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VIBRATORY MACHINE FOR DETERMINING THE COMPACTIBILITY OF AGGREGATES

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The reports of research published in this magazine are necessarily qualified by the conditions of the tests from which the data are obtained. Whenever it is deemed possible to do so, generalizations are drawn from the results of the tests; and, unless this is done, the conclusions formulated must be considered as specifically pertinent only to described conditions.

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# PRELIMINARY RESULTS OF ROAD-USE STUDIES 

BY DIVISION OF CONTROL, BUREAU OF PUBLIC ROADS

Reported by ROBERT H. PADDOCK and ROE P. RODGERS, Associate Highway Engineer Economists

MOTOR VEHICLES annually travel approximately 250 billion vehicle-miles over the streets and highways of the United States. The benefits derived from such travel may be considered one measure of the returns received on the large capital investment in highway facilities. To finance the facilities necessary for the effective handling of such a great volume of travel, a large portion of the needed revenues is collected from vehicles owners largely on the basis of motor-vehicle use. However, highways also furnish other benefits than those received directly by motorists, and highwayuser revenues are supplemented to a limited extent by revenues from other sources.
In planning highway programs an important problem is determination of where highway-user revenues should be spent to benefit the greatest number of motorists and to provide for the most essential needs on all classes of roads and streets. It is evident that a properly considered highway program should be intended, insofar as possible, to provide facilities for various classes of motorists so that the maximum benefits to the public will be realized.

Determinations of the type and extent of highway use as obtained from road-use studies will assist in formulating such a program. These studies, which are integral parts of the current State-wide highway planning surveys under way in 46 States, will provide factual bases for answering important questions concerning the nature of highway traffic. They will make possible an understanding of the relationships between highway expenditures and the travel of those who pay a large share of the transportation bill. The studies will also show the variations between the motoring habits of rural and urban residents and between the traffic of different types of motor vehicles.

The data presented in this summary are presented without the complementary material which will be available from other phases of the planning surveys, and which are essential in formulating integrated highwaydevelopment programs. However, a study of road-use data will assist in an understanding of highway-transportation problems.

## ANALYSIS MADE OF INTERVIEWS FROM 17 REPRESENTATIVE states

Road-use information was obtained by means of a large number of personal interviews with motor-vehicle owners and drivers. These interviews were carefully selected to insure a proper representation of each geographical division of a State, of each group of governmental jurisdictions within similar population ranges, of various occupations, and of vehicles according to types and ages in operation. Information obtained from vehicle owners by survey interviewers made it possible to determine the extent of the owners' travel during the preceding year, and the routes of such travel for each trip. Experience has demonstrated that the year's driving of an individual can be accounted for
reliably because of the numerous habitual trips, frequent local recreational trips, and unusual long trips that can be easily recalled.

By summarizing the data and expanding to the total State registration for each vehicle type-taking into account all known factors affecting the amount and kind of driving-information is obtained from which it is possible to estimate -

1. The total amount of travel on the various highway systems in a given area or in the State, and
2. The amount of travel performed on the various highway systems in the State by vehicle owners residing in the several governmental jurisdictions.

The two special analyses presented in this report are largely based upon preliminary road-use data obtained in the 17 States of Colorado, Florida, Iowa, Louisiana, Michigan, Minnesota, Missouri, Montana, New York, Ohio, Oklahoma, Oregon, Pennsylvania, South Dakota, Utah, Washington, and Wisconsin. Interviews covering a total of 198,809 passenger cars and 71,941 trucks were taken in these States during different periods, most of them during 1936, but some at an earlier date. All figures were adjusted to the year 1936 in proportion to the motor-vehicle registrations for the particular years under consideration. The 17-State sample was then expanded to obtain figures representing the entire United States by applying road-use data for a particular State to those surrounding or similar States for which data were not yet available.
Factors taken into consideration in these calculations included motor-vehicle registrations, the distribution of population by population groups (see table 1), motor-vehicle ownership per capita for various population groups, and existing mileages of the several highway systems in each State. A number of additional corrective factors were omitted in this preliminary analysis, but it is believed that the results are reliable.
The 17 States which formed the basis for this analysis represent:
45.4 percent of the estimated population of the United States in 1936.
47.8 percent of motor-vehicle registrations in the United States in 1936.
41.0 percent of the State primary road mileage in the United States in 1936.
Results of road-use studies indicate that these States were responsible for:
46.3 percent of estimated travel on all roads and streets in 1936.
44.9 percent of estimated travel on State-administered highways in 1936.
The close agreement of these figures indicates that for the purpose of this study, the 17 States were representative of the country as a whole.

That the estimate of total annual travel amounting to almost 250 billion vehicle-miles for all passenger cars, trucks, and busses in the United States is reasonable, can be demonstrated by comparison with the total

Table 1.- Approximate distribution of population and of motorvehicle registration in the United States in 1936 by population groups of residence

| Population group | Population ${ }^{1}$ |  | Motor-vehicle registration ${ }^{2}$ |  |
| :---: | :---: | :---: | :---: | :---: |
| Unincorporated areas. | Number <br> 44, 636,770 | Percent 36.4 | Number <br> 8,617, 876 | Percent 30.6 |
| Incoroorated places having a population of- |  |  |  |  |
| 1,000 or less | 4, 362, 746 | 3. 6 | 1, 491, 044 | 5.3 |
| 1,001 to 2,500 | 4, 820, 707 | 3. 9 | 1,544,370 | 5.5 |
| 2,501 to 10,000 | 10, 614, 746 | 8.6 | 3, 061, 979 | 10.9 |
| 10,001 to 25,000 | 9, 097, 200 | 7.4 | 2, 444, 929 | 8.7 |
| 25,001 to 100,000 | 12,917, 141 | 10.5 | 3, 419, 713 | 12.1 |
| 100,001 or more- | 36, 325, 736 | 29.6 | 7,585,639 | 26.9 |
| Total | 122, 775, 046 | 100.0 | 28, 165, 550 | 100.0 |

${ }^{1}$ Population data from 1930 census. Total midyear population for 1936 estimated by United States Census Bureau at 128,429,000.
${ }^{2}$ Includes passenger cars, trucks, and busses.
quantity of gasoline consumed in street and highway travel. The total estimated travel of $249,778,990,000$ vehicle-miles in 1936, divided by the $17,855,454,000$ gallons of gasoline used on highways in 1936, gives an average of 14 miles per gallon for all types of motor vehicles. This result is in close agreement with other estimates of average gasoline consumption per vehicle made in recent years. Both this figure and the estimated average annual travel of 8,870 miles for all types of motor vehicles also compare favorably with similar values determined from other highway planning survey data in a number of States

## OVER HALF OF ALL TRAVEL PERFORMED ON PRIMARY STATE HIGHWAYS

The highway systems over which total travel was distributed are classified as (1) primary rural highways and transcity connections, (2) secondary highways and local rural roads, and (3) city streets. Primary rural hiohways under State control consisted of 339,000 miles which, with the urban extensions and connecting links through municipalities of 20,000 miles, totaled 359,000 miles in the United States in 1936.

The secondary and local rural road classification includes 178,000 miles of rural highways under State control other than primary State highways, as well as an estimated $2,440,000$ miles of county and township roads or a total for this classification of $2,618,000$ miles for the United States.

City street mileage comprised 215,000 miles, of which 20,000 miles was urban extensions and connecting links of the primary systems and 195,000 miles was the estimated total of other streets in all incorporated places in 1936.

In determining the distribution of travel to these various systems all travel on streets of incorporated places incurred in going to or coming from rural portions of the primary highway system was summarized separately and for this particular presentation has been credited to the primary system. Similarly, travel on city streets incurred in going to or coming from rural portions of the secondary system was credited to the secondary system. Purely local city travel originating inside a municipality and not extending beyond the city limits was credited to the local street classification, even though some of that travel occurred on the urban extensions or connecting links of the primary system within the city.

Table 2 shows the distribution of estimated annual motor-vehicle travel in the United States in 1936 on the various highway systems, as performed by motorvehicle owners resident in different population groups.

Table 3 shows for each population group of residence or vehicle ownership the percentage of total annual travel performed on each of the highway systems. The composition of the total annual motor-vehicle travel occurring on each highway system according to the various population groups in which the travel originated appears in table 4.
Average annual travel figures for each highway system by motor-vehicle owners resident in each population group appear in table 5.

Table 2.-Estimated motor-vehicle travel on various highway systems in the United States in $1936^{\text {t }}$

| Travel by motor-vehicle owners resident in- | Total travel on- |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Primary rural highways and transcity connections | Secondary highways and local rural roads | City streets | All systems |
| Unincorporated areas Incorporated places having a population of1,000 or less | Million ve-hicle-miles 40, 846.6 | Million ve-hicle-miles $19,453.7$ | Million ve-hicle-miles 3,333. 0 | Million ve-hicle-miles $63,633.3$ |
|  |  |  |  |  |
|  | 9,869.0 | 2,942. 7 | 760.4 | 13,572. 1 |
| 1,001 to 2,500 | 10,368. 9 | 2,063. 6 | 3, 826.8 | 14, 259. 3 |
| 2,501 to 10,000 | $19,800.8$ | 2,909. 3 | 6, 284.5 | 28, 994. 6 |
| 10,001 to 25,000 | $15,127.6$ | 1,906. 8 | 6, 869.8 | 23, 904. 2 |
| 25,001 to 100,000 | 18, 632.8 | 2,044. 5 | 12, 710.2 | 33, 387.5 |
| 100,001 or more | 26, 328. 0 | 2,113.4 | 43, 586. 5 | 72, 027.9 |
| Total | 140,973. 7 | $33,434.0$ | 75,371. 2 | 249, 778.9 |

Based on preliminary data from road-use surveys in 17 representative States.

Table 3.-Percentage of estimated motor-vehicle travel on the various highway systems in the United States in 1936

| Travel by motor-vehicle owners resident in- | Total travel on- |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Primary rural highways and transcity connections | Secondary highways and local rural roads | City <br> streets | All systems |
| Unincorporated areas | Percent 64.2 | Percent 30.6 | Percent 5.2 | Percent 100. |
| Incorporated places having a population of- |  |  |  |  |
| 1,000 or less | 72.7 | 21.7 | 5. 6 | 100.0 |
| 1,001 to 2,500 | 72.7 | 14.5 | 12.8 | 100.0 |
| 2,501 to 10,000 | 68.3 | 10.0 | 21.7 | 100.0 |
| 10,001 to 25,000 | 63.3 | 8.0 | 28.7 | 100.0 |
| 25,001 to 100,000 | 55.8 | 6. 2 | 38.0 | 100.0 |
| 100,001 or more. | 36.6 | 2.9 | 60.5 | 100.0 |
| Total | 56.4 | 13.4 | 30.2 | 100.0 |

Table 4.-Percentage of estimated motor-vehicle travel on each highway system by population groups of residence in which travel originated in the United States in 1936

| Travel by motor-vehicle owners resident in- | Total travel on- |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Primary rural highways and transcity connections | Secondary highways and local rural roads | City streets | $\begin{gathered} \text { All } \\ \text { systems } \end{gathered}$ |
| Unincorporated areas | Percent 29.0 | Percent 58.2 | Percent 4.4 | Percent 25.5 |
| Incorporated places having a population of - |  |  |  |  |
| 1,000 or less .. | 7.0 | 8.8 | 1. 0 | 5.4 |
| 1,001 to 2,500. | 7.4 | 6.2 | 2.4 | 5.7 |
| 2,501 to 10,000 | 14.0 | 8.7 | 8.3 | 11.6 |
| 10,001 to 25,000 | 10.7 | 5.7 | 9.1 | 9.6 |
| 25,001 to 100,000 | 13.2 | 6. 1 | 16.9 | 13. 4 |
| 100,001 or more. | 18.7 | 6.3 | 57.9 | 28.8 |
| Total. | 100.0 | 100.0 | 100.0 | 100.0 |

The data presented in tables 2 and 3 indicate that of the nearly 250 billion vehicle-miles traveled in 1936 by passenger cars, trucks, and busses in the United States, 56.4 percent was travel on the primary rural highways and transcity connections, 13.4 percent on the secondary highways and local rural roads, and 30.2 percent on city streets. These figures may be more easily visualized by reference to table 5 , which shows that the average motor vehicle traveled 8,870 miles during 1936, and that the division of this travel among the three classes of highways was $5,000,1,190$, and 2,680 miles, respectively.

TABLE 5.-Estimated average travel per motor vehicle on the various highway systems of the United States in 1936

| Travel by motor-vehicle owners resident in- | Average travel on- |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Primary rural highways and transcity connections | Secondary highways and local rural roads | City streets | All systems |
| Unincorporated areas. | Vehiclemiles 4, 740 | Vehiclemiles 2, 250 | Vehiclemiles 390 | Vehiclemiles 7, 380 |
| Incorporated places having a population of - |  |  |  |  |
| 1,000 or less | 6, 620 | 1,970 | 510 | 9,100 |
| 1,001 to 2,500 | 6,710 | 1,340 | 1,180 | 9, 230 |
| 2,501 to 10,000 | 6,470 | 950 | 2,050 | 9,470 |
| 10,001 to 25,000 | 6, 190 | 780 | 2,810 | 9,780 |
| 25,001 to 100,000 | 5, 450 | 600 | 3, 710 | 9,760 |
| 100,001 or more. | 3,470 | 280 | 5, 740 | 9,490 |
| Total | 5, 000 | 1,190 | 2,680 | 8,870 |

Because the total average annual travel for motor vehicles registered in each population group was relatively uniform with the exception of those owned in unincorporated areas (table 5), the percentage of total annual travel on all highways and streets corresponded very closely to the percentage distribution of vehicle registrations within each population group. This fact is apparent from comparison of the figures in the last columns of tables 1 and 4.

## MAJOR USE OF PRIMARY HIGHWAYS WAS BY CITY CAR OWNERS

There was considerable difference, however, in the relative use of the various highway systems by vehicles registered in the several population groups. These differences are indicated in tables 3 and 5. Vehicles owned in unincorporated areas performed 64.2 percent and 30.6 percent of their travel in 1936 on the primary highways and the secondary and local rural roads, respectively, and used city streets for only 5.2 percent of their total travel.

The use of the various highway systems by vehicles owned in the smaller incorporated places was somewhat similar to that for rural-owned vehicles. However, it is interesting to note the extent of the change in use of other highway systems with increase in the size of the place of vehicle ownership. Vehicles owned in the group of smallest incorporated places used the primary highways and the secondary and local rural roads for 72.7 percent and 21.7 percent, respectively, of their total annual driving, while vehicles owned in cities having populations over 100,000 used these same systems to the extent of 36.6 percent and 2.9 percent, respectively. Vehicles owned in the smallest incorporated places were used on city streets for only 5.6 percent of their total-annual travel, but those owned in the largest cities performed 60.5 percent of their annual travel on streets of incorporated places. (See table 3.)


Figure 1.-Distribution of Estimated Annual MotorVehicle Travel in the United States in 1936 by Residents of Unincorporated Areas and Incorporated Places.

This increase in the use of city streets by vehicles owned in the larger incorporated places was, of course, accompanied by a corresponding decrease in the use of other highway systems. It should be noted, however (table 3), that even for vehicles owned in the largest cities the primary rural highways and transcity connections were used for over one-third of the total annual travel. This use was sufficiently high to account for 18.7 percent (table 4) of the annual travel by all motor vehicles on the primary highway system.

As shown in table 4 and figure 1, the largest proportion of travel on the primary highway system was that of town and city residents. Motorists living in incorporated places accounted for 71 percent of the total travel on this system.

The importance of the primary highways to the city motorist is apparent. Though his use of the rural highway system decreased (see table 3) as the size of the place in which the motorist lived increased, the percentage of his travel on such highways was considerable. Only for vehicle owners resident in cities over 100,000 population did their travel on primary rural highways fall below 50 percent of their total travel.
Considering only residents of cities having populations of more than 10,000 , table 2 shows that they accounted for more than 60 billion vehicle-miles of the $140,973,700,000$ vehicle-miles traveled on primary highways in 1936. Residents of unincorporated areas accounted for only slightly more than 40 billion vehiclemiles of the primary highway travel, or less than that provided by vehicle owners from cities having more than 25,000 population.

In considering travel by residents of incorporated places having 10,000 population or less, it is significant that the percentage of their travel on primary highways as shown in table 3 was greater than that of any other
group, even the residents of unincorporated areas. Practically all such incorporated places are located on or within a very short distance from primary roads. Highway development in this country has been such that generally it has been expected that all but the very smallest places would be given consideration in the location of primary routes. Vehicle owners living within these cities are located close to primary highways; they are much closer than many rural residents who live on township or county roads; and they are generally closer than residents of the larger cities who frequently must travel a considerable distance to reach primary routes.

It is apparent from this discussion and from the data shown in the accompanying tables that the major use of the primary highways is by city motor-vehicle owners, and that in spite of their large use of local city streets, the use of primary highways by all city-owned vehicles is greater than their use of all other highway facilities. It therefore follows that the provision of adequate rural highway facilities today is of major importance to the city motorist and that the required improvements in those facilities are largely occasioned by the city motorists' demands on the primary system.

## TRIP-LENGTH DATA OBTAINED IN 11 STATES

Table 4 shows that only 4.4 percent of the total travel on city streets was contributed by motorists living in unincorporated areas, and that most of the remaining 95.6 percent of travel performed by residents of incorporated places represented the operations of residents in the larger cities. Of all travel on city streets, 57.9 percent was performed by residents of cities having populations of over 100,000, and drivers living in cities with over 10,000 inhabitants accounted for 83.9 percent of the total travel on local city streets.

Concerning motor-vehicle use on all streets and highways, approximately one-fourth was by residents of unincorporated areas, while twice that amount, or 51.8 percent of all travel, represents the driving of those living in cities having over 10,000 inhabitants. The largest percentage of vehicle travel accounted for by residents of any one group of governmental units as shown in table 4 was that originating in cities having populations over 100,000 . Residents of these cities contributed 28.8 percent of all travel on all roads and streets.
These data on vehicle travel have also been expressed in terms of average 24 -hour traffic volumes for each class of road and street. Table 6 shows that for the

Table 6.-Approximate mileage of each highway system and average 24-hour traffic volume on each highway system in the United States in 1996

| Highway system | Approximate mileage | Estimated total annual motorvehicle travel | A verage 24-hour traffic volume |
| :---: | :---: | :---: | :---: |
| Primary rural highways and transcity connections. <br> Secondary highways and local rural roads City streets | $\begin{gathered} \text { Miles } \\ 359,000 \\ 12,618,000 \\ 2215,000 \end{gathered}$ | Million vehicle-miles 140, 973.7 33, 434.0 75, 371.2 | $\begin{array}{r} \text { Vehicles } \\ 1,076 \\ 35 \\ 960 \end{array}$ |
| All systems. | ${ }^{3} 3,172,000$ | 249, 778.9 | 216 |

[^0]country as a whole, primary rural highways and their transcity connections carried an average daily volume of 1,076 vehicles, which was slightly higher than the 960 vehicles computed as the average for city streets. These volumes were about 30 times greater than the average daily volume on secondary and local rural roads combined. Average 24 -hour traffic volume for the more than 3 million miles of roads and streets in the United States was estimated at 216 vehicles.

Another special study of considerable value was also made from road-use data concerning the radii of operation of motor vehicles. It was sought by this investigation to determine the length of vehicle trips that extend beyond the limits of cities; that is, of trips that are either partly or wholly on rural roads. Thus all trips by residents of unincorporated areas were included; but for motorists living in incorporated places, only those trips were counted that extended beyond the limits of the town or city in which the driver resided.

This special study was made in the 11 States of Florida, Kansas, Louisiana, Minnesota, New Hampshire, Pennsylvania, South Dakota, Utah, Vermont, Washington, and Wisconsin. In 1936 there were 4,862,541 passenger cars and 880,432 trucks registered in these 11 States, or a combined registration of 5,742 ,973. These figures are presented in table 7, together with information concerning the number of interviews taken in each State. The number of interviews totaled 129,407, and consisted of 94,167 for passenger cars and 35,240 for trucks. Trip-length information was not expanded to represent data for the entire country, but only to represent total registrations in each of these States.

For purposes of this analysis, all trips have been classified as one-way trips. If a motor-vehicle owner left his home and drove to some other point 10 miles distant, requiring a total travel of 20 miles from point of starting until return to that point, such a trip could be classified as two one-way trips of 10 miles each. The one-way trip classification has been used for all tabulations in this discussion.

TABLE 7.-1936 motor-vehicle registrations and number of road use interviews used for basis of analysis of total number of oneway trips outside city limits in 11 States

| State | 1936 registration |  |  | Number of interviews |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Passenger cars | Trucks | Total | Passenger cars | Trucks | Total |
| Florida | 321, 467 | 63, 885 | 385, 352 | 7,015 | 3,010 | 10,025 |
| Kansas_ | 490, 793 | 187, 113 | 577,906 | 8, 663 | 2,813 | 11, 476 |
| Louisiana | 228, 361 | 73,628 | 301, 989 | 3,891 | 1,623 | 5, 514 |
| Minnesota. | 668,915 | 114, 448 | 783, 363 | 13,059 | 5,649 | 18, 708 |
| New Hampshi | 97, 361 | 124,875 | 122, 236 | 1,936 | 914 | 2, 850 |
| Pennsylrania | 1,615,955 | 235, 834 | 1,851,789 | 23,783 | 10,567 | 34, 350 |
| South Dakota | 158, 192 | 28, 216 | 186,408 | 3, 608 | 1,533 | 5,141 |
| Utah. | 96, 768 | 19, 397 | 116, 165 | 2, 148 | 1,097 | 3,245 |
| Vermont | 75, 195 | 8,845 | 84, 040 | 1,472 | 850 | 2, 322 |
| Washingtor | 419, 493 | 79,538 | 499, 031 | 14, 027 | 1,313 | 15, 340 |
| W isconsin | 690, 041 | 144, 653 | 834, 694 | 14, 565 | 5,871 | 20, 436 |
| Total | 4, 862, 541 | 880, 432 | 5, 742, 973 | 94, 167 | 35, 240 | 129,407 |

${ }^{1}$ Includes busses.
PASSENGER-CAR AND TRUCK TRIPS PREDOMINATELY OF SHORT LENGTH
Tables 8 and 9 contain analyses of the length of oneway trips partially or wholly traveled on roads in unincorporated areas. The numbers of these trips within designated length classifications are shown graphically in figure 2 for passenger cars and trucks combined.

The short length of travel of a large part of motor-


Figure 2.-Percentage of all Passenger-Car and Truck Trips Exceeding Various Lengths.
vehicle operation is readily apparent. For passenger cars, trips of less than 5 miles constituted 38.4 percent of the number of all one-way trips traveled partly or wholly on highways in unincorporated areas. Trips of over 5 but less than 10 miles comprised 26.5 percent of the total. Of all the one-way trips tabulated, therefore, 64.9 percent of the total number were less than 10 miles long. Trips less than 20 miles long accounted for 85 percent of all passenger-car trips. Accordingly, only 15 percent of all trips extending beyond city
limits or traveled entirely on rural roads were greater than 20 miles long. Passenger cars went over 100 miles from their starting point on only 1.5 percent of all their trips.

Analysis of truck movements gave fairly similar results, 34 percent of all trips being less than 5 miles long, 59.5 percent less than 10 miles, and 80.3 percent less than 20 miles. Trips over 100 miles were 2.0 percent of the total number of all trips, and truck trips above 50 miles and less than 250 miles long constituted 6.2 percent of the total number as compared with 4 percent for passenger cars.

Considering passenger cars and trucks combined, 37.5 percent of the number of all one-way trips involving travel on roads in unincorporated areas extended less than 5 miles from the point of origin. The fact that the many short trips made wholly within incorporated areas have been omitted from these trip-length data emphasizes still further the preponderant use of motor vehicles for short trips.

Tables 10 and 11 show the States of destination of one-way trips over 100 miles long made by passenger cars and by trucks registered in the 11 States. These data are summarized in table 12 to show the percentage of such trips having destinations in the State of origin,

Table 8.-Frequency distribution of the length of all one-way trips made by passenger cars that extended outside city limits in 11 States ${ }^{1}$ TOTAL NUMBER OF TRIPS

| State | Length of one-way trips from point of origin in miles |  |  |  |  |  |  |  |  |  |  | Total all trips |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\operatorname{Less}_{5}^{\text {than }}$ | 5 to 9.9 | 10 to 19.9 | 20 to 29.9 | 30 to 39.9 | 40 to 49.9 | 50 to 99.9 | $\begin{aligned} & 100 \text { to } \\ & 249.9 \end{aligned}$ | $\begin{gathered} 250 \text { to } \\ 499.9 \end{gathered}$ | $\begin{gathered} 500 \text { to } \\ 999.9 \end{gathered}$ | $\begin{gathered} 1,000 \text { and } \\ \text { over } \end{gathered}$ |  |
|  | 1,000 | 1,000 | 1,000 | 1,000 | 1,000 | 1,000 | 1,000 | 1,000 | 1,000 | 1,000 | 1,000 | 1,000 |
| Florida | $\begin{aligned} & \text { trips } \\ & 45,189 \end{aligned}$ | trips | trips | trips | trips | trips | trips | trips | trips | trips | trips | ${ }_{\text {trips }}$ |
| Kansas. | 124, 109 | 69,011 | 54,600 | 17,267 | - ${ }_{6}^{4,526}$ | 2,282 | - 6,634 | 1,4,91 | $\begin{aligned} & 400 \\ & 458 \end{aligned}$ | $\begin{array}{r} 80 \\ 154 \end{array}$ | $\begin{aligned} & 76 \\ & 89 \end{aligned}$ | $13.9,907$ 283,721 |
| Louisiana | 43, 984 | 25, 005 | 18, 019 | 6,970 | 2,986 | 1,335 | 3, 842 | 946 | 182 | 35 | 10 | 283, 103,314 |
| Minnesota | 97, 533 | 62,426 | 51, 591 | 16,477 | 6,477 | 3,680 | 8,224 | 5,045 | 556 | 89 | 45 | 252, 143 |
| New Hampshire | 10,782 | 12,941 | 10,975 | 3, 046 | 1,393 | ${ }^{9} 900$ | 1,536 | 326 | 27 | 8 | 4 | 41, 938 |
| Pennsylvania | 296, 153 | 214,362 | 154, 277 | 47,626 | 21, 246 | 9,180 | 19,254 | 8,016 | 763 | 221 | 91 | 771,189 |
| South Dakota | 16, 704 | 11,760 | 11,880 | 4,894 | 1,631 | 1,233 | 1,950 | 734 | 180 | 41 | 24 | 51, 031 |
| Utah | 17,019 | 9,198 | 6,838 | 2, 626 | 1,653 | 908 | 1,053 | 477 | 134 | 60 | 41 | 40,007 |
| Vermont | 14, 763 | 10,650 | 6,178 | 2,079 | 718 | 562 | 757 | 223 | 28 | 1 | 1 | 35, 960 |
| Washington | 73, 201 | 42,913 | 33, 562 | 11,020 | 5, 700 | 2, 208 | 4,882 | 2,062 | 359 | 49 | 59 | 176, 015 |
| Wisconsin | 95, 622 | 77, 445 | 58,499 | 20,691 | 9,538 | 4,123 | 9,324 | 3,826 | 541 | 82 | 49 | 279, 740 |
| Total | 835, 059 | 576, 295 | 438, 222 | 143, 765 | 61, 923 | 27, 784 | 61, 232 | 25,743 | 3,628 | 825 | 489 | 2,174,965 |

percentage of total number of trips

| Florida | Percent 32.3 | Percent 29.0 | Percent 22.7 | Percent | Percent 2.9 | Percent | Percent 2.7 | Percent | Percent | Percent | Percent | Percent |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Kansas | 43.8 | 24.3 | 19.2 | 6.1 | 2. 3 | . 8 | 2.3 | - 1.9 |  |  | (2) | 100 |
| Louisiana | 42.6 | 24.2 | 17.4 | 6.8 | 29 | 1. 3 | 3.7 | . 9 | . 2 | (2) | (2) | 100 |
| Minnesota | 38.7 | 24.8 | 20.4 | 6.5 | 2.6 | 1.5 | 3.2 | 2.0 | . 2 |  | ${ }^{(2)}$ | 100 |
| New Hampshir | 25.7 | 30.9 | 26.1 | 7.3 | 3. 3 | 2.2 | 3. 6 | . 8 | . 1 | (2) | (2) | 100 |
| Pennsylvania. | 38.4 | 27.8 | 20.0 | 6.2 | 2.7 | 1.2 | 2.5 | 1. 0 | . 1 |  | (2) | 100 |
| South Dakota | 32.7 | 23.0 | 23.3 | 9.6 | 3.2 | 2.4 | 3.8 | 1.4 | . 4 | . 1 | . 1 | 100 |
| Utah | 42.6 | 23.0 | 17.1 | 6.5 | 4.1 | 2.3 | 2.6 | 1.2 | . 3 |  | . 1 | 100 |
| Vermont | 41.0 | 29.6 | 17.2 | 5.8 | 2. 0 | 1. 6 | 2.1 | . 6 | . 1 | ${ }^{(2)}$ |  | 100 |
| Washington_ | 41.6 34.2 | 24.4 27.7 | $\begin{aligned} & 19.0 \\ & 20.9 \end{aligned}$ | 6.3 7.4 | 3.2 <br> 3.4 | 1.3 1.5 | 2.8 <br> 3.3 | 1.1 1.4 | . 2 | (2) .1 | ${ }^{(2)}$ | 100 100 |
| Total | 38.4 | 26.5 | 20.1 | 6.6 | 2.8 | 1.3 | 2.8 | 1.2 | 2 | . 1 | ${ }^{(2)}$ | 100 |

CUMULATIVE PERCENTAGE OF TOTAL NUMBER OF TRIPS


[^1]TABIE 9.-Frequency distribution of the length of all one-way trips made by trucks that extended outside city limits in 11 States ${ }^{1}$ TOTAL NUMBER OF TRIPS

| State | Length of one-way trips from point of origin in miles |  |  |  |  |  |  |  |  |  |  | Total all trips |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Less than 5 | 5 to 9.9 | 10 to 19.9 | 20 to 29.9 | 30 to 39.9 | 40 to 49.9 | 50 to 99.9 | $\begin{aligned} & 100 \text { to } \\ & 249.9 \end{aligned}$ | $\begin{gathered} 250 \text { to } \\ 499.9 \end{gathered}$ | $\begin{gathered} 500 \text { to } \\ 999.9 \end{gathered}$ | $\begin{aligned} & 1,000 \text { and } \\ & \text { over } \end{aligned}$ |  |
|  | 1,000 | 1,000 | 1,000 | 1,000 | 1,000 | 1,000 | 1,000 | 1,090 | 1,000 | 1,000 | 1,000 | 1,000 |
|  | trips | trips | trips | trips | trips | trips | trips | trips | trips | trips | trips | trips |
| Florida | 11, 145 | 11, 433 | 8,504 | 3,616 | 1,456 |  | 1,358 | 842 | 179 |  |  | $\begin{aligned} & 39,080 \\ & 49,679 \end{aligned}$ |
| Kansas... | 17,622 <br> 14,769 | 12,722 | 10,389 12,645 | 3,777 <br> 4,803 | 1,540 2,539 | 741 962 | 1,946 3,909 | 792 1.325 | $\begin{array}{r} 111 \\ 97 \end{array}$ | $\begin{array}{r} 23 \\ 8 \end{array}$ | $\text { (2) }^{16}$ | 49.679 52,844 |
| Minnesota. | 20,591 | 20,750 | 14,942 | 5, 534 | 2,169 | 1,302 | 3,767 | 2, 511 | 206 | 11 | 3 | -51,786 |
| New Hampshir | 2,708 | 3,504 | 2,584 | 863 | 414 | 195 | 602 | , 75 | 3 | ${ }^{(2)}$ |  | 10, 948 |
| Pennsylvania. | 53,717 | 33,446 | 24, 704 | 7.291 | 3,789 | 1.897 | 4, 077 | 1,130 | 59 | 20 | (2) | 130, 130 |
| South Dakota | 2, 824 | 2. 528 | 3, 104 | 1,239 | 516 | 542 | 1,181 | 505 | 77 | 6 | ${ }_{2}^{2}$ | 12, 524 |
| Utah.-. | 4, 579 | 1,960 | 1,616 | 659 | 395 | 247 | 462 | 314 | 45 | 5 | $2$ | 10, 284 |
| Vermont | 3,159 | 2,817 | 1,969 | 564 | 247 | 197 | 258 | 51 | 33 | 2 | ${ }^{(2)}$ | 9, 297 |
| W ashington | 17,515 | 9,438 | 9, 724 | 4. 734 | 2,768 | 905 | 1,966 | 755 | 72 | ${ }^{6}$ | 8 | 47, 892 |
| Wisconsin. | 36,323 | 28,531 | 23,340 | 8,775 | 4,197 | 2, 759 | 4.382 | 1,611 | 152 | 10 | (2) | 110, 080 |
| Total. | 184, 952 | 138, 916 | 113, 521 | 41,855 | 20,030 | 10,262 | 23,908 | 9,911 | 1, 034 | 110 | 45 | 544, 544 |

PERCENTAGE OF TOTAL NUMBER OF TRIPS


CUMULATIVE PERCENTAGE OF TOTAL NUMBER OF TRIPS

|  | Percent | Percent | Percent | Percent | Percent | Percent | Percen | Percent | Percent | Percent | Percent | Percent |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Florida | 28.5 | 57.7 | 79.5 | 88.8 | 92.5 | 93.8 | 97.3 | 99. 4 | 99.9 | 100.0 | 100 |  |
| Kansas... | 35.5 | 61.1 | 82.0 | 89.6 | 92.7 | 94.2 | 98.1 | 99.7 | 99.9 | 100.0 | 100 |  |
| Louisiana | 27.9 | 50.2 | 74.1 | 83.2 | 88.0 | 89.9 | 97.3 | 99.8 | 100.0 | 100.0 | 100 |  |
| Minnesota | 28.7 | 57.6 | 78.4 | 86.1 | 89.1 | 90.9 | 96.1 | 99.6 | 99.9 | 100.0 | 100 |  |
| New Hampshir | 24.7 | 56.7 | 80.3 | 88.2 | 92.0 | 93.8 | 99.3 | 100.0 | 100.0 | 100.0 | 100 |  |
| Pennsylvania | 41. 3 | 67.0 | 86.0 | 91.6 | 94.5 | 95.9 | 99.0 | 99.0 | 100.0 | 100.0 | 100 |  |
| South Dakot Utah | 22.5 44.5 | 42.9 63.6 | 67.5 79 | 77.4 | 81.5 | 85.8 | 95.2 | 99.2 | 99.8 | 99.9 | 100 |  |
| Vermont. | 34.0 | 64.3 | 85.5 | 91.5 | 94.2 | 96.3 | 99.1 | 99.6 | 100.0 | 100.0 | 100 |  |
| W ashington | 36.6 | 56.3 | 76.6 | 86.5 | 92.2 | 94.1 | 98.2 | 99.8 | 100.0 | 100.0 | 100 |  |
| Wisconsir | 33.0 | 58.9 | 80.1 | 88.1 | 91.9 | 94.4 | 98.4 | 99.9 | 100.0 | 100.0 | 100 |  |
| Total | 34.0 | 59.5 | 80.3 | 88.0 | 91.7 | 93.6 | 98.0 | 99.8 | 100.0 | 100.0 | 100 |  |

1 Based on analysis of $22,268,882$ one-way trips performed by 35,240 trucks in these States.
2 Less than 500 trips.
${ }_{2}^{2}$ Less than 500 trips.
${ }^{3}$ Less than 0.1 percent.
Table 10.-Estimated total number of annual one-way trips over 100 miles long traveled by passenger cars registered in 11 States of origin, and classified by State of destination of individual trips

| State of destination | State of origin |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Florida | Kansas | Louisiana | Minnesota | New <br> Hampshire | Pennsyl- vania | South <br> Dakota | Utah | Vermont | Washing- ton | Wisconsin |
| Alabama | 38,172 | 418 | 33,422 |  |  | 960 | 1,498 | 90 |  | 80 | 102 |
| Arizona-- |  | 764 | 72 | 546 |  | 108 | 430 | 3,474 |  | 1,272 | 172 |
| California | 1,870 | 38,18122 | 43, 1,556 | 8,994 | 84 | 3,516 | 5,710 | 33,084 |  | 1. 46 | 5,716 |
| Colorado- | 1,032 | 144, 456 | 130 | 1, 434 |  | , 994 | 5,902 | 21, 856 |  | 1,090 | 5,716 1,088 |
| Connecticut | 1,060 |  | 130 |  | 9,044 | 18, 178 |  |  |  |  | $\begin{array}{r}1,088 \\ \hline 596\end{array}$ |
| Florida. | 1,746,624 | 2,048 | 8,518 | 2,156 | 1,514 | 124,146 30,266 | 86 456 |  | 7, 424 | 160 | 5,838 |
| Georgia. | 137, 300 | 688 1.204 | 6,096 | $\stackrel{110}{116}$ |  | 3,986 |  |  | 86 | 220 | 5,838 628 |
| Illinois. | 4,496 | 23, 858 | 2,060 | 59, 168 | 288 | 29,806 | 7,314 | 115,152 |  | 123,418 2,040 | 260 536,088 |
| Indiana. | 4,590 | 3, 582 | 292 | 2,398 |  | 12,360 | 1,442 | 90 |  | 494 | 536,088 34,820 |
| Iowa... | 610 296 | 20,996 $1,827,206$ | 344 | 119, 550 | 126 | 1,906 | 73,950 | 90 |  | 466 | 67,972 |
| Kentucky | 6,094 | $1,827,206$ 2,462 2 |  | 842 | -- | ¢ 950 | 3,090 | 260 |  | 312 | 990 |
| Louisiana | 7,826 | 4,026 | 873, 324 | 612 |  | 6, 904 |  | 150 |  |  | 2,916 |
| Maine -... | 1,140 | 278 |  | 110 | 47,766 | 10,444 |  |  | 9,366 |  | 286 |
| Maryland | 1,500 |  | 324 | 152 | 420 | 381, 698 | 602 |  |  |  | 994 |
| Massachusetts | 2, 570 | 690 |  | 536 | 124,144 | 36, 346 |  | 150 | 79, 258 | 96 | 1,918 |
| Michigan... | 3, 814 | 9,594 14,926 | 584 | 56, 724 | 428 | 46, 672 | 1,444 | 758 |  | 3,376 | 158,378 |
| Mississippi | 6,482 | 14,95 1,504 |  | 5,106, 198 |  | 742 | 101,304 | 76 |  | 1,528 | 322, 160 |
| Missouri | 1,156 | 505, 022 | 890 | 5, 554 | 84 | 2,610 | 2, 502 | 400 | 116 | 684 | 8,436 |
| Nebraska- | 134 | 1,570 |  | 5,232 |  | 360 | 3, 292 | 21,496 |  | 32, 826 | 1,222 |
| Nevada. | 136 | -8,424 |  | 6,592 |  | 916 | 24,628 | 544 31,824 |  | 392 844 | 4, 402 |
| New Hampshire | 538 |  |  |  | 100,766 | 7,056 |  |  | 22,576 |  |  |
| New Jersey | 2,044 |  |  |  | 856 | 1,100, 928 |  |  | 1,560 |  |  |
| New Mexico. |  | 8,696 | 924 | 76 |  |  | 100 | 256 |  | 164 | 206 |

Table 10.-Estimated total number of annual one-way trips over 100 miles long traveled by passengercars registered in 11 Statos of origin, and classified by state of destination of individual trips-Continued

| State of destination | State of origin |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Florida | Kansas | Louisiana | Minnesota | New <br> Hampshire | Pennsylvania | South Dakota | Utah | Vermont | Washingfon | Wisconsin |
| New York. | 12.708 | 4. 4.58 | 1,606 | 2, 296 | 14,820 | 1,368,620 | 2, 526 | 902 | 28, 172 | 1,620 | 9. 522 |
| North Carolina | 23, 894 | 274 | 2. 396 |  | 84 | 14, 120 |  |  | 116 | 160 | 2, 6682 |
| North Dakota. | 188 | $56 \cdot 2$ | 126 | 89, 242 |  | 244 | 20,481 |  |  | 1. 066 | 4. 032 |
| Ohio | 6, 056 | 4. 124 | 396 | 1,786 | 534 | 837, 310 | 186 | 492 | 232 | 204 | 8,896 |
| Oklahoma. | $5 \times 4$ | 284.538 | 1. 812 | 1,214 |  | 1,578 | 25x | 152 |  | 382 | 424 |
| Oregon | 100 | 2. 1442 | 256 | 640 |  | 114 | 4.52 | 4,188 |  | 343, 286 | .586 |
| Pennyslvania | 4. 482 | 1. 176 | 162 | 720 | 560 | 4,419, 820 |  |  | 1,284 | 160 | 2,256 |
| Khode Island | 202 | 70 | 72 |  | 12,698 | 4,126 |  |  | 3, 032 |  |  |
| South Carolina. | 8,336 | 988 | 200 |  |  | 2,620 |  |  | 116 |  | 688 |
| South Dakota. |  | 3. 048 |  | 62,812 |  | -228 | 698, 396 |  |  | 904 | 11,084 |
| Tennessee... | 13.486 | 1,872 | 6., 868 | 1,048 | 224 | 4,620 | 184 | 90 |  |  | 1, 650 |
| Texas.. | 2.894 | 41.381 | 93,052 | 2,308 | 84 | 1,682 | 796 | 570 |  | 774 | 2.268 |
| Vtah |  | 4612 |  | 196 |  |  | 272 | 314, 246 |  | 2,356 | 462 |
| Vermont. | 340 |  |  |  | 34,402 | 5,206 |  |  | 73,884 |  |  |
| Virginia_ | 2. 480 | 380 | 304 |  | 252 | 94, 218 | 86 |  | 13,881 | 4ti |  |
| Washington.. | 220 | 2. 196 |  | 2,158 |  | 154. 142 | 2. 450 | 3,302 |  | 1, 882, 136 | 558 |
| West Virginia | (i18 | 352 | 162 | 2, 286 |  | 159,020 |  |  |  |  | 124 |
| Wisconsin - | 976 | 1, 406 | 388 | 157, 866 |  | 3, 6694 | 5. 8.70 | 90 |  | 416 | 3,277,310 |
| W yoming. | 168 | 4. 136 | 130 | 1,748 | 84 | 490 | 10, 748 | 148, 118 |  | 4, 8.76 | 1,534 |
| District of Columbia | 5. 042 | 1.718 | 528 | 1,096 | 880 | 294, 168 |  | 210 | 402 | 296 | 1. 510 |
| Canadr... | 2, 594 | 2. 944 | 848 | 31,142 | 15,532 | 55, 096 | 2. 336 | 8, 960 | 26,328 | 76, 2\%.4 | 13, 678 |
| Mexico. | 220 | 2. 6110 | 1. 756 | 1,284 |  | 356 | 794 | 428 |  | 3, 418 | 748 |
| Total. | 2. 056.412 | 3.092. 706 | 1,172,848 | 5, 737, 020 | 366,038 | 9,090, 492 | 981.988 | 712,052 | 254,038 | 2,528, 100 | 4. 497.614 |

Table 11.-Estimated total number of annual one-way trips over 100 miles long traveled by trucks registered in 11 States of origin, and classificd by State of destination of individual trips

| state of destivation | State of origin |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Florida | Kansas | Louisiana | Minnesota | New <br> Hampshire | Pennsylvania | South Dakota | U゙tah | Vermont | Washing- ton | Wisconsin |
| Alabama | 38,538 |  | 70 | 23 |  |  |  | 2900 |  |  |  |
| Arkansas. |  | 2. 134 | 34.250 |  |  |  |  |  |  |  |  |
| California |  | - 52 |  |  | 44 | 40 |  | 5.242 |  | 8.296 | 176 |
| Colorado-.... |  | 13. 748 |  |  | 3,576 | 1,318 | 10. 704 | 7,958 | 2,078 |  |  |
| Delaware... |  |  |  |  |  | 14, 238 |  |  |  |  | -- |
| Florida. | 930, 621 |  |  |  |  | 190 |  |  |  |  |  |
| Georgia. | 49,568 |  |  | 44 50 |  | 514 | 68 | 59,912 |  |  |  |
| Illinois.. | 220 | 2. 504 |  | 12. fiog |  | 2,294 | 924 |  |  | 110 | 361,321 |
| Indiana | 121 | 2. 776 |  | $\begin{array}{r}5,827 \\ 97 \\ 97 \\ \hline\end{array}$ |  | 86 | 34 43.992 |  |  |  | 2,866 36,182 |
| Kansas. |  | 581, 866 |  |  |  |  | 460 |  |  | 110 |  |
| Kentucky | 137 | 652 |  |  |  | 4, 224 |  |  |  |  | 176 |
| Mraine... | 126 |  | 1. 266, 240 |  | 11,850 |  |  |  | 72 |  |  |
| Maryland.- | 5, 321 |  |  |  |  | 71. 294 |  |  |  |  | 176 |
| Massachusetts $M$ ichigan | 133 |  |  |  | 38, 142 | 4. 114 |  |  | 12,604 | 204 |  |
| Minnesota |  | 154 |  | 2, 489,410 |  |  | 28,570 |  |  |  | 201, 889 |
| Mississippi | 250 |  | 30, 856 |  |  |  |  |  |  |  |  |
| Missouri. |  | 233, 474 | 136 | 38, 565 |  | 132 | 806 | 548 |  |  | 176 |
| Nehraska |  | 26,368 |  | 2, 556 |  |  |  | 348 |  | 1710 | 176 |
| Nevada. |  |  |  |  |  |  |  | 11,122 |  |  |  |
| New Hampshire |  |  |  |  | 13, 990 |  |  |  | 9,426 |  |  |
| New Jersey-.. | 45 | 716 | 66 |  | 220 | 34. 420 |  |  | 4,830 |  |  |
| New York.-... | 2,504 |  |  | 56 | 2,156 | 371.56 |  |  | 36, 822 |  | 176 |
| North Carolina | 4,251 |  | 60 |  |  |  |  |  |  |  |  |
| North Dakota |  | 60 |  | 54, 566 |  |  | 5,018 |  |  |  | 176 |
| Oklahoma | 180 |  | 60 | 277 |  | 154, 158 |  |  |  |  |  |
| Oregon- |  |  |  |  |  | 40 |  | 32 |  | 80, 0.8 |  |
| Pennsylvania | 2,077 |  |  |  |  | 497, 144 |  |  | 4,026 |  | .-....... |
| Rhode Island... |  |  |  |  | 6, 600 | 54 |  |  | 86 |  |  |
| South Dakota |  | 60 |  | 7,689 |  |  | 463, 604 |  |  | 110 | 156 |
| Tennessec- | 4.921 |  |  |  |  | 32 |  |  |  |  |  |
| Texas.. | 148 | 5,690 | 95, 366 |  |  |  |  |  |  |  | ....-....- |
| Vermont |  |  |  |  | 2,260 |  |  | 235,788 | 11,802 |  |  |
| Virginia | 2,070 | 60 |  |  |  | 6, 246 |  |  |  |  | 176 |
| Washington- |  |  |  |  |  |  | 936 |  |  | 676, 740 |  |
| Wisconsin. |  |  |  | 907 |  |  |  |  |  |  | 1, 092, 571 |
| Wyoming, |  | 364 |  |  |  |  | 7,752 | 43,918 |  | 110 |  |
| Canada... | 8,913 |  |  | 516 | 132 | $\begin{array}{r} 13,622 \\ 332 \end{array}$ |  |  | 3,651 | 3,132 |  |
| Mexico. |  |  |  |  |  |  |  | 64 |  | 228 |  |
| Total | 1,053, 790 | 942, 434 | 1, 427, 650 | 2, 730,606 | 78.970 | 1, 210, 192 | 590, 040 | 367, 720 | 85, 460 | 839, 466 | 1,773, 096 |



Figure 3.-Percentage of Total Vehicle-Miles by Passenger Cars and Troces Traveled on Trips Exceeding Various Lengths.
adjoining States, and other States, providing another indication of the dispersion of motor-vehicle travel. It should be noted that even on these longer trips a very high percentage of the destinations was in the State of origin or in an adjoining State.

Table 12.-Destination of motor-vehicle travel in 11 States on one-way trips over 100 miles long

PASSENGER CARS

| State of origin | Destination of trips in- |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | State of origin | Adjoining States | Other States | Total |
| Florida | Percent $84.9$ | Percent 8 | Percent 6.6 | Percent 100 |
| Kansas | 55.5 | 37.4 | 7.1 | 100 |
| Louisiana | 74.5 | 19.2 | 6.3 | 100 |
| Minnesota | 89.0 | 8.0 | 3.0 | 100 |
| New Hampshire | 27.5 | 60.6 | 11.9 | 100 |
| Pennsylvania.. | 48.6 | 43.7 | 7.7 | 100 |
| South Dakota | 71.1 | 23.9 | 5. 0 | 100 |
| Utah. | 44.1 | 45.0 | 10.9 | 100 |
| Vermont. | 29.1 | 61.5 | 9.4 | 100 |
| Washington | 74.4 | 21.5 | 4.1 | 100 |
| Wisconsin . | 72.9 | 24.1 | 3.0 | 100 |
| TRUCKS |  |  |  |  |
| Florida | 88.3 | 8. 4 | 3.3 | 100 |
| Kansas_ | 61.7 | 35.0 | 3.3 | 100 |
| Louisiana. | 88.7 | 11.2 | . 1 | 100 |
| Minnesota. | 91.2 | G. 6 | 2.2 | 100 |
| New Hampshire | 17.7 | 63.5 | 18.8 | 100 |
| Pennsylvania | 41.1 | 56.1 | 2.8 | 100 |
| South Dakota. | 78.6 | 19.2 | 2.2 | 100 |
| Utah... | 64.1 | 34.2 | 1.7 | 100 |
| Vermont. | 13.9 | 73.1 | 13.0 | 100 |
| Washington | 80.6 | 16.2 | 3.2 | 100 |
| W isconsin . | 61.6 | 38.1 | . 3 | 100 |
| PASSENGER CARS AND TRUCKS |  |  |  |  |
| Florida | 86.0 | 8.5 | 5. 5 | 100 |
| Kansas. | 56.9 | 37.1 | 6. $\theta$ | 100 |
| Louisiana... | 81.7 | 15.1 | 3.2 | 100 |
| Minnesota_.... | 89.7 | 7.6 | 2.7 | 100 |
| New Hampshire. | 25.8 | 61.6 | 12. 6 | 100 |
| Pennsylvania.... | 47.7 | 45.1 | 7.2 | 100 |
| South Dakota | 73.9 | 22.1 | 4.0 | 100 |
| Utah ..... | 50.1 | 41.8 | 8.1 | 100 |
| Vermont. | 25.3 | 64.5 | 10.2 | 100 |
| Washington | 75.7 | 20.4 | 3.9 | 100 |
| W isconsin.. | 71.9 | 25.4 | 2.7 | 100 |

In addition to the distribution of the number of trips in various mileage ranges, the total vehicle-miles involved in these trips have also been computed and are presented in table 13 and figure 3. Here another aspect of motor-vehicle use is shown. For passenger cars, while trips of less than 5 miles constituted 38.4 percent of the total number of trips, they accounted for but 6.6 percent of the total vehicle-miles of travel partly or wholly on rural roads. Trips of less than 20
miles, accounting for 85.0 percent of all trips, involved but 40.9 percent of the total vehicle-miles of travel. Trips classified in mileage groups from 20 miles upward were responsible for a much larger percentage of travel than of total trips. In the higher mileage brackets, trips in the range from 50 to 249.9 miles were only 4.0 percent of the total number of trips, but they accounted for 28.6 percent of vehicle-miles traveled outside city limits.

These characteristics were similar but less pronounced for trucks. Thirty-four percent of the total number of one-way truck trips was classified as extending less than 5 miles, accounting for but 4.9 percent of the total vehicle-miles of travel; and trips of less than 20 miles, or 80.3 percent of all trips, constituted 33.9 percent of the mileage traveled wholly or partially on rural roads.

Table 13.-Number of trips and vehicle-miles traveled by vehicles which went outside city limits in 11 States

PASSENGER CARS

| Length of one-way trip from point of origin (miles) | Number of trips |  | Travel |  |
| :---: | :---: | :---: | :---: | :---: |
|  | 1,000 trips | Percent | Million vehicle-miles | Percent |
| 0 to 4.9-- | 835, 059 | 38.4 | 2,087. 6 | 6. 6 |
| 5.0 to 9.9 | 576, 295 | 26.5 | 4, 322.2 | 13.6 |
| 10.0 to 19.9 | 438, 222 | 20.1 | 6,573. 3 | 20.7 |
| 20.0 to 29.9 | 143,765 | 6. 6 | 3, 594. 1 | 11.3 |
| 30.0 to 39.9 | 61,923 | 2.8 | 2, 167.3 | 6.8 |
| 40.0 to 49.9 | 27, 784 | 1.3 | 1,250. 3 | 3.9 |
| 50.0 to 99.9 | 61,232 | 2.8 | 4,592. 4 | 14.4 |
| 100.0 to 249.9 | 25, 743 | 1.2 | 4,505.0 | 14.2 |
| 250.0 to 499.9 | 3,628 | . 2 | 1,360. 5 | 4.3 |
| 500.0 to 999.9 | 825 | . 1 | 618.8 | 1.9 |
| 1,000.0 and over | 489 | (1) | 733.5 | 2.3 |
| Total | 2, 174, 965 | 100.0 | 31,805.0 | 100.0 |
| TRUCKS |  |  |  |  |
| 0 to 4.9. | 184,952 | 34.0 | 462.4 | 4.9 |
| 5.0 to 9.9 | 138,916 | 25.5 | 1,041.9 | 11.0 |
| 10.0 to 19.9 | 113,521 | 20.8 | 1,702.8 | 18.0 |
| 20.0 to 29.9 | 41,855 | 7.7 | 1,046.4 | 11.0 |
| 30.0 to 39.9 | 20, 030 | 3.7 | 701.0 | 7.4 |
| 40.0 to 49.9 | 10,262 | 1.9 | 461.8 | 4.9 |
| 50.0 to 99.9 | 23,908 | 4.4 | 1,793.1 | 18.8 |
| 100.0 to 249.9 | 9,911 | 1.8 | 1,734. 4 | 18.3 |
| 250.0 to 499.9 | 1,034 | (1) .2 | 387.8 | 4.1 |
| 500.0 to 999.9 | 110 | (1) | 82.5 | . 9 |
| 1,000.9 and over | 45 | (1) | 67.5 | . 7 |
| Total | 544, 544 | 100.0 | 9,481.6 | 100.0 |
| PASSENGER CARS AND TRUCKS |  |  |  |  |
| 0 to 4.9 | 1, 020, 011 | 37.5 | 2,550. 0 | 6.2 |
| 5.0 to 9.9 | 715, 211 | 26.3 | 5,364. 1 | 13.1 |
| 10.0 to 19.9 | 551, 743 | 20.3 | 8,276. 1 | 20.1 |
| 20.0 to 29.9 | 185, 620 | 6.8 | 4,640.5 | 11.2 |
| 30.0 to 39.9 | 81,953 | 3.0 | 2, 868. 3 | 6.9 |
| 40.0 to 49.9 | 38,046 | 1. 4 | 1,712. 1 | 4.1 |
| 50.0 to 99.9 | 85, 140 | 3.1 | 6,385. 5 | 15.5 |
| 100.0 to 249.9 | 35, 654 | 1.3 | 6, 239. 4 | 15.1 |
| 250.0 to 499.9 . | 4,662 | . 2 | 1,748.3 | 4.2 |
| 500.0 to 999.9 | 935 | . 1 | 701.3 | 1.7 |
| 1,000.0 and over | 534 | (1) | 801.0 | 1.9 |
| Total | 2,719,509 | 100.0 | 41,286. 6 | 100.0 |

${ }^{1}$ Less than 0.1 percent.

## AVERAGE TRIP LENGTH ONLY 15.2 MILES

One-way truck trips less than 50 miles long constituted 93.6 percent of all truck trips outside city limits and accounted for 57.2 percent of all truck travel performed wholly or partially on rural roads. Trips less than 100 miles long accounted for 98.0 percent of such truck trips and 76.0 percent of all truck travel on rural roads. Corresponding figures for trips less than 250 miles were 99.8 percent of the number of trips and 94.3 percent of travel. It may be noted, however, that for distances
over 250 miles，the passenger car was bed molatively more than the truck．Thus，passenger－car and truck trips of 250 miles or more were 0.3 and 0.2 pereent，re－ spectively，of total momber of trips，while the travel gemerated wats 8.5 peremt of tobal passenger－rar ve－ hicle－milese and hut 3.7 pereont of all vehicle－miles of traved by trucks performed wholly or partially on ramal romes．

Computations have also been mate in this study of the mean and median lengthe of trips involving the use of romeds in mincorporated areas by residents of varions governmental jurisdictions．Results are given in table 14．For the purpose of this particular presenta－ tion，unincorporated areas and incorporated places with a population of 2,500 or less have been grouped to－ gether，because motor－vehicle owners resident in these two classifications were considered to have travel characteristies sufficiently similar to warrant their combination．For motorists of these smaller cities， rural roads，cither primary or purely local，are appeni－ mately as easily accessible as such roads are to strietly rural motorists．

Figure 4 shows that for both passenger sars and trucks the mean and median lengths of one－way trips that estended outside city limits were greatest for the largest place of residence of the owners．Thus the mean length of trips male by passenger cars owned by residents of unincorporated areas and places of 2,500 or less inhabitants was 10.6 miles，while for residents of cities having in excess of 100,000 persons the mean
length was 37.1 miles．（orrespondinger values for median（rip lengths were 5.9 and 16.3 miles．Figures for（rip）lengths for trucks were somewhat higher for all places of origin exeopt the largest cities．

The mean one－way trip length for combined pas－ senger－ar and truck travel for all governmental juris－ dietions was 15.2 miles，and the median trip， 7.4 miles．

The relative effect of the size of cities on highway use is also strikingly illustrated in tables 15 and 16 and figure 5 ，which show the average number of trips made outside cities by motor－vehicle owners of cities of various sizes．Is in provious tables，a single round trip starting inside the city and going to some place outside the city limits was considered as two one－way trips for purposes of mileage classification．Thus，the average passenger－car owner resident in citics havingr from 2,501 to 10,000 population went outside the city of residence for 75 round trips less than 10 miles long， or as it has been expressed in table 15，for 150 one－way trips less than 5 miles long．

Trips which extended for one－way distances of jot miles or more were made approximately the same num－ ber of times during the year by the average passenger－ car operators resident in all sizes of cities．However， the average number of trips extending beyond city limits in the shorter trip－length ranges decreased rapidly with increased size of the city of residence．For ex－ ample，table 15 shows that residents of cities having populations of over 100,000 made about one－half as many trips in the 20.0 －to 29.9 －mile trip－length range，

Table 14．－Length of trips traveled outside city limits by vehicles registered in the various population groups in 11 States PASSENGFR CARS

| State | Length of trips traveled by vehicles registered in－－ |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Unincorporated areas and incor－ porated places having a popula－ tion of 2,500 or less |  | Incornorated places having a population of－ |  |  |  |  |  |  |  | All incorporated places having a ropulation of more than 2，500 |  | All places |  |
|  |  |  | 2，501 to 10，000 |  | 10，001 to 25，000 |  | 25，001 to 100,000 |  | More than 100，000 |  |  |  |  |  |
|  | Mean？ | Median ${ }^{3}$ | Mean | Median | Mean | Median | Mean | Merian | Mein | Median | Mean | Median | Mean | Median |
| Florida <br> Kansas <br> Ioulisiana． <br> Minnesota <br> New Hampshire <br> Pennsylvania． <br> Soutlı Dakota． <br> itah＿ <br> Vermont <br> Washingon <br> W isennsin | Miles 11.4 | $\begin{gathered} \text { Miles } \\ 6.3 \end{gathered}$ | Miles 20.2 | $\begin{aligned} & \text { Miles } \\ & 9.9 \end{aligned}$ | Miles $29.7$ | Miles 14.6 | $\begin{array}{r} \text { Miles } \\ 30.5 \end{array}$ | Miles 14.9 | Miles 22.8 | Miles <br> 10.0 | $\begin{gathered} \text { Miles } \\ 23.5 \end{gathered}$ | Miles $11.3$ | Miles $16.1$ | ${ }_{\text {Miles }}^{\text {S．}}$ |
|  | 9.6 | 5． 0 | 23.2 | 11.6 | 29.7 | 15.6 | 34.6 | 16.0 | $41.1)$ | 18． 5 | 29.6 | 14.6 | 13.3 | 6.3 |
|  | 9.9 | 5． 5 | 17.9 | 7.8 | 22.1 | 11.2 | 26．6 | 13.5 | 74.2 | 57.5 | 24．3 | 12．6 | 14.2 | 6． |
|  | 11.4 | 6． 1 | 24.9 | 11.6 | 27.4 | 12．3 |  |  | 54.3 | 19.7 | 34.7 | 15.1 | 16.4 | 7.3 |
|  | 13.0 | 8.3 $\times 8.4$ | 13.7 | S． 6 | 30.1 | 11.1 | 27.2 | 16． 7 |  |  | 18.5 | 9.9 | 15.5 | 8.9 |
|  | 9.8 15.4 | 5.9 8.9 8.6 | $1,3.3$ 25.2 | 7.1 8.3 | 15.1 34.2 | $\times .4$ 10.0 | ${ }^{19.7}$ | 9.4 219.9 | 30.8 | 13． 6 | 17.5 30.9 | 8.5 9.9 | 13.5 $1 \times .7$ | 7.1 8.7 |
|  | 111． S | 4.4 | is． 0 | 8.2 | 38.3 | 15． 4 | 35.8 | 18．9 | 52.9 | 24.0 | 34.2 | 14.3 | 17.4 | 6． 6 |
|  | 9.3 | 5． 7 | 2012 | 9.5 | 24.5 | 11.4 |  |  |  |  | 21.6 | 4.8 | 11.7 | 6． 5 |
|  | 11.5 | 5．$x$ | 23.2 | $\times 1$ | 31）． i | 13.5 | 26． 2 | 14.7 | 40.5 | 211． 11 | 30 Fr | 14.1 | 14． 19 | 6.7 |
|  | 10.9 | 6.4 | 24.5 | 12.6 | 27.9 | 13.9 | 33． 2 | 8.4 | 48．2 | 25． 5 | 31.2 | 15.9 | 15．9 | 7.9 |
| A verage | 10．6 | 5． 93 | 17.2 | \＆．3 | 21.5 | 9.9 | 24.4 | 11.7 | 37.1 | 16． 3 | 22.9 | 10.0 | 14.6 | 7.2 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Floridid | 15.3 | 7.7 | 18． 1 | 9.5 | 32.3 | 15.7 | 21.8 | 13．2 | 20.6 | 9． 5 | 25． 8 | 10.1 | 19.4 | 87 |
| kansas．． | 10.9 | 6． 2 | 19．9 | 11.1 | 29.5 | 13.3 | 47.0 | 30.0 | 54.4 | 24.1 | 31.7 | 14.8 | 170 | 7.8 |
| Louisiana | 12.0 | 7.4 | 28． 7 | 21.3 | 42.6 | 30.6 | 57.6 | 36.8 | 52.0 | 27.1 | 44.8 | 26.8 | 21.5 | 9． 9 |
| Minnesota． | 15.1 | 7.5 | 26．6 | 11.7 | 38.1 | 211． 2 |  |  | 36.1 | 11．8． | 33.8 | 13.2 | 21.1 | 8.7 |
| New Hampshire | $\begin{array}{r}121 \\ 119 \\ \hline 19\end{array}$ | 8． 2.5 | 11.4 10.6 | 7．4 | 24.6 12.6 | 13.7 $6 . \%$ |  |  |  |  | 21．） 14 | 10.0 6.9 | 16.1 | 8.9 |
| Pennsylvania．．． | 11.9 21.7 | 6.5 10.5 | 10.6 37.3 3.8 | 5.0 17.6 | 12.6 64.8 | 6.8 44.7 | 18.1 66.9 | 8.4 45.3 | 19.3 | 0.4 | 14.2 | 6.9 28.0 | 130 <br> $2 \times 1$ | 6.7 12.3 |
| Ttah | 16.0 | 5.2 | 14.4 | 5． 5 | 20.3 | 12.0 | $41 .$. | 22.5 | 58.4 | 21）． 10 | 2x．8 | 4.7 | 19.9 | 6.4 |
| Vermont | 11.9 | 7.7 | 14.9 | 6． 3 | 45.11 | 13.5 |  |  |  |  | 21.2 | 7.4 | 14.2 | 7.1 f |
| Washington | 12.6 | 6． 5 | 18.3 | 9.7 | 32.5 | 21.3 | 24.8 | 15． 4 | 4．5． 4 | 31.3 | 31.6 | 19.2 | 17.5 | 8.4 8.3 |
| W isconsin ．－ | 11.6 | 6． 9 | 21.2 | 11.6 | 31.0 | 16．1 | 33.11 | 16． 5 | 35.9 | 19.0 | 29.2 | 15．0 | 16．${ }^{\text {a }}$ | 8.3 |
| A rerase． | 12.8 | \％． 1 | 17． 6 | 8.6 | 24．8 | 11.2 | 29． 7 | 13．7 | 35.6 | 15.3 | 26.0 | 11.1 | 17.4 | 8． 1 |
|  |  |  |  |  | SENOF | R CARS | 入り T！ | CKく |  |  |  |  |  |  |
| I verawe | 11.1 | （6．） 1 | 17.3 | x． 4 | 21.3 | 10． 11 | 25.1 | 12.0 | 36． 7 | 15．0 | 23.1 | 10.2 | 15．2 | 7.4 |

[^2]

Figtre 4.-Mean and Median Lengths of One-Way Trips That Went Odtside of City Limits by Vehicles Registered in Various Population Grours.

Table 15.-Average number of one-way trips of various lengths traveled outside city limits by passenger cars registered in various population groups

| T.ength of nue-way trip in miles from point of origin | Average number of one-way trips traveled by passenger cars registered in cities having populations of - |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | $\begin{gathered} 2,501 \\ \text { to } \\ 10,000 \end{gathered}$ | $\begin{gathered} \text { in,001 } \\ \text { to } \\ 25,000 \end{gathered}$ | $\begin{gathered} 25,001 \\ \text { to } \\ 100,000 \end{gathered}$ | $\begin{aligned} & \text { More } \\ & \text { than } \\ & 100,060 \end{aligned}$ |
| $\bigcirc 1049$ | 1.50 | 84 | 40 | 14 |
| S.0 to 9.9. | 126 | 84 | 56 | 25 |
| 10.0 to 19.9. | 96 | 92 | 6 fn | 34 |
| 20.0 to 29.9 | 38 | 32 | 30 | 16 |
| 300 to 39.8 | 18 | 18 |  | 10 |
| 40.0 to 49.9 | 8 | 10 | 6 | 4 |
| 50.0 and over | 26 | 28 | 28 | 22 |
| Total. | 462 | 348 | 234 | 128 |

Table 16.-Percentage of trips of various lengths traveled outside rity limits by passenger cars registered in various population groups

| liength of nne-way trip in miles from boint of origin | Fercentage of trins traveled hy passenger cars registered in cities having populations of |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | $\begin{gathered} 2,501 \text { to } \\ 10,000 \end{gathered}$ | $\begin{gathered} 10,001 \text { to } \\ 25,000 \end{gathered}$ | $\begin{gathered} 25,001 \text { to } \\ 100,000 \end{gathered}$ | $\begin{aligned} & \text { More than } \\ & 100,000 \end{aligned}$ |
| 0 to 4.9 | 32.5 | 24.1 | 17.1 | 10.9 |
| 5.0 to 9.9 | 27.3 | 24.1 | 23.9 | 21.9 |
| 10.0 to 19.9 | 20.8 | 26.4 | 25.6 | 26.6 |
| 20.0 to 29.9 | 8.2 | 9.2 | 12.8 | 12.5 |
| 30.0 to 39.9 | 3.9 | 5.2 | 6.0 | 7.8 |
| 40.0 to 49.9 | 1.7 | 2.9 | 2.6 | 3.1 |
| 50.0 and over | 5.6 | 8.1 | 12.0 | 17.2 |
| Total. | 100.0 | 100.0 | 100.0 | 100.0 |

one-third the number in the 10.0 - to 19.9 -mile range, and one-tenth as many trips in the 0 to 4.9 -mile range, as did residents of cities having populations of 2,501 to 10,000.

This smaller number of short trips by vehicles owned in the larger cities is to be expected because of the greater area covered by the larger cities. Since the analysis involved only those trips that extended beyond city limits, a large number of the shorter trips made hy residents of large cities did not extend beyond the city limits and are not included in those trips shown here. It is probable that vehicle owners resident in the larger cities make as many, or possibly more, individual trips per year as do residents of the smaller eities. Many of these trips, however, are confined within the rather extensive city limits.

## DATA EXPLAIN TRAFFIC CONGESTION NEAR LARGE CITIES

These data should not be considered as evidence that the vehicle owner in smaller cities makes more trips per year than does the owner resident in the larger cities. Rather, the data are an indication that the rural highway is of greater interest to the vehicle owner of large cities for long trips than for short ones, and that the rural highways serve vehicle owners resident in the smaller cities for local travel purposes to a much greater extent proportionally than they do residents of large rities. That is, for those trips extending to rural portions of the highway system there is proportionally a greater interest in longer trips by the residents of a large city than by the residents of small places. Table 16 illustrates this point. Passenger-car owners resident in cities of 2,501 to 10,000 population made 32.5 percent of their trips involving rural highways within the 0 to 4.9 -mile trip-length range, while the residents of cities of over 100,000 population made only 10.9 percent of their out-of-city trips within that travel range. The percentages for trips of 50 miles or more one way were 5.6 and 17.2 percent, respectively.

These trip-length data indicate that much of the dense traffic often resulting in congestion on rural portions of highways near city limits is composed of a multitude of cars making short trips oriminating within the city. Heavily traveled sections of highway extend greater distances from the limits of large cities than from smaller cities because of the greater con mitration of vehicles in the city and also becanse of the higher percentage of longer trips.

Facts derived from road-use data provide important. guidance in outlining future highway policies, in regard to beth physical and fimancial planis. The extent and location of the improvements made on the primary highway system are of eonsiderable importance to aill residents of the State. Except for those who live in the largest cities, all motorists in the State use the primary highway system more than any other class of roads. The condition of this sytem, therefore, is of comparable interest to all motorists except those residing in the largest cities. The latter do the greatest part of their traveling on city streets. On the other hand, it is significant that these motorists, resident in large cities, because of their large numbers, are responsible for a considerable amount of the total travel on primary highways. Therefore their interest in such roads, although comparatively less per motorist than for other residents of the State, is still very large iu the aggregate.

[^3]
# A NEW VIBRATORY MACHINE FOR DETERMINING THE COMPACTIBILITY OF AGGREGATES 

BY THE DIVISION OF TESTS, BUREAU OF PUBLIC ROADS

Reported by J. T. PAULS, Senior Highway Engineer, and J. F. GOODE, Junior Highway Engineer

THE IMPORTANCE of compaction in highway construction has long been recognized. Recent laboratory and field investigations have repeatedly emphasized the value of thorough consolidation in both the base and surfacing courses. Thorough compaction is known to produce the following desirable results:

1. It increases interlocking of the aggregate particles, which is the primary factor in developing a high degree of stability.
2. It retards the entrance of moisture, thus preventing excessive loss of stability under adverse service conditions.
3. It reduces the flow of air and water through bituminous mixtures and is therefore an effective means of lessening damage from weathering and film stripping.

In order to obtain consistently a high degree of consolidation during construction, it is essential to know in advance the limits of compactibility of the materials used. Such tests as have been employed to determine the attainable density of materials, among which are dry rodding, shaking, and various molding test.s involving tamping and direct compression, do not always give consistent results. Furthermore, as will be shown in this report, they fail to show the maximum compactibility limits of many aggregates.

The Bureau has been using for some time a small vibrator ${ }^{1}$ called the roids determinator for the determination of voids in sheet asphalt aggregates. This vibrator, however, does not give consistent results for mixtures containing high percentages of dust; and, since the testing cylinder has a capacity of only 25 cubic centimeters, it is not suitable for testing aggregates containing large fragments. Accordingly, a new machine has been developed that produces more consistent results and hicher densities, and which appears to be rqually satisfactory for all gradations of aggregates commonly used in both hase and surface construction.

APFARATUS CONSISTS ESSENTIAIG OF A VIRKATINK: TABIEE
The general : appearame of the menly developed test apparatus is shown in the emer illustration. The primeiple of its operation is more clearly brought out in figure 1.

The machine consists essentially of a floating table that is made to move vertically in periodic motion by rotating eccentric masses rigidly comected to its lower surface. The table is a steel plate 13 by 24 inches in size and $3 / 4$ inch thick. It is supported at each corner by a helical spring through which there is a vertical guidepost on which the table slides.

On the lower surface of the table, mounted parallel to the long axis of the plate, are two shafts running in ball bearings and geared to rotate at the same speed but in opposite directions. Four steel blocks of equal

[^4]size and weight are symmetrically monnted at the ends of the two shafts, one at each end of each shaft. The size of these blocks and the speed at which they are rotated determine the magnitude of the unbalanced force. Since the two shafts rotate in opposite directions only vertical accelerations are imparted to the system.

The weight shafts are rotated at speeds of 4,300 , 2,500 , or 1,500 revolutions per mimute hy a 3 -horsepower electric motor with a 3 -speed, $V$-belt drive.

By trial it was found that the best compaction was obtained with a total eceentric weight of 1,100 grams located $1 \frac{1}{16}$ inches off center and rotating at 4,300 revolutions per minute. For these particular conditions the maximum centrifugal foree developed by each of the four eecentrie masses is thenetedeally about 33.5 pounds. In the extreme upper and lower positions these forecs add to give a theoretical total vertical resultant of about 1,350 pounds while at the midpenint between these positions the forres developed by the weights on one shaft exactly balance those of the other shaft and the total horizontal resultant is 19 pound.

At a frequency of 4,300 cyeles per minute a powerful vibration is developed in the entire mass.
The assembly for holding the aggregate to be tested is bolted to the top of the vibrating plate or table. It is shown in section in figure 2. Its essential parts are a base plate and bottom plunger bolted to the table, a cylinder fitting over the bottom plunger and resting on a rubber support, and a top plunger which rests on the test material in the cylinder.

A micrometer dial mounted on a suitable base is used in conjunction with a series of calibrated gage blocks to measure the thickness of the compacted specimen without removing it from the eylinder.

The top plunger imposes a dead had of 1.75 pounds per stpuare inch on the sample to be compacted. This doad load gemerally provides sufficient confinement to flaten the top of the sperimen and to present segregation of the particle sizes. Both the fop and botom phugers have just sufficient clearane withon the eydinder to allow free vertical movement during vibration, and each is litted with thee bromze guide strips to maintain it in a position parallel to the axis of the cylinder. The loss of fine aggregate is held to a minimum by the insertion of close-fitting pasteboard gaskets or pads above and below the test specimen. A suitable correction is made in the measured height of the specimen to allow for the final thickness of the pads.

## equipment adaptable for testing different aggregates

In making a test, the first step is to obtain an initial or zero reading with the micrometer dial on the combined height of the two plungers with the two pasteboard pads compressed between the plungers by vibration for a short period. For this zero reading a steel spacer gage of the approximate thickness of a compacted specimen is inserted under the dial so that its $3 / 4-$ or


Figure 1.-Essential Elements of the Vibratory Compactor.

1 -inch range of travel will not be exceeded when the specimen is in place.

For tests in the 4 -inch cylinder, which is the one used for aggregates up to about 1 -inch maximum size, sufficient aggregate is used to produce a compacted specimen approximately $1 \frac{1}{2}$ to $13 / 4$ inches high. This requires about 750 grams of aggregate.

If desired, a much smaller cylinder may be used when testing fine aggregates such as soil, sand, rock dust, or sheet asphalt aggregate, and the depth of the compacted specimen may be reduced to 1 inch or less and its weight to as litile as 75 srams. For very large agregates, a batger cytinder should be ased and the thickness of the compacted specimen should be inereased so that it is at least one-half to three-fourths inch more than the nominal diameter of the largest individual aggregate particle. The weight of the top plunger should be such that the dead load is approximately 1 pound per square inch per inch of depth of the compacted specimen.

It is essential that the loose aggregate be placed in the cylinder without segregation. When the aggregate to be tested has a large percentage retained on the No. 10 sieve it has been found that the addition of about 50 to 70 cubic centimeters of kerosene to 750 grams of aggregate aide greatly in preventing segregation and does not interfere with compaction. The most satisfactory amome of kerosene seems to be that which will just fill the voids in the compacted agregrate.

Materials such as fine soil, sand, clay, ete, are not particularly subject to segregation and, because of the greater difliculty with which air is forced out of them when wet, do mot always compact as well with kerosene as without. They are therefore tested dry.


Figure 2.-Cylinder and Plunger Assembly With Measuring Device.

For determining whether or not to use kerosene it has been found that in general the following conditions will govern:

1. For aggregates having less than 35 percent passing the No. 10 sieve, use kerosene.
2. For aggregates having more than 50 percent passing the No. 10 sieve, test dry.
3. For aggregates having more than 35 percent and less than 50 percent passing the No. 10 sieve, test both with and without kerosene and report the higher density value obtained.

## VIBRATION FOR 20 MINUTES ADOPTED AS STANDARD PROCEDURE

All aggregates should be oven-dried before testing, since very small amounts of water or other liquid, as distinguished from the relatively large amount of kerosene added in testing coarse materials, have a marked bulking effeet which interferes with the obtaining of accurate test results. Drying is also necessary in order to obtain the true sample weights for use in caleulating the density after vibration. Lumps or clods of clay in the aggregate impair the accuracy of the test and should be thoroughly broken down before placing the sample in the eylinder for compaction.

After the material is placed in the cylinder, with a pasteboard pad underneath and another on top, the upper plunger is inserted and the assembly is vibrated for a period of 20 minutes. The final reading is taken with the dial, and from this reading and the initial reading the over-all volume of the material in the cylinder is calculated. This rolume, the dry weight, and the apparent specilic gravity of the aggregate are used in calculating the density. In this report density is expressed as the percentage of aggregate volume per unit of total volume.

The method of detemming this percentage is illustrated with is typieal example:

Apparent specific gravity ${ }^{2}$ of aggregate_-.................. 2. 67
Weight of aggregate sample, grams-.--------------------- 736
Volume of vibrator-compacted sample, cubic centimeters. 313.9
Unit weight of compacted sample (grams per cubic centimeter) $736 / 313.9$
2. 35

Density of compacted sample, (percent) $\frac{2.35}{2.67} \times 100 \ldots \ldots-$ - $\quad 88.0$
The densities of a number of aggregates were determined for various periods of vibration up to a maximum of 60 minutes. The results of these tests are shown in figure 3. The asphaltic concrete aggregate, the sheet asphalt aggregate, and the fine sand showed practically no increase in density after 20 minutes of vibration. The sand-clay and the sand-clay-gravel each showed an apparent increase of 1.1 percent in density for the time increment from 20 to 60 minutes, and the micaceous soil showed an increase of 1.4 percent. It was found, however, that loss of dust, which became quite noticeable late in the test because of wear on the gaskets, accounted for most of the reduction in volume and consequent apparent increase in density after the initial 20 minutes of vibration. Vibration for a period of 20 minutes has, therefore, been adopted as regular procedure for the test.

The results of compaction tests on three different types of aggregate are shown in table 1 and demonstrate the ability of the apparatus to produce results that check. The maximum variation in results for these tests was slightly under 0.5 percent. However, for routine testing by various operators, this degree of accuracy probably could not be expected.

Table 1.-Consistency of check tests using the vibratory compacting machine

| Type of aggregate | Density (aggregate volume per unit of total volume) |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{aligned} & \text { Test } \\ & \text { No. } 1 \end{aligned}$ | $\begin{gathered} \text { Test } \\ \text { No. } 2 \end{gathered}$ | $\begin{aligned} & \text { Test } \\ & \text { No. } 3 \end{aligned}$ | $\begin{gathered} \text { Test } \\ \text { No. } 4 \end{gathered}$ | Average |
|  | Percent | Percent | Percent | Percent | Percent |
| Sand-clay- | 79.4 | 79.6 | 79.6 | 79.6 | 79.6 |
| Sand-clay-gravel-.- | 86.7 | 86.9 | 86.7 | 87.1 | 86.9 |

In the following tables and discussion, the results of a number of compaction tests using the vibratory machine and several other methods of compaction are shown. Table 2 shows the comparative effects of vi-

[^5]

Figure 3.-Densities of Various Types of Aggregates and Their Rates of Consolidation.
bration and direct compression on the grading of the aggregate and demonstrates that little or no change in grading was produced by the vibratory method of compaction, whereas direct compression resulted in sufficient crushing to alter materially the grading of the aggregate samples.

HIGHER DENSITIES OBTAINED BY VIBRATORY METHOD THAN BY OTHER METHODS
A comparison of densities obtained by several methods of compaction on various types of aggregates is shown in table 3. In the upper section of the table dealing with the aggregates for base courses, the densities obtained from circular-track test sections built and compacted under the most favorable laboratory conditions agree closely with those obtained by the vibratory

Table 2.-Effect of compaction by compression and vibration on breakage of various aggregates

| Type of aggregate | Method of compaction | Grading, total aggregate passing- |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 1-inch sieve | $\begin{gathered} 3 / \text {-inch } \\ \text { sieve } \end{gathered}$ | $\begin{aligned} & \text { 38-inch } \\ & \text { sieve } \end{aligned}$ | $\text { No. } 4$ sieve | $\begin{aligned} & \text { No. } 10 \\ & \text { Sieve } \end{aligned}$ | $\begin{aligned} & \text { No. } 40 \\ & \text { sieve } \end{aligned}$ | $\begin{aligned} & \text { No. } 100 \\ & \text { sieve } \end{aligned}$ | No. 200 sieve |
| Graded, fine-high dust content Do. | None.. Vibration | Percent | Percent | Percent | Percent 100.0 100.0 | $\begin{array}{r} \text { Percent } \\ 99.7 \\ 99.7 \end{array}$ | Percent $\begin{aligned} & 82.5 \\ & 82.2 \end{aligned}$ | Percent | Percent 15.5 14.7 |
| Graded, fine-low dust content Do | None $\qquad$ <br> Vibration |  |  |  | $\begin{aligned} & 100.0 \\ & 100.0 \end{aligned}$ | $\begin{aligned} & 99.4 \\ & 99.8 \end{aligned}$ | $\begin{aligned} & 82.3 \\ & 82.8 \end{aligned}$ |  | 2.4 <br> 2.8 |
|  | None Compression, $3,000 \mathrm{lb} . / \mathrm{sq}$. in |  |  |  |  | $\begin{aligned} & 100.0 \\ & 100.0 \end{aligned}$ | $\begin{aligned} & 75.2 \\ & 76.9 \end{aligned}$ | $\begin{aligned} & \text { 18.9 } \\ & \text { 25. } 6 \end{aligned}$ | 2.2 7 |
| Graded, coarse-medium dust con Do. | None $\qquad$ Vibration | $\begin{aligned} & 100 \\ & 100 \end{aligned}$ | $\begin{aligned} & 92.5 \\ & 92.5 \end{aligned}$ | $\begin{aligned} & 75.8 \\ & 75.8 \end{aligned}$ | $\begin{aligned} & 66.8 \\ & 66.6 \end{aligned}$ | $\begin{aligned} & 63.7 \\ & 63.3 \end{aligned}$ | $\begin{array}{r} 48.6 \\ 48.3 \end{array}$ |  | 9.4 9.4 |
| Graded, coarse-low dust content Do. |  | $\begin{aligned} & 100 \\ & 100 \end{aligned}$ | $\begin{aligned} & 98.3 \\ & 98.2 \end{aligned}$ | $\begin{aligned} & 76.7 \\ & 82.0 \end{aligned}$ | $\begin{array}{r} 55.9 \\ 61.6 \end{array}$ | $\begin{array}{r} 50.0 \\ 52.0 \end{array}$ | $\begin{aligned} & 38.5 \\ & 39.8 \end{aligned}$ | $\begin{aligned} & 11.4 \\ & 14.4 \end{aligned}$ | 1.8 <br> 4.1 <br> 17 |
| Graded, coarse--high dust conten Do | None Compression, $3,000 \mathrm{lb}$./sq. in | $\begin{aligned} & 100 \\ & 100 \end{aligned}$ | $\begin{aligned} & 90.8 \\ & 92.5 \end{aligned}$ | 77.0 80.2 | $\begin{aligned} & 66.3 \\ & 69.8 \end{aligned}$ | $\begin{aligned} & 47.2 \\ & 53.2 \end{aligned}$ | $\begin{aligned} & 31.7 \\ & 36.0 \end{aligned}$ |  | 17.3 21.0 |

test. The other methods of compaction shown, with few exceptions, gave considerably lower densities. Direct compression appears to be quite effective for the fine-grained materials, but, as previously shown, the crushing of the aggregate in this test renders the results somewhat unsatisfactory

Table 3.-Comparison of aggregate densities obtained by various methods of compaction
base course materials:


Shoot asphalt, D. C
Sheet asphalt, Ohio
Fine bituminous concrete, ohio-
Do Dium bituminous concrete, Ohio..................................
Coarse bituminous concrete. Ohio
Do.
be made as rich in bituminous material as hot paving mixtures of comparable aggregate grading.

To attain consistently during construction a satisfactory degree of compaction for any particular material it is necessary to know in advance its compactibility limit, to have an idea of how closely this limit may be approached by practical construction methods, and how closely it needs to be approached to insure satisfactory behavior provided the materials are otherwise satisfactory. Tests have been made on a large number of materials. The few typical results given in table 4 illustrate the relations between field densities and compactibility limits, as determined by the vibratory machine.

For the plastic sand-clay and sand-clay-gravel materials that have been found by various tests to be suitable for base-course construction, the compaction obtained during construction appears to be the deciding factor influencing service behavior. The importance of consolidation is particularly well illustrated in the behavior of the plastic sand-clay-gravel referred to in the footnote of table 4. This material, which is representative of a group of materials that showed similar behavior, was placed in the test section as a base course with insufficient moisture to permit compaction to the density obtained in the vibratory test. It failed in service as soon as unfavorable sub-base moisture conditions were imposed. It was later scarified and recompacted with a higher moisture content. It was then easily compacted to essentially the same density as was obtained in the vibratory test and gave excellent service under very adverse moisture conditions.

Table 4.-Relation of density of soil-type bases to service behavior for base course materials

| Character of material testei |  |  |  |  | Density (aggrezate volume per unit of total volume) |  | Bohavior in test sections |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Typo |  | Grading, total aggregate passing - |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| Micacejus soil | 0 | $\begin{array}{r} \text { Per- } \\ \text { cent } \\ 98 \end{array}$ | $\begin{array}{r} \text { Per- } \\ \text { cent } \\ 76 \end{array}$ | Percent 62 | Percent 57.3 | Percent 53.4 | Unsatisfactory. |
| Micacevus soil with 3 percent cement. | 0 | 98 | 77 | 63 | 57.5 | 57.8 | Satisfactory. |
| Micacejus soil with 11 percent coment. | 0 | 98 | 78 | 65 | 57.0 | 57.0 | Do. |
| Sand-clay .-.-.... | 0 | 100 | 71 | 25 | 80. 6 | 74.8 | Do. |
| DO. | 6 | 100 | 51 | 25 | 814 | 81.2 | Do. |
| Do. | 9 | 100 | 68 | 27 | 796 | 79.5 | Fairly satisfactory. |
| Sand-clay-gravel | 0 | 54 | 34 | 17 | 89.7 | 82.8 | Satisfactory. |
| Do.l (Same material) ${ }^{1}$ | 5 | 48 | 31 | 15 | 88.0 | 83.2 | Unsatisfactory. |
| Du. - $^{\text {(Same material }}$ | 5 | 48 | 31 | 15 | 88.0 | 87.0 | Satisfactory. |

When this material was plac-d in the roadway it had so low a moisture content that it did not compact to a satisfactory density. It failed early in service but when remixed and relaid with the corrct moisture content, it compacted to within 1 percant of the density obtained by vibration and gave satisfactory service.

## VIBRATORY METHOD USEFUL IN BLENDING AGGREGATES TO <br> OBTAIN DENSE MIXTURE

In highway base- and surface-course construction it is frequently necessary to blend two or more aggregates to provide a material suitable for the intended use. The vibratory compactor provides a means by which the best combination of two or more available aggregates may be determined. The application of this test to the design of aggregate blends and bituminous mixtures will be discussed in connection with figures $4,5,6$, and 7 .

Figures 4 and 5 illustrate two methods of using the vibratory compactor to obtain the densest combination


Figure 4. - Blending Curves for Crushed Gravel, Sand, and Pulverized Soil. Aggregate Numbers Correspond to Those in Table 5.
of three different aggregates for use as a base-course material. For this type of construction the combination of the arailable materials that gives the densest mixture is generally the most desirable. The three aggregates used in producing the blending curves of figures 4 and 5 were crushed gravel (1-inch maximum size), fine sand, and a pulverized soil. The densities and the gradings of the individual constituents and the various blends are shown in table 5.

In the first method illustrated in figure 4, an initial series of blends was made of the gravel and sand and the densest blend of these two materials was determined. This blend, designated as aggregate 3 in table 5, was then blended in various proportions with the pulverized soil, the densest blend in this series being presumably the densest possible blend of the three materials. This blend, designated aggregate 9 in table 5, had a density of 87.9 percent aggregate solids and the following composition: Gravel 72 percent, sand 18 percent, and pulverized soil 10 percent.


Figure 5.-Blending Curves for Crushed Gravel, Sand, and Pulverized Soll. Aggregate Numbers Correspond to Those in Table 5.

A less dense blend of the gravel and sand, selected at random and designated aggregate 4 in table 5, was also blended with the pulverized soil as shown in figure 4. The highest density obtained by blending with aggregate 4 was lower than that obtained with aggregate 3, indicating that the procedure of selecting the densest combination of the coarse materials for blending with the fines was the correct method.

In the second method, illustrated in figure 5, the order of the tests was reversed. The initial series of blends was made with the sand and pulverized soil. The densest blend of these, designated aggregate 19 in table 5 , was then blended with the gravel. The two methods gave identical final results both as to maximum density and proportions of the three constituents, the density at the high point of the second curve being again 87.9 percent and the proportions of material being: Gravel 72 percent, sand 18 percent, and soil 10 percent.

It is of interest to note that the grading of the densest blend of these three materials, which were selected more

Table 5.-Densities and gradings of blended aggregates, sand-clay-gravel base-course type



Figure 6.-Blending Curves for Bituminous Concrete. Aggregate Numbers Correspond to Those in Table 7.
or less at random, conformed to the grading requirements now recommended for base-course construction. This relationship is shown in table 6.

In figure 6 is shown the application of method 1 in blending crushed stone, sand, and limestone dust for a typical bituminous concrete aggregate. The densities and gradings of the various blends are shown in the lower section of table 7 . As shown in figure 6 a maximum density of 89.8 percent solids was obtained, using

Tabie 6.-Comparison of grading obtained by blending sand, clay, and gravel for maximum density, with recommended grading requirements for base-course construction

|  | Grading, total aggregate passing- |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{aligned} & \text { 1-inch } \\ & \text { sieve } \end{aligned}$ | $\begin{array}{\|c\|} 3 / 4-\text { inch } \\ \text { sieve } \end{array}$ | 3/8-inch sieve | $\text { No. } 4$ sieve | No. 10 sieve | $\begin{aligned} & \text { No. } 40 \\ & \text { sieve } \end{aligned}$ | $\begin{aligned} & \text { No. } 200 \\ & \text { Sieve } \end{aligned}$ |
|  | Percent | Percent | Percent | Perient | Percent | Percent | Percent |
| blend............... | 100 | 87.0 | 62.9 | 45.9 | 28.5 | 24.3 | 9. 2 |
| A. A. S. H. O. specification for type B, sand-clay-gravel base | 100 | 70-100 | 50-80 | 35-65 | 25-50 | 15-30 | 5-15 |



Figure 7.-Blending Curve for Crushed Limestone and Artificial Limestone Sand. Aggregate Numbers Correspond to Those in Table 7.

15 percent of limestone dust with the densest blend of the stone and sand (aggregate 35, table 7).

VIBRATOR ENABLES DESIGN OF MIXTURES WITHOUT OVERFILLING VOIDS

Here is an example where too dense an aggregate for practical use in bituminous concrete was obtained since the voids remaining would only permit the use of about 10 percent by volume or approximately 5 percent by weight of asphalt. To produce a practical aggregate it would be necessary to reduce its density. This would best be accomplished by reducing the dust content since the densest possible combination of the coarse fractions is always desirable. Reduction of the dust content to range between $3 \frac{1}{2}$ and 6 percent would reduce the density to between 85 and 86.5 percent solids, thus permitting the use of approximately 6 to 7 percent asphalt by weight and bringing the design into line with established practice.

Figure 7 shows a blending curve for bituminous concrete aggregate composed of crushed stone and artificial limestone sand without dust. This type of aggregate is used extensively in Ohio. The densities and gradings of the constituents and blends are shown in the upper section of table 7 . This type differs from the previous

Table 7.-Densities and gradings of blended aggregates of the type used in bituminous concrete
WITHOUT MINERAL FILLER

| Identification | Composition of aggregate |  |  |  | Density (aggregate volume per unit of total volume) | Grading, total aggregate passinc- |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Coarse |  | Fine |  |  | $\begin{aligned} & 3 \text { i-inch } \\ & \text { sieve } \end{aligned}$ | $\begin{aligned} & 3,8 \text {-inch } \\ & \text { sieve } \end{aligned}$ | No. 4 sieve | No. 10 sieve | No. 40 sieve | $\begin{aligned} & \text { No. } 200 \\ & \text { sieve } \end{aligned}$ |
|  | ; Type | Amount | Type | Amount |  |  |  |  |  |  |  |
|  | Crushed stone.... | Percent 10075605450300 | Artificial sand. | $\begin{array}{r} \text { Percent } \\ 0 \end{array}$ | Percent 69.8 | Percent | Percent 90.0 | Percent |  | $\begin{gathered} \text { Percent } \\ 0 \end{gathered}$ |  |
|  |  |  |  | 25 | 85.9 | 100 100 | 90.0 92.5 | $\begin{array}{r} 5.0 \\ 28.8 \end{array}$ | $\begin{aligned} & 0 \\ & 23.3 \end{aligned}$ | $9.3$ | ${ }^{0} .9$ |
|  |  |  | - do.... | 40 46 | 87.3 87.5 | 100 | 94.0 | 43.0 | 37.3 | 14.8 | 1.41.6 |
|  |  |  | do. | 50 | 87.4 85.4 | 100100 | $\begin{aligned} & 94.0 \\ & 95.0 \\ & 97.0 \end{aligned}$ | $\begin{aligned} & 52.5 \\ & 71.5 \end{aligned}$ | 46.6 <br> 65. 2 | 18.5 |  |
|  |  |  | do | 70 | 8.181.1 |  |  |  |  |  | 1.8 1.8 2.5 |
|  |  |  | do | 100 |  |  |  |  |  | $\begin{array}{r} 25.9 \\ 37.0 \end{array}$ | 2. 5 |
| WITH MINERAL FILLER |  |  |  |  |  |  |  |  |  |  |  |
| 32 | Crushed stone | 100 | Sand... | 0 15 |  |  | ${ }^{0}{ }^{0}$ | ${ }_{15}^{0} 0$ | ${ }_{11}{ }^{7}$ |  |  |
| 33 | -----do.-- | 85 70 |  | 15 30 | 74.7 81.7 | 100 | 15.0 30.0 | 15.0 30.0 | 11.7 <br> 23 <br> 1 | 4.5 9.0 | . 3 |
| 35. | - do. | 60 | do | 40 | 83.0 | 100 | 40.0 | 40.0 | 31.2 | 12.0 | . 8 |
| 36. | do. | 50 | do | 50 | 82.4 | 100 | 50.0 | 50.0 | 39.0 | 15.1 | 1.0 |
| 37. | do. | 0 |  | 100 | 70.8 |  |  | 160.0 | 78.0 | 30.1 | 2.0 |
| 35. | Aggregate No. 35 | 100 | Limestone dust. | 0 | 83.0 | 100 | 40.0 | 40.0 | 31.2 | 12.0 | . 8 |
| 38. | - .-. do........-- | 90 | -. do.- | 10 | 88.3 | 100 | 46.0 | 46.0 | 38.1 | 20.8 | 10.4 |
| 39. | -do | 85 |  | 15 | 89.8 | 100 | 49.0 | 49.0 | 41.5 | 25.2 | 15.2 |
| 40. | do | 80 |  | 20 | 89.2 | 100 | 52.0 | 52.0 | 45.0 | 29.6 | 19.9 |
| 41. | -do | 75 |  | 25 100 | 88.1 | 100 | 55.0 | 55.0 | 48.4 | 34.0 | 24.7 |
| 42. | -do | 0 | do. | 100 | 72.4 |  |  |  |  | 100.0 | 96.4 |



Figure 8.-Blending Curve for Sheet Asphalt Sand and Commercial Limestone Dust. Aggregate Numbers Correspond to Those in Table 8.
example in that the densest combination of the two aggregate constituents provides sufficient void space for the bituminous material. It is therefore desirable to use the densest blend, this being easily found by means of the vibratory compactor.

The aggregate void space in the densest blend shown in figure 7 would permit the use of about 6 percent asphalt, which conforms approximately to the design used successfully in Ohio with the same type of aggregate.
The results of vibratory compaction tests on blends of fine sand and limestone dust to give a dense aggregate for sheet asphalt are shown in figure 8. The densities and gradings of the two constituents and the blends are given in table 8. A maximum density of 79.9 percent solids was obtained with the blends consisting of 70 percent fine sand and 30 percent dust and 75 percent sand and 25 percent dust. Again the 20 percent voids in this blend provide insufficient space for the proper amount of asphalt and the high dust content would produce an aggregate that would be difficult to mix and handle.

Table 8.- Densities and gradings of blended aggregates of the type used in sheet asphalt

| Sample identifcation | Composition of aggregate |  | Density (aggregate volume per unit of total volume) | Grading, total aggregate passing- |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Sand | Limestone dust |  | No. 10 sieve | No. 40 sieve | No. 80 sieve | No. 200 sieve |
|  | Percent | Percent | Percent | Percent | Percent | Percent | Percent |
| 43 | 100 | 0 | 71.2 | 100 | 81.3 | 33.8 | 3.5 |
| 44 | 95 | 5 | 73.4 | 100 | 82.2 | 37.1 | 8.1 |
| 45 | 90 | 10 | 75.6 | 100 | 83.2 | 40.4 | 12.7 |
| 45 | 85 | 15 | 77.4 | 100 | 84.2 | 43.7 | 17.3 |
| 47 | 80 | 20 | 78.9 | 100 | 85.1 | 47.0 | 21.9 |
| 48. | - 75 | 25 | 79.9 | 100 | 86.0 | 50.4 | 26.4 |
| 49. | 70 | 30 | 79.9 | 100 | 86.9 | 53.7 | 31.0 |
| 50 | 50 | 50 | 78.5 | 100 | 90.7 | 66.9 | 49.4 |
| 51 | 25 | 75 | 73.7 | 100 | 95.3 | 83.5 | 72.4 |
| 52 | 0 | 100 | 68.1 | 100 | 100.0 | 100.0 | 95.3 |

In this type of construction the problem of design utilizing the vibratory compactor might be attacked from either of two angles:

1. The amount of dust could be set on the basis of well-established practice, which would call for considerably less than 25 percent dust, and the asphalt content required to fill the void space could then be determined by vibratory tests on the fixed aggregate blend.


Figure 9.-Variation in Density Obtained by Vibration and Corresponding Asphalt Capacity of Sheet Asphalt Aggregates Contaring Various Percentages of Limestone Dust Filler.
2. The amount of asphalt could be set also on the basis of well-established practice, and the amount of dust to be used could then be adjusted by vibratory compaction tests on a series of blends covering a narrow range of dust contents to produce an aggregate that would hold the fixed amount of asphalt.

Figure 9 illustrates the relation between asphalt capacity as determined by vibratory compaction tests and the dust content of the aggregate.

The use of the compaction test to coordinate content of bituminous materials and capacity for them appears to offer special possibilities in the design of dense surfacing mixtures where overfilling of the voids might seriously impair stability.

## SIMMARY

As shown in the preceding discussion the vibratory test appears to offer valuable aid in connection with the following problems of design and construction:

1. Establishment of a definite optimum degree of compaction toward which field compaction may be aimed.
2. Determination of the best combination of two or more available aggregates for base-course or surface construction.
3. Investigation of the capacity for bituminous materials of certain aggregates to insure against overbituminization.
4. Modification of aggregate blends to permit the use of sufficient bituminous material for workability and surface sealing without overfilling the void spaces and destroying stability.


Figure 5.- Number of Trips Made by Vehicles Registered in Various Incorporated Places According to Length of One-Way Trifs That Went Outside of City Limits.

The expenditure of motor-vehicle tax revenue on secondary highways and local roads does not create highway user benefits as widespread as those created by primary road expenditures, because these roads are used to a much smaller extent than the primary system or city streets. The use of secondary highways and local roads by residents of unincorporated areas and small towns is comparable with the use of local city streets by city residents.

The preceding data show the extensive use of motor vehicles for local travel and the self-imposed limitations on their use which results in a large percentage of their travel being performed within a surprisingly small area around their place of ownership. Accordingly, those roads radiating from centers of population are very important links in the highway system. It is apparent, then, that appreciable portions of the expenditures of motor-vehicle tax revenue on the primary system, in order to benefit the large cities properly, must be so applied as to alleviate the conditions of congestion
and accompanying danger that exist within slort distances of population centers.

Data on the use of rural roads and city streets and the extent of such use cannot be used alone to determine adequate plans for a highway program. Road-use data must be supplemented by data regarding the condition of existing roads, by other types of traffic data, and by financial data. For example, road-use information might point to the desirability of improving primary highway conditions in the vicinity of large cities but special traffic studies would be necessary to determine whether improvement at a particular location should consist of a by-pass route to accommodate an existing high percentage of through traffic or whether it should consist of extensions to main city thoroughfares of adequate width and design to accommodate a high percentage of local traffic together with a relatively small amount of through traffic. Studied alone, however, road-use information presents an essential picture of highway operations and a background of
travel characteristics which are extremely valuable in projecting comprehensive plans for a highway system to serve the best interests of all motorists.

## SUMMARY

These preliminary analyses of road-use data indicate:

1. Use of the rural road facilities by urban motorists decreases with increase in size of the city in which they reside.
2. Motorists residing in incorporated places perform 71 percent of all travel occurring on primary highways.
3. In the case of all motorists except those resident in cities of more than 100,000 population, more than
half their annual travel occurs on primary highways.
4. Motor-vehicle use is largely comprised of short trips for passenger cars as well as for truck:
5. A large amount of rural highway travel is occasioned by the travel of city motor-vehicle owners within short distances of their residences.
6. The proportional amount of such travel by urban residents decreases with increase in the size of the cities in which the vehicle owners reside.
7. Expenditures for rural highway facilities in the vicinity of cities, especially the larger ones, will proride proportionally greater benefits for urban than for rural motor-vehicle owners.
STATUS OF FEDERAL-AID HIGHWAY PROJECTS





[^0]:    1 Based on latest available estimates.
    2 Estimate includes 20,000 miles of transcity connections which are also included with primary system mileage, because exclusively local city travel includes travel over such connections.
    ${ }^{3}$ Excludes duplication of 20,000 miles of trans-city connections.

[^1]:    ${ }^{1}$ Based on analysis of $42,407,204$ one-way trips performed by 94,167 passenger cars in these States.
    ${ }^{2}$ Less than 0.1 percent.

[^2]:    1 This is the one－way distance of all trips．I trip from Washington，D．（．，to Baltimore，ATd．，and return would be considered as 2 trips of 40 miles each．
    ${ }^{2}$ The mean shows the arithmetical average lengit of all trips．
    ${ }^{3}$ The median indicates the length of that trip below and above which equal mumbers of trips oceur．

[^3]:    (Continued on Tage 62)

[^4]:    Research na Bituminoas Paviny Mixtyres, by W. J. Ennozs, PUBLIC ROADS, vol. 7, No. 10, December 1926.

[^5]:    ${ }^{2}$ Standard Definitions of Terms Relating to Specific Gravity, A. S. T. M. Designation E12-27.

