.9 25.6	2.2 7.5				
Graded, coarse—medium dust content Do	None	100 100	92.5 92.5	75. 8 75. 8	66. 8 66. 6	63.7 63.3	48.6 48.3		9.4 9.4				
Graded, coarse—low dust content Do	None Compression, 3,000 lb./sq. in	100 100	98.3 98.2	76. 7 82. 0	55. 9 61. 6	50. 0 52. 0	38.5 39.8	11.4 14.4	1.8 4.1				
Graded, coarse—high dust content	None Compression, 3,000 lb./sq. in	100 100	90. 8 92. 5	77. 0 80. 2	66.3 69.8	47.2 53.2	31.7 36.0		17.3 21.0				

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The other methods of compaction shown, with test. few exceptions, gave considerably lower densities. Direct compression appears to be quite effective for the fine-grained materials, but, as previously shown, the crushing of the aggregate in this test renders the results somewhat unsatisfactory.

TABLE 3.—Comparison of aggregate densities obtained by various methods of compaction

BASE COURSE MATERIALS¹

Character of material		Densit u	y (aggreg nit of tot	gate volu al volum	me per e)	
		1	Sam-	sity (aggregate volume j unit of total volume) Aggregates compac in laboratory ut Vibra- tory vit trov ter pres- vio b, Sq. nt Percent Percent Per 4 57.3 8 57.5 0 57.0 8 80.6 8 81.4 2 81.4 1 78.5 8 80.7 8 80.7 8 82.2 8 88.7 8 88	npacted pry	
Type Micaceous soil. Micaceous soil with 3 percent coment. Micaceous soil with 11 percent cement Sand-clay. Do. Do. Do. Do. Do. Do. Do. Do.	Plas- ticity- index	Pass- ing No. 200 siøvø	ples cut from road or test track	Vibra- tory method	Com- pres- sion 3,000 lb./sq. in.	Voids de- termi- nator
Micacoous soil	0	Percent 62	Percent 53.4	Percent 57.3	Percent	Percent
cement	0	63	57.8	57.5		
Micaceous soil with 11 percent	0	65	57.0	E 7 0		
Sand-clav	0	25	74.8	80.6	75.3	80.2
Do	5	26	80.8	80.8	81.4	76.5
Do	, 6	20	81.2	81.4	78.8	75.6
Do	1 13	27	79.5	79.6	81.4	75.1
Do.	18	29	76.2	76.3	82.2	71.1
Sand-clay-gravel	0	17	82.8	89.7		
Do	0	1	77.8	86.9	76.7	
Do.	. 5	15	87.0	88.0 80.0	 82 0	
Do	7	22	87.3	87.5	83.2	
Do	8	12	89.1	89.9	84 2	
Do.	9	25	83.9	86.7	84.1	
D0	16	16	80. 2 84. 0	87.1 87.2		
HOT, PLANT-M	IIX SUI	RFACIN	G MAT	ERIAL	S 2	
Filesterminalt D. C.			-			
Sheet asphalt, D. C		14.9	70.7	76.5		71.4
Fine bituminous concrete. Ohio.		4.7	82.0	85.9		
D0		6.0	77.9	80.5		
Medium bituminous concrete, O	hio	4.1	76.4	80.6		
Coarse bituminous concrete Obi	0	0.7	81.3	88.1		
Do		4.2	81.6	89 2		

 Samples taken from circular track test sections.
 Field samples from pavements. Laboratory compaction tests made on extracted aggregates.

The densities obtained by means of the new vibratory machine are in general much higher than those obtained by the voids determinator.³ The new apparatus has the further advantage of permitting the testing of largesize aggregates. The voids determinator used in previous work of the Bureau is not suitable for testing materials larger than those passing the No. 10 sieve.

The lower section of table 3 shows a comparison between the aggregate densities of asphaltic pavements of the hot-mix type and the densities obtained by vibrating the extracted aggregates from these pavements. The data shown indicate that construction operations and traffic may not generally produce as high densities in hot-mix pavements as are produced by vibrating the dry aggregates. The highly viscous binders apparently resist the free adjustment of the aggregate particles to form their densest possible arrangement. This resistance is known to be considerably less for the liquid binders than for the highly viscous ones. Mixtures containing the liquid materials often attain densities closely agreeing with the vibratory test results, which accounts for the fact that such mixtures cannot safely

³ Research on Bituminous Paving Mixtures, by W. J. Emmons. Public Roads, vol 7, No. 10, December 1926.

be made as rich in bituminous material as hot paving mixtures of comparable aggregate grading.

To attain consistently during construction a satisfactory degree of compaction for any particular material it is necessary to know in advance its compactibility limit, to have an idea of how closely this limit may be approached by practical construction methods, and how closely it needs to be approached to insure satisfactory behavior provided the materials are otherwise satis-Tests have been made on a large number of factory. materials. The few typical results given in table 4 illustrate the relations between field densities and compactibility limits, as determined by the vibratory machine.

For the plastic sand-clay and sand-clay-gravel materials that have been found by various tests to be suitable for base-course construction, the compaction obtained during construction appears to be the deciding factor influencing service behavior. The importance of consolidation is particularly well illustrated in the behavior of the plastic sand-clay-gravel referred to in the footnote of table 4. This material, which is representative of a group of materials that showed similar behavior, was placed in the test section as a base course with insufficient moisture to permit compaction to the density obtained in the vibratory test. It failed in service as soon as unfavorable sub-base moisture conditions were imposed. It was later scarified and recompacted with a higher moisture content. It was then easily compacted to essentially the same density as was obtained in the vibratory test and gave excellent service under very adverse moisture conditions.

TABLE 4.—Relation of density of soil-type bases to service behavior for base course materials

Character of r	nateria	l teste:	1		Der (aggr	sity egate	
	index	Gra aggrøg	ding, t ate pas	otal sing—	unit o volu	f total me)	Behavior in test sections
Турө	Plasticity	No. 10 sieve	No. 40 sieve	No. 200 sieve	Vibratory compac- tion	F i e l d compac- tion	
		Per-	Per-	Per-	Per-	Per-	
Micaceous soil Micaceous soil with 3 per-	0 0	<i>cent</i> 98 98	<i>cent</i> 76 77	62 63	<i>cent</i> 57.3 57.5	cent 53.4 57.8	Unsatisfactory. Satisfactory.
Micaceous soil with 11	0	98	78	65	57.0	57.0	Do.
percent coment. Sand-clay Do Do	0 6 9	$ \begin{array}{c} 100 \\ 100 \\ 100 \end{array} $	$71 \\ 51 \\ 68$	$25 \\ 20 \\ 27$		$74.8 \\ 81.2 \\ 79.5$	Do. Do. Fairly satisfac-
Sand-clay-gravel Do.1-(Same material) ¹	$\begin{pmatrix} 0\\ 5\\ 5 \end{bmatrix}$	$54 \\ 48 \\ 48 \\ 48$	$34 \\ 31 \\ 31 \\ 31$	17 15 15	89.7 88.0 88.0	82. 8 83. 2 87. 0	Satisfactory. Unsatisfactory. Satisfactory.

¹ When this material was placed in the roadway it had so low a moisture content that it did not compact to a satisfactory density. It failed early in service but when remixed and relaid with the corract moisture content, it compacted to within 1 per-cent of the density obtained by vibration and gave satisfactory service.

VIBRATORY METHOD USEFUL IN BLENDING AGGREGATES TO OBTAIN DENSE MIXTURE

In highway base- and surface-course construction it is frequently necessary to blend two or more aggregates to provide a material suitable for the intended use. The vibratory compactor provides a means by which the best combination of two or more available aggre-gates may be determined. The application of this test to the design of aggregate blends and bituminous mixtures will be discussed in connection with figures 4, 5, 6, and 7.

Figures 4 and 5 illustrate two methods of using the vibratory compactor to obtain the densest combination



FIGURE 4.—BLENDING CURVES FOR CRUSHED GRAVEL, SAND, AND PULVERIZED SOIL. AGGREGATE NUMBERS CORRESPOND TO THOSE IN TABLE 5.

of three different aggregates for use as a base-course material. For this type of construction the combination of the available materials that gives the densest mixture is generally the most desirable. The three aggregates used in producing the blending curves of figures 4 and 5 were crushed gravel (1-inch maximum size), fine sand, and a pulverized soil. The densities and the gradings of the individual constituents and the various blends are shown in table 5.

In the first method illustrated in figure 4, an initial series of blends was made of the gravel and sand and the densest blend of these two materials was determined. This blend, designated as aggregate 3 in table 5, was then blended in various proportions with the pulverized soil, the densest blend in this series being presumably the densest possible blend of the three materials. This blend, designated aggregate 9 in table 5, had a density of 87.9 percent aggregate solids and the following composition: Gravel 72 percent, sand 18 percent, and pulverized soil 10 percent.





A less dense blend of the gravel and sand, selected at random and designated aggregate 4 in table 5, was also blended with the pulverized soil as shown in figure 4. The highest density obtained by blending with aggregate 4 was lower than that obtained with aggregate 3, indicating that the procedure of selecting the densest combination of the coarse materials for blending with the fines was the correct method.

In the second method, illustrated in figure 5, the order of the tests was reversed. The initial series of blends was made with the sand and pulverized soil. The densest blend of these, designated aggregate 19 in table 5, was then blended with the gravel. The two methods gave identical final results both as to maximum density and proportions of the three constituents, the density at the high point of the second curve being again 87.9 percent and the proportions of material being: Gravel 72 percent, sand 18 percent, and soil 10 percent.

It is of interest to note that the grading of the densest blend of these three materials, which were selected more

	С	ompositior	a of aggregate		Density (aggre-		Grading,	total aggre	egate passir	ng	
Identification	Coarse		Fine		gate volume per unit	¾-inch	3%-inch	No. 4	No. 10	No. 40	No. 200
	Туре	Amount	Туре	Amount	of total volume)	sieve	sieve	sieve	sieve	sieve	sieve
1	Country of an anal	Percent	Care d	Percent	Percent	Percent	Percent	Percent	Percent	Percent	Percent
0	crushed gravel	100	Sand	15	11.9	82.0	48.0	24.9	15.7	12.0	6
£	do	80	do	10	82.0	95.6	50.2	30.2	20.6	12.0	.0
A	do	70	do	20	82.0	87 4	64 0	47 A	30.6	23.8	1 3
5	do	50	do	50	79.6	91.0	74.3	62.5	50.4	39.4	2.2
6	do	25	do	75	75.6	95.5	87.1	81.2	75.2	59.0	3.2
	do	0	do.	100	69.6				100. 0	78.5	4.3
3	Aggregate No 3	100	Pulverized soil	0	82.8	85.6	58.8	39.9	20.6	15.9	. 9
8	do	95	do	5	87.0	86.3	60.9	42.9	24.6	20.1	5.1
9	do	90	do	10	87.9	87.0	62.9	45.9	28.5	24.3	9.2
10	do	75	do	25	86.6	89.2	69.1	54.9	40.5	36.9	22.1
11	do	40	do	60	78.3	94.2	83.5	76.0	68.2	66, 4	51.7
12	do	0	do	100	66.2					100.0	85.6
4	Aggregate No. 4	100		0	82.1	87.4	64.0	47.4	30.6	23.8	1.3
13	do	95	do	5	86.2	88.0	65.8	50.0	34.1	27.6	5. 5
14	do	90	do	10	87.6	88.7	67.6	52.7	37.5	31.4	9.7
15	do	75	do	25	86.1	90.6	73.0	60.6	48.0	42.9	22.4
16	do	40	do	60	78.1	95.0	85.6	79.0	72.2	69.5	51.9
12	do	0	do	100	66.2	******				100.0	85, 6
7	Sand	100	do	0	69 6				100.0	78.5	4.3
17		85	do	15	75.8				100.0	S1.7	16. 5
18	do	70	do	30	79.5				100.0	85.0	28.7
19	do	65	do	35	79.8				100.0	86, 0	32.7
20	do	50	do	50	78.6				100.0	89.3	45.0
21	do	25	do	75	73.3				100.0	94.6	65.3
12	do	0	do	100	66.2					100.0	85.6
1	Crushed gravel	100	Aggregate No. 19	0	77.9	82.0	48.5	24.9	. 8	. 3	0
22	do	85	do	15	86.3	84.7	56.2	36.2	15.7	13.2	4.9
23	do	72		28	87.9	87.0	62.9	45.9	28.5	24.3	9.2
24	do	40	do	60	84.7	92.8	79.4	70.0	60.3	51.7	19, 6
19	do	0	do	100	79.8				100.0	86.0	32.7

TABLE 5.—Densities and gradings of blended aggregates, sand-clay-gravel base-course type



FIGURE 6.—BLENDING CURVES FOR BITUMINOUS CONCRETE. Aggregate Numbers Correspond to Those in Table 7.

or less at random, conformed to the grading requirements now recommended for base-course construction. This relationship is shown in table 6.

In figure 6 is shown the application of method 1 in blending crushed stone, sand, and limestone dust for a typical bituminous concrete aggregate. The densities and gradings of the various blends are shown in the lower section of table 7. As shown in figure 6 a maximum density of 89.8 percent solids was obtained, using

 TABLE 6.—Comparison of grading obtained by blending sand, clay, and gravel for maximum density, with recommended grading requirements for base-course construction

		Gra	ding, tot	al aggreg	ate passi	ng—	
	1-inch sieve	¾-inch sieve	3%-inch sieve	No. 4 sieve	No. 10 sieve	No. 40 sieve	No. 200 sieve
Manimum density	Percent	Percent	Percent	Percent	Percent	Percent	Percent
A. A. S. H. O. speci-	100	87.0	62.9	45.9	28.5	24.3	9.2
fication for type B, sand-clay-gravel base-	100	70-100	50-80	35-65	25-50	15-30	5-15





15 percent of limestone dust with the densest blend of the stone and sand (aggregate 35, table 7).

VIBRATOR ENABLES DESIGN OF MIXTURES WITHOUT OVERFILLING VOIDS

Here is an example where too dense an aggregate for practical use in bituminous concrete was obtained since the voids remaining would only permit the use of about 10 percent by volume or approximately 5 percent by weight of asphalt. To produce a practical aggregate it would be necessary to reduce its density. This would best be accomplished by reducing the dust content since the densest possible combination of the coarse fractions is always desirable. Reduction of the dust content to range between 3½ and 6 percent would reduce the density to between 85 and 86.5 percent solids, thus permitting the use of approximately 6 to 7 percent asphalt by weight and bringing the design into line with established practice.

Figure 7 shows a blending curve for bituminous concrete aggregate composed of crushed stone and artificial limestone sand without dust. This type of aggregate is used extensively in Ohio. The densities and gradings of the constituents and blends are shown in the upper section of table 7. This type differs from the previous

TABLE 7.—Densities and gradings of blended aggregates of the type used in bituminous concrete

WITHOUT MINERAL FILLER

	C	omposition	of aggregate		Density (aggre-	Grading, total aggregate passing-							
Identification	Coarse		Fine		gate volume per unit	34-inch	3%-inch	No. 4	No. 10	No. 40	No. 200		
	; Туре	Amount	Туре	Amount	of total volume)	sieve	sieve	sieve	sieve	sieve	sieve		
25	Crushed stone Percent		Artificial sand	Percent 0 25 40 46 50 70 100	Percent 69. 8 85. 9 87. 3 87. 5 87. 4 85. 4 85. 4 81, 1	Percent 100 100 100 100 100 100	Percent 90.0 92.5 94.0 94.6 95.0 97.0	Percent 5. 0 28. 8 43. 0 48. 7 52. 5 71. 5 100. 0	Percent 0 23. 3 37. 3 42. 9 46. 6 65. 2 93. 2	Percent 0 9.3 14.8 17.0 18.5 25.9 37.0	Percent 0 9 1. 4 1. 6 1. 8 2. 5 3. 5		
			WITH MINER	AL FILL	ER								
32 33 34 35 36 37 37 35 38 39 40 41 42 	Crushed stone	$ \begin{array}{r} 100 \\ 85 \\ 70 \\ 60 \\ 50 \\ 0 \\ 100 \\ 90 \\ 85 \\ 80 \\ 75 \\ 0 \end{array} $	Sand do do do Limestone dust do	$\begin{array}{c} 0 \\ 15 \\ 30 \\ 40 \\ 50 \\ 100 \\ 0 \\ 10 \\ 0 \\ 10 \\ 20 \\ 25 \\ 100 \end{array}$	$\begin{array}{c} 64.5\\ 74.7\\ 81.7\\ 83.0\\ 82.4\\ 70.8\\ 83.0\\ 88.3\\ 89.8\\ 89.8\\ 89.2\\ 88.1\\ 72.4\\ \end{array}$	100 100 100 100 100 100 100 100 100	0 15.0 30.0 40.0 50.0 	$\begin{array}{c} 0\\ 15, 0\\ 30, 0\\ 40, 0\\ 50, 0\\ 160, 0\\ 40, 0\\ 40, 0\\ 49, 0\\ 52, 0\\ 55, 0\end{array}$	$\begin{array}{c} 0\\ 11.7\\ 23.4\\ 31.2\\ 39.0\\ 78.0\\ 31.2\\ 38.1\\ 41.5\\ 45.0\\ 48.4 \end{array}$	$\begin{array}{c} 0\\ 4,5\\ 9,0\\ 12,0\\ 15,1\\ 30,1\\ 12,0\\ 20,8\\ 25,2\\ 29,6\\ 34,0\\ 100,0\\ \end{array}$	$\begin{matrix} 0 \\ .3 \\ .6 \\ .8 \\ 1.0 \\ 2.0 \\ .8 \\ 10.4 \\ 15.2 \\ 19.9 \\ 24.7 \\ 96.4 \end{matrix}$		



Figure 8.—Blending Curve for Sheet Asphalt Sand and Commercial Limestone Dust. Aggregate Numbers Correspond to Those in Table 8.

example in that the densest combination of the two aggregate constituents provides sufficient void space for the bituminous material. It is therefore desirable to use the densest blend, this being easily found by means of the vibratory compactor.

The aggregate void space in the densest blend shown in figure 7 would permit the use of about 6 percent asphalt, which conforms approximately to the design used successfully in Ohio with the same type of aggregate.

The results of vibratory compaction tests on blends of fine sand and limestone dust to give a dense aggregate for sheet asphalt are shown in figure 8. The densities and gradings of the two constituents and the blends are given in table 8. A maximum density of 79.9 percent solids was obtained with the blends consisting of 70 percent fine sand and 30 percent dust and 75 percent sand and 25 percent dust. Again the 20 percent voids in this blend provide insufficient space for the proper amount of asphalt and the high dust content would produce an aggregate that would be difficult to mix and handle.

 TABLE 8.—Densities and gradings of blended aggregates of the type

 used in sheet asphalt

Comple identifi	Compo aggi	position of Density (aggre- gregate gate vol-								
cation	Sand	Lime- stone dust	ume per unit of total vol- ume)	No. 10 sieve	No. 40 sieve	No. 80 sieve	No. 200 sieve			
43	$\begin{array}{c} Percent \\ 100 \\ 95 \\ 90 \\ 85 \\ 80 \\ -75 \\ 70 \\ 50 \\ 25 \\ 0 \end{array}$	Percent 0 5 10 15 20 25 30 50 75 100	Percent 71, 2 73, 4 75, 6 77, 4 78, 9 79, 9 79, 9 79, 9 78, 5 73, 7 68, 1	Percent 100 100 100 100 100 100 100 10	Percent 81.3 82.2 83.2 84.2 85.1 86.0 86.9 90.7 95.3 100.0	Percent 33.8 37.1 40.4 43.7 47.0 50.4 53.7 66.9 83.5 100.0	Percent 3.5 8.1 12.7 17.3 21.9 26.4 31.0 49.4 72.4 95.3			

In this type of construction the problem of design utilizing the vibratory compactor might be attacked from either of two angles:

1. The amount of dust could be set on the basis of well-established practice, which would call for considerably less than 25 percent dust, and the asphalt content required to fill the void space could then be determined by vibratory tests on the fixed aggregate blend.



FIGURE 9.—VARIATION IN DENSITY OBTAINED BY VIBRATION AND CORRESPONDING ASPHALT CAPACITY OF SHEET ASPHALT AGGREGATES CONTAINING VARIOUS PERCENTAGES OF LIME-STONE DUST FILLER.

2. The amount of asphalt could be set also on the basis of well-established practice, and the amount of dust to be used could then be adjusted by vibratory compaction tests on a series of blends covering a narrow range of dust contents to produce an aggregate that would hold the fixed amount of asphalt.

Figure 9 illustrates the relation between asphalt capacity as determined by vibratory compaction tests and the dust content of the aggregate.

The use of the compaction test to coordinate content of bituminous materials and capacity for them appears to offer special possibilities in the design of dense surfacing mixtures where overfilling of the voids might seriously impair stability.

SUMMARY

As shown in the preceding discussion the vibratory test appears to offer valuable aid in connection with the following problems of design and construction:

1. Establishment of a definite optimum degree of compaction toward which field compaction may be aimed.

2. Determination of the best combination of two or more available aggregates for base-course or surface construction.

3. Investigation of the capacity for bituminous materials of certain aggregates to insure against overbituminization.

4. Modification of aggregate blends to permit the use of sufficient bituminous material for workability and surface sealing without overfilling the void spaces and destroying stability.



Figure 5.— Number of Trips Made by Vehicles Registered in Various Incorporated Places According to Length of One-Way Trips That Went Outside of City Limits.

The expenditure of motor-vehicle tax revenue on secondary highways and local roads does not create highway user benefits as widespread as those created by primary road expenditures, because these roads are used to a much smaller extent than the primary system or city streets. The use of secondary highways and local roads by residents of unincorporated areas and small towns is comparable with the use of local city streets by city residents.

The preceding data show the extensive use of motor vehicles for local travel and the self-imposed limitations on their use which results in a large percentage of their travel being performed within a surprisingly small area around their place of ownership. Accordingly, those roads radiating from centers of population are very important links in the highway system. It is apparent, then, that appreciable portions of the expenditures of motor-vehicle tax revenue on the primary system, in order to benefit the large cities properly, must be so applied as to alleviate the conditions of congestion and accompanying danger that exist within short distances of population centers.

Data on the use of rural roads and city streets and the extent of such use cannot be used alone to determine adequate plans for a highway program. Road-use data must be supplemented by data regarding the condition of existing roads, by other types of traffic data, and by financial data. For example, road-use information might point to the desirability of improving primary highway conditions in the vicinity of large cities but special traffic studies would be necessary to determine whether improvement at a particular location should consist of a by-pass route to accommodate an existing high percentage of through traffic or whether it should consist of extensions to main city thoroughfares of adequate width and design to accommodate a high percentage of local traffic together with a relatively small amount of through traffic. Studied alone, however, road-use information presents an essential picture of highway operations and a background of

travel characteristics which are extremely valuable in projecting comprehensive plans for a highway system to serve the best interests of all motorists.

SUMMARY

These preliminary analyses of road-use data indicate: 1. Use of the rural road facilities by urban motorists decreases with increase in size of the city in which they

reside. 2. Motorists residing in incorporated places perform

71 percent of all travel occurring on primary highways. 3. In the case of all motorists except those resident in cities of more than 100,000 population, more than half their annual travel occurs on primary highways. 4. Motor-vehicle use is largely comprised of short trips for passenger cars as well as for trucks.

5. A large amount of rural highway travel is occasioned by the travel of city motor-vehicle owners within short distances of their residences.

6. The proportional amount of such travel by urban residents decreases with increase in the size of the cities in which the vehicle owners reside.

7. Expenditures for rural highway facilities in the vicinity of cities, especially the larger ones, will provide proportionally greater benefits for urban than for rural motor-vehicle owners.

	FALANCE OF FUNDS AVAIL.	ABLE FOR PRO- CRAMMED PROJ- ECTS	# 3, 181, 375 1,952, 195 1,761,085	4,717,588 2,590,278 1,659,871	1, 418, 868 3, 817, 233 7, 032, 558	1,682,989 4,428,536 3,403,782	2,553,356 4,266,095 3,294,273	3,100,818 996,965 1,969,785	2,965,358 3,687,008 4,673,917	3, 246, 311 4,980, 107 4,835,423	3,004,158 1,643,938 1,457,890	2,859,493 1,764,412 5,172,368	2,893,893 4,405,888	2,712,800	1, 373, 648 2, 494, 519 3, 750, 545	4,674,044 7,873,543	2,106,061 2,106,061	3,057,039 3,370,316	1,449,500 1,449,500 502,865	159,289,123
	7	Miles	50.3 13.8 15.5	38.8	20.0 6.3 89.0	20 53 20 55 20 55 20 20 20 20 20 20 20 20 20 20 20 20 20	35.9 207.3 59.9	23.8	12.0 21.6 61.2	4.1 186.7 92.5	296.6	2.7.2	69.6 167.3	54-1-1-200	3.1 68.0	59-5 116-2	10.4 10.4 10.8	12.4	3.8	2,173.7
	FOR CONSTRUCTION	Federal Aid	€ 650,570 223,396 340,912	558,364 1,124,009 321,920	329,690 141,950 788,585	559,414 1,631,560 1,455,490	226,300 1,919,996 1,174,674	671,501 59,555 635,000	721,555 581,180 616,902	185,150 2,063,220 1,023,955	1,425,550 8,827 247,842	178,605	692, 690 777, 848	791,645	165,515	915,690 861,970 208,327	100,095 363,844	193,535 173,965 141,186	38,000 239,928 167,985	29,001,555
COJECTS	APPROVEI	Estimated Total Cost	# 1, 307, 250	1,058,749 2,156,750 644,150	695,890 283,900 1.577.150	917,503 3,263,211 3,119,099	672,451 3,855,402 2,348,952	1,380.075 119,110 1.486.470	1, 451,005 1,169,755 1,235,614	414,300 4,309,223 1,808,947	2,848,104 10,264 502,882	357,210	1,453,190 1,451,312	1,487,800 260,297	331.030	1,831,380	200,670 727,688 308,064	387,070 366,002 307,000	76,000 484,577 338,549	58,072, 344
VAY PR		Miles	282.2 48.7 228.2	72.9 85.0	9.9 53.9 270.7	45.0 170.2 69.3	146.0 184.5 59.0	42°5 33°6	24.5 125.8 264.1	394.6 76.2 74.9	463.6 63.6 2.0	26.4 98.7 192.1	389.6 57.5 78.0	47.7 100.7 88.0	3.5 86.2 1453.7	72.0 696.6 80.8	17.7 85.8 38.4	39.1 183.3 108.4	9+5 39-5	6,342.7
D HIGHW 39	R CONSTRUCTION	Federal Aid	\$ 3,845,646 874,131 3,625,843	2,804,228 1,456,727 504,690	343,194 1,343,243 2,629,605	4,009,358 1,760,638	1,843,033 2,119,283 1,375,281	2,642,130 820,728 1,353.851	1,694,505 2,051,862 2,875,292	3,435,536 1,601,196 1,086,932	2,834,730 1,552,165 77,222	1,454,438 1,396,594 5,866,776	2,992,402 243,744	1,370,385 1,370,327 1,310,954	1,276,376 2,571,990	1,813,916 7,322,071 1,710,420	343,793 1,574,219 1,593,650	3,272,880 729,771	421,240 851,255	98,436,113
ERAL-AI RIL 30, 19	UNDE	Estimated Total Cost	\$ 7,712,955 1,231,929 3,629,785	5,154,134 2,734,853 1,021,668	695,941 2,686,486 5,259,210	1,378,664 8,025,824 3,521,276	4,465,034 4,238,567 2,750,562	11, 115, 551 1, 641, 459 2, 729, 978	3,392,414 4,103,724 5,796,123	9.047,642 3.260,364 1.928,464	5,631,473 1,799,236 155,856	2,913,986 2,300,174 11,983,150	5,992,519 434,490 8,079,402	1,679,375 2,246,158 8,691,991	390, 482 2, 860, 644 4, 650, 859	3,626,389 14,871,392 2,406,783	722,784 3,155,558 3,044,447	1,636,172 6,694,362 1,186,722	859,420 1,711,401	197,247,832
)F FED vs of AP	L YEAR	Miles	239.4 118.2 86.8	240.0 99.3 8.9	14.2 64.1 260.9	200.7 307.3 154.6	263.5 727.9 209.2	38.3 65.0 17.1	9.0 172.8 304.8	211.1 156.9 83.6	362.0 168.8 23.7	18.3 242.6 253.3	259.6 260.9	254.7	16.4 266.5 246.1	182.2 898.4 107.2	211.4	66.7 176.2 281.4	18.0 16.4	8,863.9
TATUS C	RING CURRENT FISCA	Federal Aid	\$ 3,146,870 1,679,245 1,266,330	5,559,953 1,368,526 455,835	254,021 1,414,222 2,491,320	1,202,494 5,682,378 2,948,099	2,570,857 2,570,857 2,747,346	1,388,934 542.728	938,724 3,909,644 2,356,216	2,122,923 2,734,544 929,612	1,984,688 1,178,382 579,858	1,309,420 1,468,905 6,738,841	3, 189, 099 3, 233, 548	3,720,600 1,845,945 1,201,896	2, 369, 645 2, 369, 848 1, 128, 306	2,825,832 6,970,620 740,894	614,434 3,005,159 2,070,943	1,309,939 2,502,304 1,526,619	396,078 199,770	112,109,118
S	COMPLETED DUI	Estimated Total Cost	♣ 6, 865, 382 2, 283, 575 1, 280, 707	10, 198, 246 2,578, 157 974, 030	5,205,428	2,120,236 11,478,998 6.077,173	7.727.570 5.172.155 5.556.100	1,516,025 2,860,227 1.085,456	1,877,599 8,295,221 4,902,005	4,975,778 5,764,313 1,653,927	4,119,631 1,404,575 1,178,535	2,637,665 2,253,381 14,230,225	6,763,450 3,437,527 8,727,886	7,093,181 3,183,317 8,575,988	1,179,290 5,361,442 2,016,762	5,741,748 14,143,847 1 121 537	1, 295, 969 6, 022, 569 4, 036, 518	1,851,632 5,069,889 2,526,025	809,490 408,596	219,007,427
		STATE	Alab ama Arizona Arkanaas	California Colorado Connecticut	Delaware Florida Georgia	Idaho Illinois Indiana	lowa Kansas Kentucky	Louisiana Maine Mary'land	Masachusetts Michigan Mimesota	Mississippi Missouri Montana	Nebraska Nevada New Hampshire	New Jersey New Mexico New York	North Carolina North Dakota Ohio	Oklahoma Oregon Pernsylvania	Rhode Island South Carolina South Dakota	Tennessee Texas Utah	Vermont Virginia Washington	West Virginia Wisconsin Wyoming	District of Columbia Hawaii Puerto Rico	TOTALS

PUBLIC ROADS

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STATUS OF FEDERAL-AID SECONDARY OR FEEDER ROAD PROJECTS

AS OF APRIL 30, 1939

* 833 746 * 400 529 874,060 874,060 874,060 286,549 286,249 288,393 289,500 288,5000 288,5000 288,5000 288,5000 288,5000 288,5000 28 31,817,263 BALANCE OF FUNDS AVAIL-ABLE FOR PRO-GRAMMED PROJ-ECTS 29.5 26.0 26.0 26.0 8.878.8 23.9 2.5 58.3 6.4 13.6 4.6 5.H Miles APPROVED FOR CONSTRUCTION 93,916 79,530 88,495 88,495 88,495 88,495 88,235 88,235 157,150 150,150 150,150 150,150 150,150 150,150 150,150 150,1 114,200 262,220 262,220 291,148 194,880 235,715 37,035 66,200 68,540 220,041 32,751 20,500 77,767 22,300 90°,995 206,822 70,081 62,425 43,395 5.533.569 Federal Aid 143,050 163,280 56,990 287,990 170,780 35,913 35,913 35,913 460,097 392, 736 251, 231 361, 231 2705, 200 148, 000 148, 000 148, 000 148, 000 126, 226 226, 0524 26, 0524 26, 0524 182.553 235, 280 42, 770 602, 040 602, 040 324, 711 74, 070 74, 070 137,080 476,245 54,585 43,300 171,950 112,098 124.850 187,308 268,172 11,290,469 Estimated Total Cost 2. 20 20. 42 1.837.5 Viiles 310, 542 1, 170, 258 180, 073 45, 153 421, 640 375, 696 375, 696 378, 060 220, 069 19, 348 79, 348 596, 042 23, 267 40, 420 23, 265 40, 420 23, 265 40, 420 23, 265 102, 695 102, 695 395, 375 60, 961 1281, 576 94, 487 94, 487 1336, 467 1336, 467 1338, 586 147, 588 147, 34,065 UNDER CONSTRUCTION 12,932,222 Federal Aid 834,850 110,104 335,124 46,934 46,934 516,233 519,225 519,225 191,253 191,255 766, 884 441, 554 90, 306 90, 306 868, 214 715, 043 153, 296 667, 079 356, 182 68,130 26,089,099 Estimated Total Cost 96.0 686.0 686.0 680.0 171.2 7.7 7.7 7.5 7.5 7.5 57.8 57.8 11.3 5.3 5.3 5.3 5.3 5.3 5.3 5.3 75.8 75.8 14.8 15.2 15.2 15.2 61.4 23.1 23.1 23.1 23.1 37.0 53.0 2,328.8 18.4 14.8 11.6 11.6 23.3 Miles COMPLETED DURING CURRENT FISCAL YEAR 295,507 6,563 6,563 6,563 6,563 6,563 7,105 7,10 81.718 245.084 67.635 180.745 203,281 201,478 110,876 11.829.430 Federal Aid 522 032 123 040 123 040 123 040 123 040 147 145 147 145 147 145 147 145 147 145 147 145 147 145 147 145 147 145 147 145 148 14 224,900 13,126 591,456 599,455 599,455 599,455 591,456 20,122 22,730 22,130 20,130 20,130 20,100 20,100 20,100 20,100 20,100 20,1000 20,1000 2 163,438 799,835 144,531 368,580 109,561 278,383 420,585 224,621 23,886,623 Estimated Fotal Cost **FOTALS** District of Columbia Hawaii Puerto Rico Nebraska Nevada New Hampshire North Carolina North Dakota Ohio Massachusetts Michigan Minnesota Rhode Island South Carolina South Dakota West Virginia Wisconsin Wyoming New Jersey New Mexico New York Oklahoma Oregon Pennsylvania Vermont Virginia Washington California Colorado Connecticut Mississippi Missouri Montana Tennessee Texas Utah Louisiana Maine Maryland Delaware Florida Georgia lowa Kansas Kentucky Alabama Arizona Arkansas Idaho Illinois Indiana

		BALANCE OF FUNDS AVAIL ABLE FOR PROBRAMINED PROJECTS	\$ 895,506 527,973 1.298,166	1, 261, 848 903, 714 827, 380	510,525 1,158,058 2,310,750	2, 493, 011 2, 493, 011	1 744 114	1,053,899 296,231	1,690,082 2,128,334 1,862,250	938, 487 1,918, 286 364, 726	593, 445 142, 893 331, 386	1, 665, 995 640, 726 4, 962, 223	1,170,521 867,488 3 971 041	2,370,198 484,121 4.883,502	152,459 969,965 1.143,378	2, 787, 694 2, 787, 694 291, 230	316, 385 918, 196 541, 588	981,352 1,309,682 508,822	134, 198 360, 830 418, 719	64,098,308
		Grade Grade Crousings Protect- sed by Signals or Other- wite	-	19	- u	53	- 10	t	01		33		35	=	36	5	8 E E	-=		380
		UMBER Grade Crossiag Struc- Lures Re- construct- ed		-	-		M	-		CJ		-	-	-	•		cu			18
	CTION	Crade Crossings Eliminated by Separa- tion or Relocation	ຸດ		- 0	- 50	80 (1) (0	9-		CI JI	16	- 01	N Z V	e eu	3	m0	#	tt. U	- or	116
GCTS	OVED FOR CONSTRU	Federal Aid	\$ μ, 800 104.053	80,272 114,138 171,520	79.700	120,500 979,590 169,040	136,706 164,619 386,625	393,570 147,150 18,200	261, 410 292, 820 290, 819	126,800 959,930	618, 287 146, 1408 102, 302	151,050 87,240	367,770 221,220 1440 180	129,997 129,997 255,686	355.054	472,400 994,375 262,300	25,300 406,404 86,637	64, 400 295, 726 17, 010	243,750 29,220	11, 198, 945
IG PROJI	APPR	Estimated Total Cost	\$ 4,800 104.053	80,272 117,900 171,920	79.700	1,036,590 1,036,590	166,547 164,619 1434,574	394,361 147,150 18,200	262,260 292,820 290,819	1,026,220	618,287 56,717 102,775	151.050 87.240 141.300	367,770 221,220 1400,180	129,997 129,997 255,686	355.054	472,400 1,084,101 262,300	25,490 106,404	64,400 316,850 17,010	283, 5444 29, 220 180, 009	11,621,174
SIN		Grade Crossing: Protect- Protect- signals or Other-	-		6	53	31		10		-	CU .	3	04	N	S	CJ	N		249
Soz		UMBER Grade Crossing Struc- tures Re- construct- ed	-			~~~	-	N	mour	-	-	10 2		-	maa	m.#		-	-	59
CR	NC	Crade Crossings Eliminated by Separa- tion or Relocation	15 mm	0 m	ame	4	ra ö r	tt m-	- 50	- a o	1 <u>4</u> - R	- v	-+	-cu , z	- 50	12 CI	10	5=-	m.9	263
) GRADE	UNDER CONSTRUCTION	Federal Aid	\$ 1,227,324 227,701 209,877	1,725,162 4440,348 12,665	45,420 434,894 436,950	249,386 2,362,545 867,216	230,900 955,333	428,478 329,136 72,188	315,372 588,806 779,733	576.014 147,800 870,293	743,908 209,031 87,797	15,276 15,276 2.011,005	1, 252, 540 591, 290 642, 800	288 590 287,701 1.524,040	438,791 402,427 281,970	323,910 2,168,782 47,359	372, 604 372, 604 821, 164	321,681 1,153,188 128,040	201,200 220,980	29,257,497
RAL-AII		Estimated Total Cost	# 1,229,179 229,905 210,157	1,726,257 1440,348 18,930	45,420 434,894 436,950	280,682 2,362,545 894,116	247,534 955,333 449,315	435,221 329,136 72,188	316,093 588,806 780,054	576,014 1447,800 870,293	743,908 209,031 87,856	15,276 15,276 2,020,155	1,287,640 639,692 642,800	322 590 422 059 1.735 939	438,791 456,943 281,970	2, 199, 487 2, 199, 487 47, 359	7,406 461,604 822,574	337,441 1,194,012 207,460	201,200 222,399	30,137,833
SDE AS		Grada Crossings Protect- Protect- Signals or Other-	5	CJ	14	=	25		35		502	cu	-	Ħ	00	t, D		m 7		141
E	YEAR	UMBER Grade Crossing Strac- tures Re- constract- ed		m		4	N			-	m-	M		N	-	3	aam	Q	-	04
IO S	FISCAL	Grade Grade Crossingn Eliminated by Separa- tion or Relocation	9 10	50		よこれ	22-02-	N	80	t tM	9	- 1- J	N	- 01	- 01	6 8	0 <u>1</u> 2 0	- 10	N	138
STATUS	DURING CURRENT	Federal Aid	# 250,911	1,361,783	35,305	172.543 400.280 584.751	1,001,309 535,054	11,980 53,877	54, 710 924, 372 38, 332	253, 500 295, 421 350, 704	156,459 158,241 69,765	111,665 275,206 991,800	154.540	30,792 307,742 197,923	55,406 128,517	7,360 1481,127 101,648	229, 239 475, 459 236, 347	217,381 200,987 154,992	30,215 61.550	12,094,927
	COMPLETE	Estimated Total Cost	\$ 251,110 492,462	1,362,358 84,715	35.784	180,246 400,280 690,037	1,038,978 535,159 145,000	11.980 53.997	54, 710 932, 761 38,606	253,500 296,960 355,586	162,252 158,241 70,205	116,891 275,206 992,501	154.540	40, 774 308, 391 213, 129	55,856 129,150	7,350 1482,860 101,648	243,221 476,532 247,816	218,401 202,131 154,992	30,215 61,900	12,338,507
		STATE	Alabama Arizona Arkansas	California Colorado Connecticut	Delaware Florida Georgia	Idabo Illinois Indiana	lowa Kansas Kentucky	Louisiana Maine Maryland	Massachusetts Michigan Minnesota	Mississippi Missouri Montana	Nebraska Nevada New Hampshire	New Jersey New Mexico New York	North Carolina North Dakota Ohio	Oklahoma Oregon Pennsylvania	Rhode Island South Carolina South Dakota	Tennessee Texas Utah	Vermont Virginia Washington	West Virginia Wisconsin Wyoming	District of Columbia Hawaii Puerto Rico	TOTALS



