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THE EFFECT OF USING A BLEND OF PORTLAND AND NATURAL CEMENT ON THE PHYSICAL PROPERTIES OF MORTAR AND CONCRETE¹

BY THE DIVISION OF TESTS, BUREAU OF PUBLIC ROADS

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WITHIN the last few years considerable scaling of concrete pavements has resulted from the use of chemical salts in ice treatment in the Northern States. In an effort to combat this condition the New York State highway department began about 4 years ago to experiment with a blend of portland and natural cement for concrete pavement construction.

Preliminary tests made in the laboratory of the New York Department of Public Works indicated that concretes containing blends of portland and natural cements had greater resistance to alternate freezing and thawing in a solution of calcium chloride than had concretes containing only portland cement. These preliminary laboratory experiments were followed by the construction of a number of experimental sections of actual pavements, using portland cements of various chemical compositions and two natural cements, both made in New York State and both readily available commercially.

The Bureau of Public Roads became interested in this work not only from the technical viewpoint but also because of a request from the Division of Highways of the New York Department of Public Works for permission to conduct certain of its field experiments on pavements financed either wholly or in part with Federal funds. It was felt that before approving the use of blended cements on any extended mileage of highway very definite information should be obtained regarding their effect on other essential qualities, such as strength, volume change, etc., and, furthermore, that these tests should be made in the laboratory under closely controlled conditions.

This paper reports the results of such a study as well as the results of a series of freezing and thawing tests on specimens taken from certain of the experimental pavements referred to above. The data on laboratory fabricated specimens are presented in two sections, the first dealing with tests of mortars, and the second with concretes. The results of freezing tests on concrete cores are discussed in a third section.

Tests on the mortar specimens included determinations of crushing strength, flexural strength, absorption, density, durability, and volume change. Two portland cements, differing considerably in chemical composition, and two sands of widely different mineral composition were used in this portion of the investigation. Tests on concrete specimens fabricated in the laboratory included determinations of crushing strength, flexural strength, and modulus of elasticity, using the same two portland cements with local fine and coarse aggregate. One brand of natural cement was used in all specimens fabricated in the laboratory.

The freezing tests discussed in the third part of the report were made on cores taken from four of the experimental concrete pavements. Two natural cements as well as portland cements conforming to four different classifications as to compound composition were used in these roads. Core specimens were taken from sections in which straight portland cement was used as well as from sections containing blends in the proportion of 6 parts of portland cement to 1 part of natural cement, and 5 parts of portland cement to 2 parts of natural cement, by weight. These specimens were subjected to alternate freezing and thawing in water and in a 10 percent solution of commercial calcium chloride.

REPRESENTATIVE MATERIALS USED IN LABORATORY FABRICATED SPECIMENS

The two portland cements included one brand from the Hudson River Valley region, designated as cement A, and one from the Lehigh Valley of Pennsylvania, designated as cement B. Both cements satisfactorily met the requirements for portland cement of the American Society for Testing Materials. The natural cement represented material produced in the northeastern part of the United States and is typical of the old type produced in vertical kilns. The fineness as determined by the Wagner turbidimeter was about the same for both portland cements, but the natural cement had a fineness more nearly corresponding to that of high-early-strength cement. The results of the physical and chemical tests on the three cements are shown in table 1.

Three concrete sands were used in the investigation. Sands A and C were composed essentially of quartz, and sand B consisted essentially of shale and quartz particles with appreciable amounts of limestone, slate, and sandstone. The sieve analyses and physical properties of the three sands and the coarse aggregate used in the tests are given in table 2. The coarse aggregate used in the laboratory fabricated concrete specimens was a limestone of known satisfactory quality and graded uniformly from No. 4- to 1½-inch material.

For the various tests on mortars, each portland cement and blend was used with each sand. Specimens were made using 100 percent portland cement and using two blends, one in which 14 percent and the other in which 28 percent by weight of portland cement was replaced by natural cement. These values correspond to the percentages of natural cement used in the New York experimental roads, which were of the order of 1 sack or 2 sacks in a 7-sack batch, or 14 and 28 percent, respectively.

For each of the portland cements, a sufficient quantity of water was added to the straight portland cement-sand A mixture to give a flow of approximately 90 on

¹ Paper presented at the forty-first annual meeting of the American Society for Testing Materials, Atlantic City, N. J., 1938.

TABLE 1.—Properties of cements used in laboratory fabricated specimens

PHYSICAL PROPERTIES			
Item	Cement A	Cement B	Natural cement
Apparent specific gravity.....	3.14	3.11	3.00
Specific surface, cm ² /gm.....	1,730	1,805	2,680
Percent retained on No. 200 sieve.....	7.1	9.6	10.5
Normal consistency, percent.....	23.7	24.0	26.0
Time of set, hours:			
Initial.....	3.3	2.9	1.1
Final.....	5.1	4.9	1.8
Tensile strength, lb. per sq. in.:			
7 days.....	380	370	50
28 days.....	445	445	150

CHEMICAL ANALYSES			
Oxides	Percent	Percent	Percent
Silica.....	22.41	20.80	24.91
Alumina.....	4.78	6.47	9.65
Iron.....	3.72	2.38	
Lime.....	64.55	63.22	33.93
Magnesia.....	1.15	3.45	19.92
Sulphuric anhydride.....	1.72	1.77	1.42
Loss on ignition.....	1.37	1.47	8.55

COMPUTED COMPOUND COMPOSITIONS			
Compounds			
Tricalcium silicate.....	50	47	
Dicalcium silicate.....	26	25	
Tricalcium aluminate.....	6	13	
Tetracalcium aluminoferrite.....	11	7	

TABLE 2.—Properties of aggregates
SIEVE ANALYSES OF FINE AGGREGATES

Total retained on—	Sand A	Sand B	Sand C
3/4-inch sieve.....	0	0	0
No. 4 sieve.....	2	4	7
No. 8 sieve.....	11	21	24
No. 16 sieve.....	27	45	39
No. 30 sieve.....	49	67	53
No. 50 sieve.....	84	87	86
No. 100 sieve.....	97	95	97
Fineness modulus.....	2.70	3.19	3.06

PHYSICAL PROPERTIES				
Item	Sand A	Sand B	Sand C	Coarse aggregate
Absorption, percent.....	0.3	2.2	0.9	0.32
Organic matter.....	(¹)	(¹)	(¹)	
Decantation loss, percent.....	1.4	2.5	1.5	
Compressive strength ratio:				
7 days.....	140	123	121	
28 days.....	130	124	121	
Percentage of wear, Deval.....				3.0

¹ Satisfactory.

the 10-inch flow table. The same quantity of water was then used in all other mixtures containing this cement. This resulted in slightly lower flows (drier mortars) for the mixtures containing sand B and also somewhat lower flows for the mixtures containing natural cement. All mixtures, however, were plastic and workable, insofar as could be determined when fabricating the test specimens. The consistencies of the various mixes, as determined by the use of the 10-inch flow table, are shown in table 3.

An outline of the various tests made on the mortar specimens follows.

1. Determinations of crushing and flexural strengths of 2-inch cubes and 2- by 3- by 18-inch beams, using a mix of 1:2 by weight. Specimens were cured continuously in water at 70° F., and broken at ages of 7,

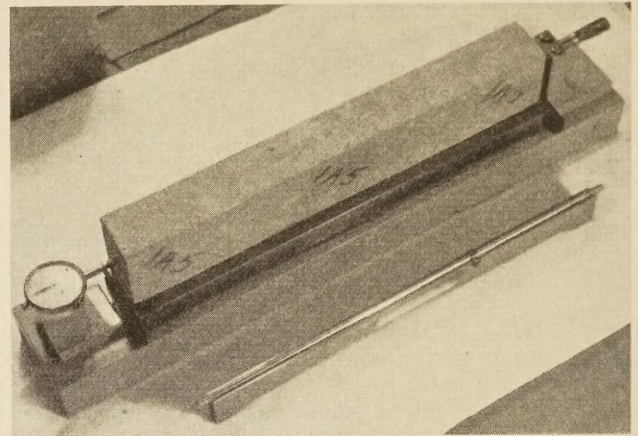


FIGURE 1.—HORIZONTAL COMPARATOR USED IN MEASURING LENGTH CHANGE OF MORTAR BEAMS.

TABLE 3.—Consistencies of mortars as determined by the 10-inch flow table

Percentage of portland cement replaced by natural cement	MIXTURES CONTAINING SAND A			
	Cement A		Cement B	
	Beams	Cubes	Beams	Cubes
0.....	91	93	93	93
14.....	87	90	90	89
28.....	83	83	88	85

Percentage of portland cement replaced by natural cement	MIXTURES CONTAINING SAND B			
	Cement A		Cement B	
	Beams	Cubes	Beams	Cubes
0.....	83	83	85	85
14.....	79	76	82	80
28.....	74	70	79	78

28, 90, 180, and 360 days. Duplicate sets of specimens were cured in air (with 7 days initial water cure) and broken at 28, 90, 180, and 360 days. Beam specimens were broken as cantilevers, using portions of the same beam for tests at various ages.

2. Determinations of tensile strength of briquets containing standard Ottawa sand in a 1:3 mix by weight. Specimens were broken at 7 and 28 days.

3. Tests for volume change on 2- by 3- by 18-inch beams, using a 1:2 mix by weight. One set of specimens was stored in air for 120 days, the other continuously in water at 70° F. for 180 days. As a supplementary portion of this phase of the work, after 180 days of water storage the beams were frozen and thawed for 6 cycles, dried at 120° F. for 4 days, and resaturated in water at 70° F. for 3 days, after which the freezing and thawing cycle was again repeated. In this test measurements of length were made at the conclusion of each 3-day resaturation period. The linear comparator used in all measurements of volume change is shown in figure 1.

4. Miscellaneous tests included absorption and bulk specific gravity tests on 2- by 4-inch cylinders of 1:2 mortar, sodium sulphate soundness tests on 1- by 1- by 6-inch bars, using a mix of 1:5 by weight of Ottawa sand, and autoclave tests on 1- by 1- by 10-inch neat cement bars.

USE OF BLENDED CEMENTS DECREASED STRENGTH OF MORTARS

The results obtained in the strength tests of mortars are given in tables 4 to 7, inclusive, and are shown graphically in figures 2 to 5 inclusive. In the figures, the

values have been plotted so as to show on the left the average results for the two sands with each cement and on the right the average results for the two cements with each sand.

Table 4 and figure 2 give the results of crushing tests on specimens continuously water cured. When the results are averaged as in the figure it will be seen that, for each combination of materials, the straight portland cement mixes developed the greatest strengths

TABLE 4.—Crushing strengths of 2-inch mortar cubes, mix 1:2 by weight, water cured

Percentage of portland cement replaced by natural cement	7 days		28 days		90 days		180 days		360 days	
	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²
0.....	6,180	100	8,570	100	9,625	100	9,910	100	9,990	100
14.....	5,805	94	7,360	86	8,580	89	9,005	91	9,320	93
28.....	4,620	75	6,105	71	7,255	75	7,595	77	8,290	83

CEMENT A—SAND B										
Percentage of portland cement replaced by natural cement	7 days		28 days		90 days		180 days		360 days	
	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²
0.....	5,000	100	6,975	100	8,280	100	9,125	100	9,645	100
14.....	5,010	100	6,555	94	7,740	94	8,270	91	9,245	96
28.....	4,430	89	5,845	84	7,190	87	7,880	86	8,660	90

CEMENT B—SAND A										
Percentage of portland cement replaced by natural cement	7 days		28 days		90 days		180 days		360 days	
	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²
0.....	6,400	100	8,375	100	9,825	100	9,665	100	9,355	100
14.....	5,090	80	6,710	80	8,165	83	8,695	90	8,620	92
28.....	4,130	65	5,420	65	6,920	70	7,175	74	7,490	80

CEMENT B—SAND B										
Percentage of portland cement replaced by natural cement	7 days		28 days		90 days		180 days		360 days	
	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²
0.....	5,015	100	6,815	100	7,840	100	8,620	100	8,980	100
14.....	4,745	95	6,140	90	7,345	94	7,725	90	8,590	96
28.....	3,930	78	5,215	77	6,645	85	7,120	83	7,735	86

¹ Each value is the average of 15 tests.
² Figures indicate percentage of the strength of 100 percent portland cement mixes.

TABLE 5.—Crushing strengths of 2-inch mortar cubes, mix 1:2 by weight, air cured after 7-day initial water cure

Percentage of portland cement replaced by natural cement	28 days		90 days		180 days		360 days	
	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²
0.....	8,825	100	8,870	100	8,715	100	9,085	100
14.....	8,155	92	8,550	96	8,535	98	8,865	98
28.....	6,895	78	7,335	83	7,540	87	7,870	87

CEMENT A—SAND B								
Percentage of portland cement replaced by natural cement	28 days		90 days		180 days		360 days	
	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²
0.....	7,205	100	7,555	100	7,870	100	7,105	100
14.....	7,170	100	7,470	99	7,780	99	6,885	97
28.....	6,540	91	7,100	94	7,210	92	6,295	89

CEMENT B—SAND A								
Percentage of portland cement replaced by natural cement	28 days		90 days		180 days		360 days	
	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²
0.....	8,755	100	8,975	100	9,040	100	8,765	100
14.....	7,530	86	7,980	89	8,255	91	8,175	93
28.....	6,290	72	6,705	75	7,065	78	6,920	79

CEMENT B—SAND B								
Percentage of portland cement replaced by natural cement	28 days		90 days		180 days		360 days	
	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²
0.....	7,040	100	7,425	100	7,670	100	7,195	100
14.....	6,700	95	6,895	93	7,465	97	6,525	91
28.....	5,495	78	5,505	74	5,825	76	6,040	84

¹ Each value is the average of 15 tests.
² Figures indicate percentage of the strength of 100 percent portland cement mixes.

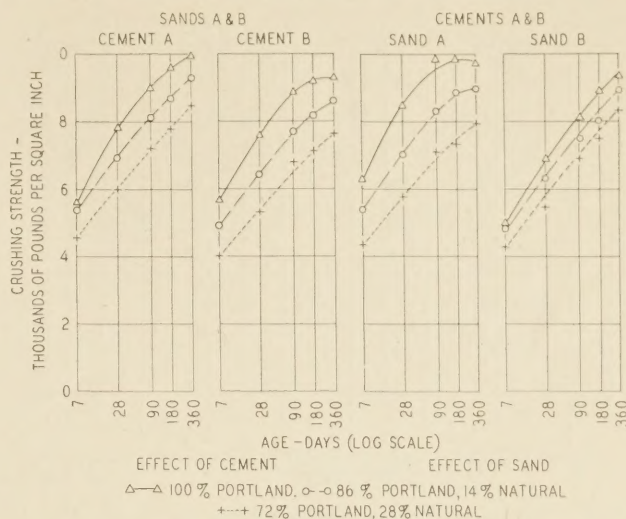


FIGURE 2.—EFFECT OF BLENDING PORTLAND AND NATURAL CEMENT ON THE CRUSHING STRENGTH OF 2-INCH MORTAR CUBES CURED IN WATER.

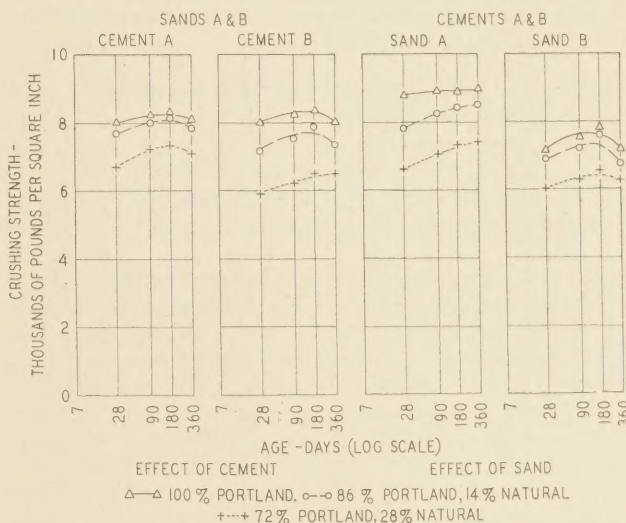


FIGURE 3.—EFFECT OF BLENDING PORTLAND AND NATURAL CEMENT ON THE CRUSHING STRENGTH OF 2-INCH MORTAR CUBES CURED IN AIR.

at all ages. With the exception of the 180- and 360-day values for the straight portland cement mortar in which sand A was used with cements A and B, all mortars exhibited gain in strength from the 7- up to the 360-day periods.

In the left hand portion of figure 2 it may be seen that mortars containing cement A had slightly greater strengths than those containing cement B. The right hand portion of this figure shows the effect of using different sands. In the unblended mix, as well as in the mortar containing 14 percent of natural cement, specimens containing sand A showed greater strengths at all ages. However, in the mixes containing 28 percent of natural cement, the strengths were about the same. In other words, replacing portland cement with natural cement seemed to affect the strength of mortars made with sand A much more than those made with sand B.

The values obtained on air-cured, 2-inch cubes (after 7 days of initial water storage) are given in table 5 and in figure 3. The average values given in the figures

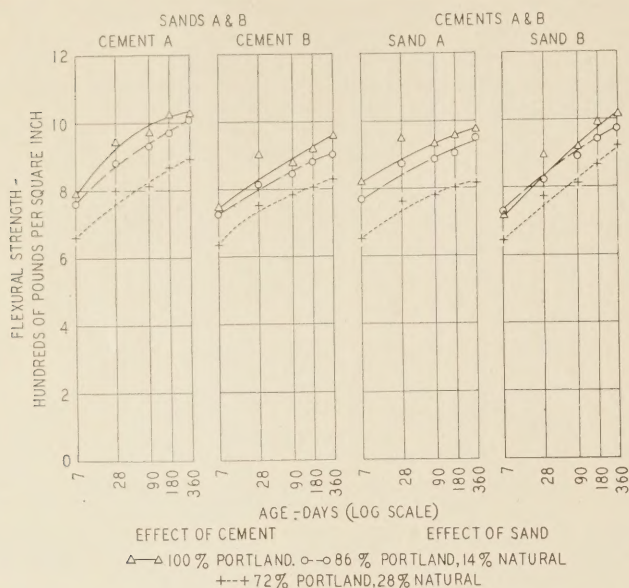


FIGURE 4.—EFFECT OF BLENDING PORTLAND AND NATURAL CEMENT ON FLEXURAL STRENGTH OF 18-INCH MORTAR BEAMS CURED IN WATER.

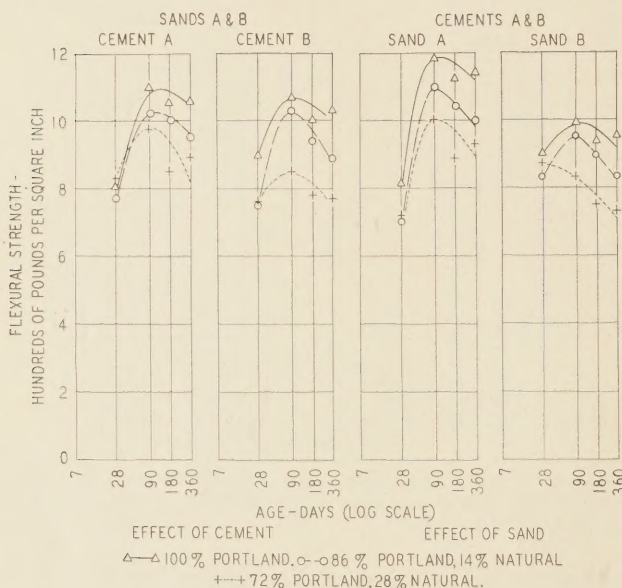


FIGURE 5.—EFFECT OF BLENDING PORTLAND AND NATURAL CEMENT ON FLEXURAL STRENGTH OF 18-INCH MORTAR BEAMS CURED IN AIR.

show, in all cases, higher strengths for the mixes containing straight portland cement. Comparing figures 2 and 3 it will be noted that the specimens moist-cured 7 days and air-dried for 21 days were in general higher in strength than those cured continuously in water for 28 days. Further air storage, however, resulted in only slight gain in strength at 90 days, with little or no gain thereafter.

Figure 3 shows the effect of the cement and sand upon the crushing strength of the air-dried specimens. Specimens containing cement A exhibited slightly greater strengths in the blends, with the unblended mortars having about the same strength at all ages. Specimens containing sand A showed definitely higher strengths throughout. The most interesting point in connection with this figure is the decrease in strength

TABLE 6.—Flexural strengths of 18-inch mortar beams, mix 1:2 by weight, water cured

CEMENT A—SAND A										
Percentage of portland cement replaced by natural cement	7 days		28 days		90 days		180 days		360 days	
	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²
0.....	850	100	995	100	970	100	1,010	100	1,030	100
14.....	770	91	905	91	940	97	945	94	995	97
28.....	660	78	780	78	795	82	825	82	880	85

CEMENT A—SAND B										
Percentage of portland cement replaced by natural cement	7 days		28 days		90 days		180 days		360 days	
	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²
0.....	735	100	900	100	965	100	1,045	100	1,025	100
14.....	755	103	860	96	920	95	985	94	1,055	103
28.....	660	90	820	91	835	87	915	88	915	89

CEMENT B—SAND A										
Percentage of portland cement replaced by natural cement	7 days		28 days		90 days		180 days		360 days	
	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²
0.....	790	100	905	100	885	100	900	100	915	100
14.....	760	96	845	93	835	94	865	96	915	100
28.....	650	82	745	82	765	86	790	88	760	83

CEMENT B—SAND B										
Percentage of portland cement replaced by natural cement	7 days		28 days		90 days		180 days		360 days	
	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²
0.....	700	100	890	100	870	100	930	100	1,000	100
14.....	700	100	795	89	865	99	895	96	885	89
28.....	625	89	745	84	800	92	820	88	910	91

¹ Each value is the average of 5 tests.
² Figures indicate percentage of the strength of 100 percent portland cement mixes.

TABLE 7.—Flexural strengths of 18-inch mortar beams, mix 1:2 by weight, air cured

CEMENT A—SAND A										
Percentage of portland cement replaced by natural cement	28 days		90 days		180 days		360 days			
	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²
0.....	720	100	1,195	100	1,140	100	1,180	100	1,180	100
14.....	680	94	1,110	93	1,070	94	1,035	94	1,035	88
28.....	725	100	1,085	91	915	80	970	80	970	82

CEMENT A—SAND B										
Percentage of portland cement replaced by natural cement	28 days		90 days		180 days		360 days			
	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²
0.....	885	100	1,005	100	950	100	935	100	935	100
14.....	865	98	930	93	920	97	875	97	875	94
28.....	930	105	850	85	795	84	805	84	805	86

CEMENT B—SAND A										
Percentage of portland cement replaced by natural cement	28 days		90 days		180 days		360 days			
	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²
0.....	905	100	1,165	100	1,095	100	1,100	100	1,100	100
14.....	700	77	1,080	98	1,015	93	970	93	970	88
28.....	710	79	905	78	855	78	880	78	880	80

CEMENT B—SAND B										
Percentage of portland cement replaced by natural cement	28 days		90 days		180 days		360 days			
	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²
0.....	900	100	970	100	915	100	975	100	975	100
14.....	800	89	970	100	860	94	800	94	800	82
28.....	810	90	810	83	705	77	655	77	655	67

¹ Each value is the average of 5 tests.
² Figures indicate percentage of the strength of the 100 percent portland cement mixes.

caused by the replacement of 28 percent of the portland cement with natural cement. The strengths obtained with this combination ran about 1,000 to 2,000 pounds per square inch lower than those of the straight portland cement mixes for the different cements and sands.

The effect of blending portland and natural cement upon the flexural strength of water-cured mortar beams is shown in table 6 and in figure 4. It will be noted that when the results are averaged as in figure 4, for

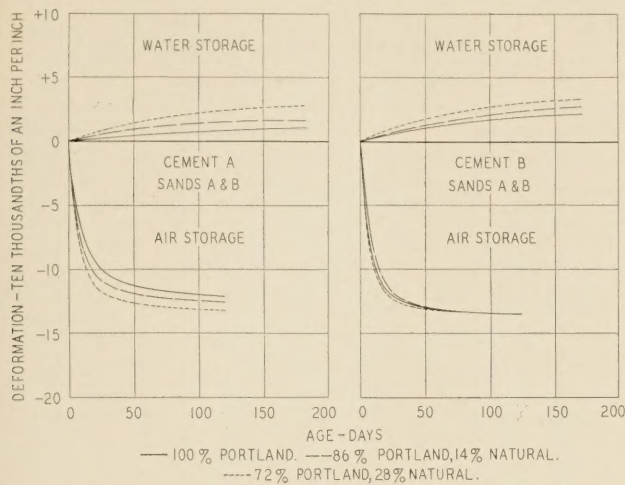


FIGURE 6.—EFFECT OF BLENDING PORTLAND AND NATURAL CEMENT ON VOLUME CHANGE OF 18-INCH MORTAR BEAMS.

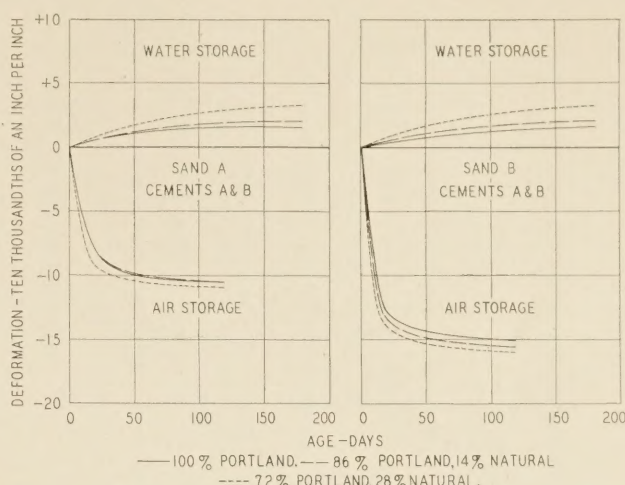


FIGURE 7.—EFFECT OF BLENDING PORTLAND AND NATURAL CEMENT ON VOLUME CHANGE OF 18-INCH MORTAR BEAMS.

each combination of materials, the straight portland cement mixes, with a single exception, produced the greatest strengths at all ages. As in the water-cured compression specimens, the gain in strength with age was fairly uniform. However, the reduction in strength caused by blending was not proportional to the amount of natural cement used, the 28-percent replacement causing a proportionately greater reduction than the 14-percent replacement. Cement A in general produced greater strengths for all combinations of materials than did cement B.

The results of flexure tests on air-cured beams are given in table 7 and in figure 5. It will be observed that here also for each combination of materials, with a single exception, the straight portland cement mixes produced the greatest strengths at all ages. With few exceptions the highest strengths were obtained at 90 days, all mixes showing retrogression after that period. In every case the strengths at 90 days were higher than those of the corresponding water-cured specimens.

The effect of blending portland and natural cement upon volume change of 18-inch mortar beams in continuous water storage is shown graphically in the upper portion of figures 6 and 7. The changes are shown in 0.0001 inch per inch at the ages indicated on the abscissas. It will be noted from the figures that blends containing 28 percent of natural cement had the greatest expansion at 180 days, and that the straight portland cement mixes for all combinations exhibited the least expansion. It is of interest to note (fig. 6) that specimens containing cement B had slightly greater expansions for both blended and unblended mixtures than those containing cement A. However, as will be seen from figure 7, changing the sand had very little effect upon volume change.

MORTAR BEAMS, SUBJECTED TO ACCELERATED WEATHERING TESTS

Volume changes resulting from continuous air storage are shown in the lower portions of figures 6 and 7. The treatment of these beams consisted of storage for 24 hours in the moist closet, at the end of which time an initial reading was taken, followed by continued storage in laboratory air. During the period from 90 to 120 days the beams were placed in a cabinet in which the air was dried with calcium chloride to decrease further the amount of moisture in the specimens. As expected, all

beams showed slightly increased shrinkage during this period.

Beams containing sands A and B and cement A had approximately the same amount of shrinkage as those containing cement B (fig. 6). However, specimens containing cements A and B and sand B had shrinkages at 120 days about 50 percent greater than those containing sand A (fig. 7). The figures show that although the initial shrinkage was somewhat greater in specimens containing the blended cements, the total shrinkage at the end of 120 days of continuous air storage was about the same.

The mortar beams from the continuous water storage series were measured for length at the end of 180 days, and then subjected to 15 rounds of the following accelerated weathering treatment. Each round required 13 days for completion and consisted of 6 cycles of freezing and thawing, each 24 hours in duration, followed by drying for 4 days in an electric oven at 120° F., and resaturation for 3 days in water at 70° F. Readings were taken on each beam at the end of the resaturation period and were discontinued at the end of the fifteenth round, at which time surface disintegration of the specimens had progressed to a point which affected the accuracy of the readings because of loosening of the gage points. Unit length changes at the end of the first, third, sixth, ninth, twelfth, and fifteenth rounds are shown in figures 8 and 9. Here the data have been averaged in the same manner as in the previous figures so as to bring out the effect of variations in the cement and sand.

Several points in connection with these figures are of interest. In the first place, all specimens showed an initial contraction followed by expansion up to the fifteenth round. The amount of initial contraction seemed to have been affected greatly by the combination of materials used, being greatest for the combinations involving cement A with the two sands (fig. 8) and sand B with the two cements (fig. 9). The combinations involving sand A with cements A and B showed very little initial contraction. This effect may have resulted from the drying cycle which was introduced. Drying at 120° F. for 4 days undoubtedly induced high shrinkage in the cement paste, subjecting the particles of aggregate to high compressive stress. It is possible that the difference in the ability of the sands to resist this stress without permanent deforma-

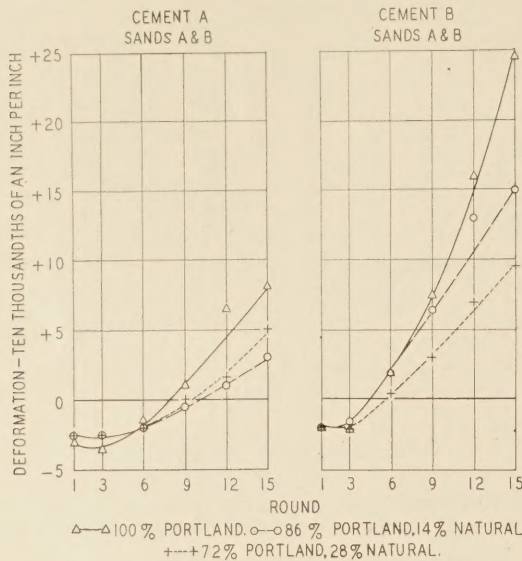


FIGURE 8.—EFFECT OF FREEZING AND THAWING, FOLLOWED BY DRYING, COOLING, AND RESATURATION ON VOLUME CHANGE OF MORTAR BEAMS.

tion may have accounted for the residual shrinkage observed in the specimens containing sand B, which was composed of comparatively low-strength material.

The quartz particles composing sand A may have possessed sufficient strength to resist the shrinkage stresses without permanent deformation, so that the specimens when resaturated returned practically to their original length. The accumulated effect of this action may account also for the relatively small expansions which were observed for specimens containing sand B as compared to the expansions shown by specimens containing sand A.

It will be observed also that, without exception, specimens containing the blended cement showed lower expansions than the corresponding specimens containing only portland cement. It is usually assumed, insofar as expansion is concerned, that repeated freezing and thawing produces a gradually expanding paste, resulting in the eventual disintegration of the specimen. On this basis, the use of the blended cements may be considered beneficial. However, visual examination of the specimens as the test proceeded failed to reveal any consistent difference in appearance at any stage between the specimens containing the blended cements and those containing the straight portland cement. So far as could be determined by visual inspection of the surface, all of the specimens failed at about the same rate. All were badly exfoliated or surface pitted by the end of the fifteenth round and apparently possessed little strength.

BLENDED INCREASED RESISTANCE OF MORTARS TO SULPHATE ACTION

The effect of replacements with natural cement on the tensile strength of 1:3 Ottawa sand briquets is shown in table 8. It is of interest to note that, for both cements at 28 days, the specimens containing 14 percent of natural cement had very nearly the same strengths as the straight portland cement mortars. The strengths at 28 days of specimens containing 28 percent of natural cement were approximately 90 percent of the strengths of the straight portland cement mortars.

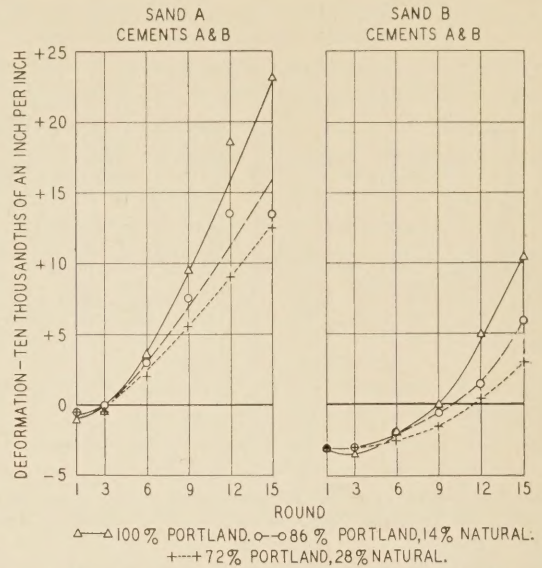


FIGURE 9.—EFFECT OF FREEZING AND THAWING, FOLLOWED BY DRYING, COOLING, AND RESATURATION ON VOLUME CHANGE OF MORTAR BEAMS.

TABLE 8.—Tensile strengths of briquets, mix 1:3, standard Ottawa sand

Percentage of portland cement replaced by natural cement	Cement A				Cement B			
	7 days		28 days		7 days		28 days	
	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²	Lb. per sq. in. ¹	Per-cent ²
0	390	100	455	100	390	100	450	100
14	370	95	450	99	360	92	440	98
28	345	88	405	89	330	85	405	90

¹ Each value is the average of 15 tests.

² Figures indicate percentage of the strength of 100 percent portland cement mixes.

The effect of blending the cements upon the bulk specific gravity and absorption of mortar specimens is shown in table 9. Cylinders 2 inches by 4 inches in size of 1:2 mortar, after 24 hours in moist air, were stored in water at 70° F. and the bulk specific gravity and absorption determined after 7 and 28 days by first determining the saturated-surface dry weights of the specimens in air and in water and then drying to constant weight. It will be observed that blending with natural cement lowered the bulk specific gravity and increased the absorption in all cases. The decrease in density which is indicated by these tests may have been the result of a slight increase in the air voids brought about by the use of natural cement. Other tests have indicated that air voids in concrete are increased somewhat by using blends.

Bars 1 by 1 by 6 inches long, using a 1:5 standard Ottawa sand mortar, and with 0, 14, and 28 percent of the portland cement replaced with natural cement, were fabricated and stored for 6 days in water at 70° F. At the end of this period the bars were measured for length and then immersed in a 10-percent solution of sodium sulphate. After 21 days of continuous storage in this solution at 70° F., they were again measured for length. The amount of expansion was read to 0.001 inch. The results are shown in figure 10 (left panel). Each value is the average of three tests. It will be noted that specimens containing cement B showed greater expansion in all cases than those con-

TABLE 9.—Bulk specific gravity and absorption of 2 by 4-inch mortar cylinders, mix 1:2 by weight, water cured

CEMENT A—SAND A								
Percentage of portland cement replaced by natural cement	Bulk specific gravity ¹				Percentage absorption ¹			
	7 days		28 days		7 days		28 days	
		Per-cent ²		Per-cent ²		Per-cent ²		Per-cent ²
0.....	2.10	100	2.12	100	9.52	100	9.23	100
14.....	2.07	99	2.09	99	9.89	104	10.01	108
28.....	2.03	97	2.04	96	10.32	108	10.59	115

CEMENT A—SAND B								
Percentage of portland cement replaced by natural cement	Bulk specific gravity ¹				Percentage absorption ¹			
	7 days		28 days		7 days		28 days	
		Per-cent ²		Per-cent ²		Per-cent ²		Per-cent ²
0.....	2.09	100	2.10	100	10.49	100	10.32	100
14.....	2.07	99	2.08	99	10.84	103	10.73	104
28.....	2.04	98	2.05	98	11.34	108	11.19	108

CEMENT B—SAND A								
Percentage of portland cement replaced by natural cement	Bulk specific gravity ¹				Percentage absorption ¹			
	7 days		28 days		7 days		28 days	
		Per-cent ²		Per-cent ²		Per-cent ²		Per-cent ²
0.....	2.07	100	2.11	100	10.03	100	9.98	100
14.....	2.05	99	2.06	98	10.66	106	10.62	106
28.....	2.02	98	2.02	96	11.27	112	11.40	114

CEMENT B—SAND B								
Percentage of portland cement replaced by natural cement	Bulk specific gravity ¹				Percentage absorption ¹			
	7 days		28 days		7 days		28 days	
		Per-cent ²		Per-cent ²		Per-cent ²		Per-cent ²
0.....	2.09	100	2.07	100	10.95	100	10.76	100
14.....	2.04	98	2.06	100	11.34	104	11.11	103
28.....	2.01	96	2.04	99	11.73	107	11.57	108

¹ Each value is the average of 3 tests.
² Figures indicate percentage of the corresponding values for 100 percent portland cement mixes.

taining cement A, and also that blending with natural cement reduced the amount of expansion in both cases. These tests seem to indicate that blending with natural cement increases resistance to the action of sodium sulphate solutions.

The expansions of neat cement bars exposed for 3 hours at 420° F. in the autoclave are shown in the right panel of figure 10. In both cases, the replacement of 14 and 28 percent of portland cement by natural cement resulted in increased expansion. It will be noted, however, that cement B, unblended, showed considerably greater expansion than cement A with a 28 percent replacement of natural cement. The relatively high expansions shown by cement B may possibly be associated with the higher magnesia and greater C₃A content of this cement. (See table 1.)

The same two portland cements and the same natural cement that were used in the mortar tests were combined with a local fine and coarse aggregate in fabricating the concrete specimens. The characteristics of the aggregates are shown in table 2. Sand C was used in the concrete tests. The coarse aggregate was uniformly graded from No. 4 to 1½-inch. Two blends were used, one with 14 percent and the other with 28 percent of natural cement as in the mortar tests.

A typical concrete paving mix containing 6 sacks of cement per cubic yard with sufficient water for a 2½-inch slump was used. This resulted in a net water-cement ratio by volume of approximately 0.75.

Although the tests made on the mortars showed that the consistency of the mix was affected somewhat by the substitution of natural cement for portland cement, the slump of the concrete containing the blends was the same as that obtained with the straight portland cement. However, in manipulating the concrete it was found that the blended cement mixtures had a stickiness which was not apparent in mixtures containing the straight portland cement.

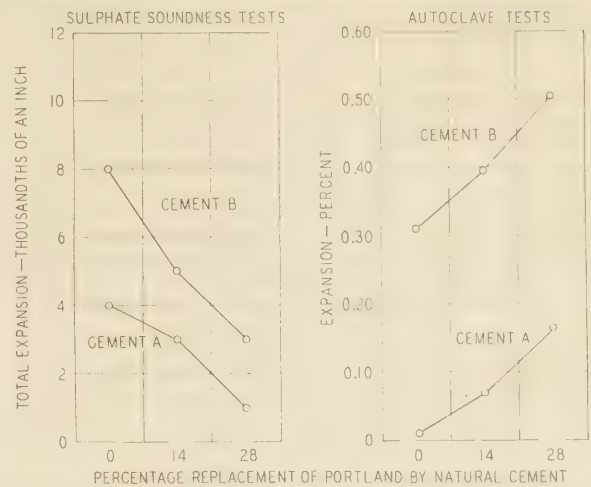


FIGURE 10.—EFFECT OF BLENDING PORTLAND AND NATURAL CEMENT UPON VOLUME CHANGE IN SULPHATE SOLUTION AND IN THE AUTOCLAVE.

USE OF BLENDED CEMENTS SLIGHTLY DECREASED STRENGTH OF CONCRETE

Flexure tests were made on beams 6 by 6 by 21 inches in size, loaded at the third points of an 18-inch span. Compression tests as well as measurements for modulus of elasticity, using a Martens extensometer, were made on 6-inch by 12-inch cylinders. Five specimens were made for each test condition and tests were made at 7, 28, 90, 180, and 360 days. All specimens were stored continuously in moist air at 70° F. until tested.

The results of the tests for strength and modulus of elasticity on laboratory fabricated concrete specimens are shown in table 10 and are shown graphically in figure 11.

Referring to the data for cement A (table 10 and left panel of fig. 11), it will be observed that there was a gradual increase in crushing strength with age for specimens containing the unblended as well as the blended cement. Blending with natural cement decreased the average strength at all ages about 1 percent in the specimens containing 14 percent of natural cement and about 8 percent in the specimens containing 28 percent of natural cement. Similar data for cement B are shown in the next panel to the right. In the specimens containing straight portland cement it will be observed that the same order of strengths was obtained as with cement A. However, in specimens with the blended cements as compared to the specimens with straight portland cement, the reduction in strength was more marked, the corresponding figures being 7 percent for the 14 percent blend and 17 percent for the 28 percent blend.

It is interesting to observe that specimens containing the 14 percent blend for both cements A and B exhibited higher strengths at 360 days than the corresponding straight portland cement specimens. This was due to the fact that the straight portland cement concretes showed no increase in strength beyond the 180-day period for cement A, while for cement B no increase was observed after the 90-day period. On the other hand, the concrete containing the blended cements showed a fairly uniform increase in strength up to 360 days.

Flexural strengths of the concrete specimens are shown also in table 10 and are plotted in the two center panels of figure 11. For each cement the specimens containing the 14 percent blend had strengths com-

TABLE 10.—Effect of blending portland and natural cements on crushing and flexural strengths and modulus of elasticity

CRUSHING STRENGTH—CEMENT A										
Percentage of portland cement replaced by natural cement	Test results at age indicated ¹									
	7 days		28 days		90 days		180 days		360 days	
	Lb. per sq. in.	Per-cent ²	Lb. per sq. in.	Per-cent ²	Lb. per sq. in.	Per-cent ²	Lb. per sq. in.	Per-cent ²	Lb. per sq. in.	Per-cent ²
0.....	3,730	100	5,510	100	6,190	100	6,650	100	6,660	100
14.....	3,860	103	5,270	96	6,010	97	6,470	97	6,830	103
28.....	3,430	92	4,750	86	5,870	95	6,080	91	6,480	97

MODULUS OF RUPTURE—CEMENT A										
0.....	600	100	683	100	777	100	858	100	831	100
14.....	612	102	697	102	777	100	845	98	857	103
28.....	542	90	631	92	742	95	801	93	853	103

MODULUS OF ELASTICITY—CEMENT A										
0.....	1,000 lb. per sq. in.	100	1,000 lb. per sq. in.	100	1,000 lb. per sq. in.	100	1,000 lb. per sq. in.	100	1,000 lb. per sq. in.	100
14.....	5,272	98	5,947	98	6,490	103	6,785	99	7,102	105
28.....	5,170	98	5,537	92	6,268	97	6,735	93	7,432	105

CRUSHING STRENGTH—CEMENT B										
0.....	3,990	100	5,640	100	6,450	100	6,310	100	6,480	100
14.....	3,700	93	5,000	89	5,760	89	5,960	94	6,530	101
28.....	3,190	80	4,330	77	5,250	81	5,670	90	5,730	88

MODULUS OF RUPTURE—CEMENT B										
0.....	620	100	677	100	781	100	783	100	821	100
14.....	577	93	698	103	753	96	820	105	859	105
28.....	542	87	580	86	716	92	801	102	788	96

MODULUS OF ELASTICITY—CEMENT B										
0.....	1,000 lb. per sq. in.	100	1,000 lb. per sq. in.	100	1,000 lb. per sq. in.	100	1,000 lb. per sq. in.	100	1,000 lb. per sq. in.	100
14.....	5,225	97	5,810	96	6,178	96	6,465	97	7,070	101
28.....	5,030	96	5,623	92	6,018	94	6,330	95	6,408	92

¹ Each value for crushing strength and modulus of rupture is the average of 5 tests. Each value for modulus of elasticity is the average of 4 tests.
² Figures indicate percentage of the strength of 100 percent portland cement mixes.

parable to those obtained with the straight portland cement specimens. Specimens with the 28 percent blend had strengths that averaged about 95 percent of those of the straight portland cement specimens. It is to be observed also that for both portland cements the blended cement concrete showed higher strength ratios in flexure than in compression.

Results of tests for modulus of elasticity are shown in the two right panels of figure 11. These results are also included in table 10. Here again the addition of the natural cement is reflected in the somewhat lower values obtained with specimens containing the blended cements.

In general, it may be said that the values obtained in flexure and modulus of elasticity for the specimens containing the 28 percent blend were about 95 percent of those obtained for the specimens containing the straight portland cements. The strength ratios of specimens in compression were not the same for both cements, specimens containing cement B giving an average ratio of 83 percent, which was about 10 percent lower than that obtained for specimens containing cement A. Comparing the results of tests on concrete specimens with the corresponding results of tests on mortar specimens,

it will be observed that replacement with the natural cement had a much greater effect on mortar strength than on concrete strength. This would, of course, be expected because of the higher cement content in the mortar specimens.

FREEZING AND THAWING TESTS MADE ON PAVEMENT CORES

The results of freezing and thawing tests on cores taken from four experimental projects in New York State are given in this portion of the report. On each of these projects specimens of straight portland cement concrete and blended cement concrete were taken from sections containing cement conforming to each of the four classifications as to compound composition given in table 11.

Natural cement of the brand used in the laboratory tests already described was used on two of the projects. The other two contained natural cement from another mill in New York State. A 6:1 blend was used on two of the projects, a 5:2 blend on the other two.

TABLE 11.—Classification of four cements as to compound composition

Specification designation	Tricalcium aluminate C ₃ A (percent)	Tricalcium silicate C ₃ S (percent)
935.....	Not over 9.....	Not over 35.
950.....	Not over 9.....	Between 48 and 52.
1439.....	Not less than 14.....	Not over 39.
1450.....	Not less than 14.....	Between 48 and 52.

This portion of the work was done for the purpose of testing specimens of concrete which had been subjected to the usual field manipulation during placing in order to study the effects of the "bleeding" or "water-gain" which frequently occurs during the finishing operation. This action results in the accumulation of an excess amount of water in the top surface of the pavement, leaving a weak, porous layer which is lacking in resistance to the disintegrating effects of ice and chemical salts.

In a preliminary series of freezing and thawing tests made on full depth cores it was found that disintegration was confined almost entirely to the top 2 inches of the specimen. Because of this, and also to facilitate performance of the test, it was decided to study the effect of the blend by testing the upper portion of the concrete only. Disks 2 inches thick were sawed from the tops of the cores. One set of these disks was alternately frozen and thawed in a 10 percent solution of calcium chloride, the other set in plain water. In addition, a few tests were made to determine the resistance of similar disks sawed from the lower surface of the cores.

It was found desirable in this work to use a minimum amount of the chloride solution and to freeze rapidly in order to keep the solution from separating into layers of different density. This was accomplished by using close-fitting containers and immersing these containers in the brine solution of the refrigerator. The minimum temperature obtained was approximately -10° F., one complete cycle of freezing and thawing being performed each day. The time required to lower the temperature of the specimen to this minimum was approximately 10 hours. The calcium chloride solution was changed after 10 cycles of freezing and thawing. These time-temperature relations applied to freezing both in chloride and in water.

Periodic examinations were made of the specimens, this examination consisting of weight loss determina-

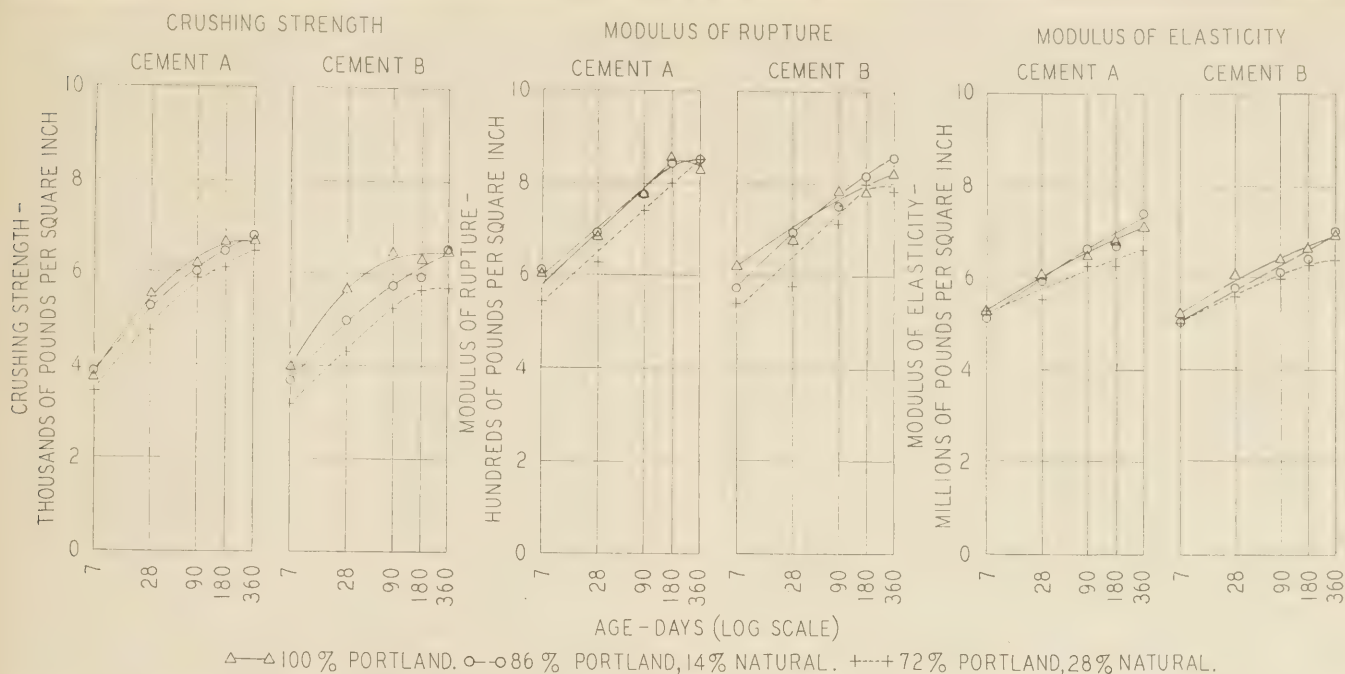


FIGURE 11.—EFFECT OF BLENDING PORTLAND AND NATURAL CEMENT ON STRENGTH AND MODULUS OF ELASTICITY OF CONCRETE.

tions and visual inspections. It was found that the weight losses of the 2-inch disks were meaningless. For instance, a specimen which had not lost more than 2 percent in weight would become unsound and disintegrate. While some scaling of the surface developed, unsoundness in general was caused by failure of the bond between the cement and aggregate, resulting in the eventual disintegration of the entire specimen.

For each project and for each type of freezing there were two groups of 8 specimens each. In one group there were 4 pairs of specimens containing straight portland cement and 4 pairs containing the same portland cements blended with natural cement in the proportion of either 6:1 or 5:2. The 4 pairs of specimens in each group represented concrete containing the 4 classes of portland cement mentioned above.

In order to evaluate the resistance to freezing and thawing, the individual specimens were examined at various intervals and rated in accordance with their appearance and ring when struck with a hammer. A specimen which was entirely sound was given a rating of 12½. An unsound specimen was rated at 2½, while a specimen which appeared to be between these two conditions was considered as questionable and was given a rating of 7½. Any specimen which had disintegrated was given a rating of zero. If all of the specimens of a particular group were sound the rating would be 100. On the other hand, if all of the specimens were disintegrated, the rating would be zero. By this method of rating, the results obtained with the four cements used on each project were averaged for comparison with the results obtained with the same portland cements blended with natural cement. This procedure was decided upon after a study of the individual results had failed to reveal any consistent relation between the composition of the portland cement and its resistance to freezing and thawing either when used straight or when blended. With a few exceptions, concrete made from portland cements conforming to the various composition classifications, when used on a

given job, behaved about alike insofar as resistance to freezing and thawing was concerned.

BLENDED SPECIMENS MOST RESISTANT TO FREEZING IN CALCIUM CHLORIDE SOLUTION AND WATER

The ratings of the several groups of specimens after 35 alternations in a 10-percent solution of calcium chloride as well as the corresponding ratings when frozen in water for various periods up to 150 alternations, are given in table 12. The appearance of each individual specimen at the end of 35 alternations in calcium

TABLE 12.—Resistance of core specimens to freezing and thawing

Blend ¹	Rating ² at number of freezing and thawing cycles indicated					
	35 cycles in calcium chloride	35 cycles in water	50 cycles in water	75 cycles in water	100 cycles in water	150 cycles in water
PROJECT A						
7-0.....	0	15	10	0	0	0
6-1G.....	90	95	90	90	90	90
PROJECT B						
7-0.....	25	50	30	25	25	25
5-2G.....	100	100	100	100	100	100
PROJECT C						
7-0.....	40	30	5	³ 0	0	0
6-1H.....	15	25	10	⁴ 0	0	0
PROJECT D						
7-0.....	0	10	5	0	0	0
5-2H.....	55	65	55	30	30	25

¹ Figures indicate the number of sacks of portland and natural cement in a 7-sack batch; letters refer to the brand of natural cement.
² Rating of 100 indicates all 8 specimens sound; rating of 0 indicates all 8 specimens disintegrated.
³ At 65 alternations.
⁴ At 60 alternations.

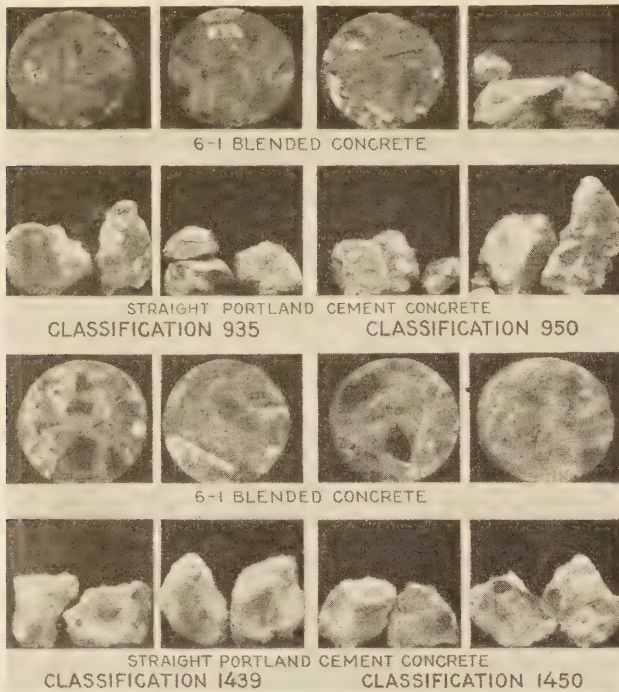


FIGURE 12.—PROJECT A. APPEARANCE OF BLENDED AND STRAIGHT PORTLAND CEMENT CONCRETE SPECIMENS AFTER 35 ALTERNATIONS OF FREEZING AND THAWING IN A 10 PERCENT SOLUTION OF CALCIUM CHLORIDE.

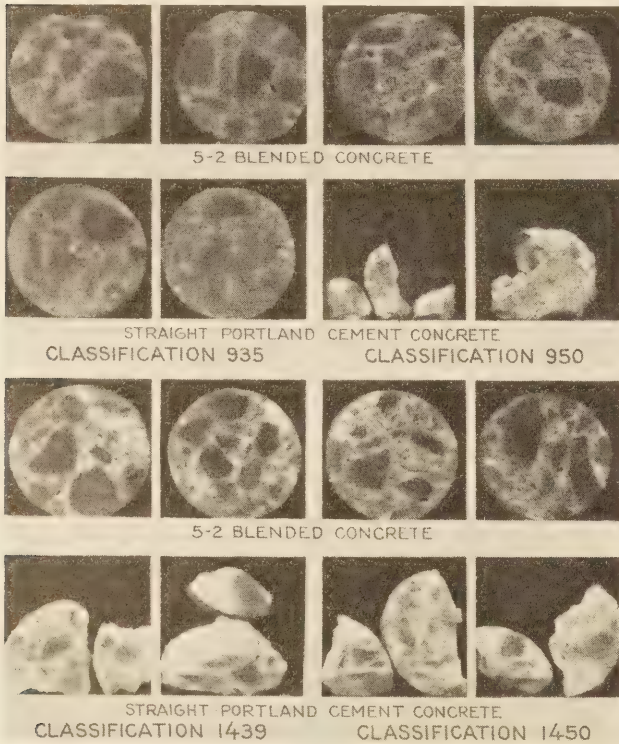


FIGURE 13.—PROJECT B. APPEARANCE OF BLENDED AND STRAIGHT PORTLAND CEMENT CONCRETE SPECIMENS AFTER 35 ALTERNATIONS OF FREEZING AND THAWING IN A 10 PERCENT SOLUTION OF CALCIUM CHLORIDE.

chloride and at the end of 50 alternations in water is shown in figures 12 to 19 inclusive. In each of these figures the specimens are arranged as follows: The four specimens shown in the upper left quadrant contain

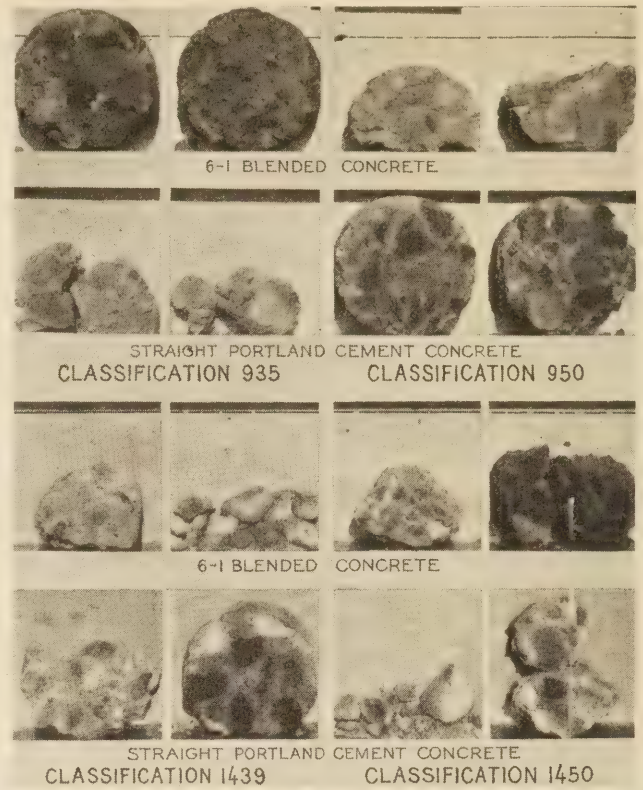


FIGURE 14.—PROJECT C. APPEARANCE OF BLENDED AND STRAIGHT PORTLAND CEMENT CONCRETE SPECIMENS AFTER 35 ALTERNATIONS OF FREEZING AND THAWING IN A 10 PERCENT SOLUTION OF CALCIUM CHLORIDE.

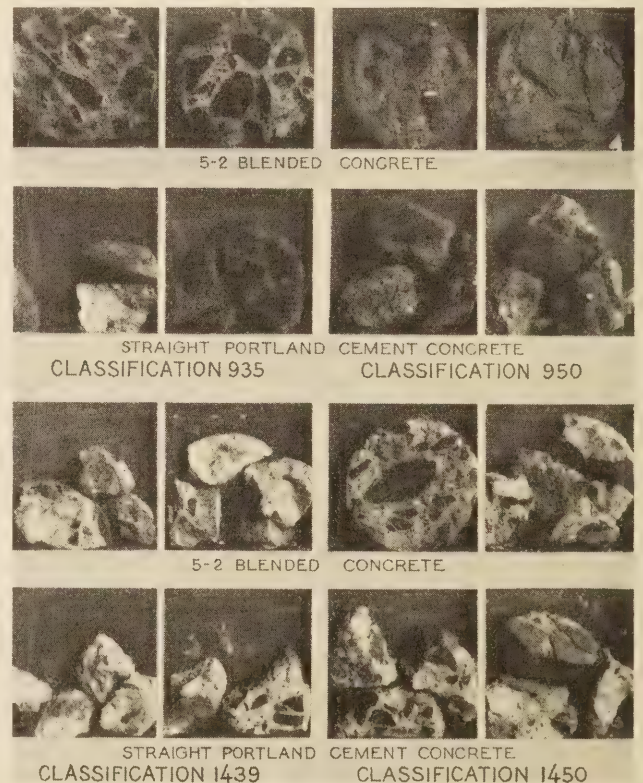


FIGURE 15.—PROJECT D. APPEARANCE OF BLENDED AND STRAIGHT PORTLAND CEMENT CONCRETE SPECIMENS AFTER 35 ALTERNATIONS OF FREEZING AND THAWING IN A 10 PERCENT SOLUTION OF CALCIUM CHLORIDE.

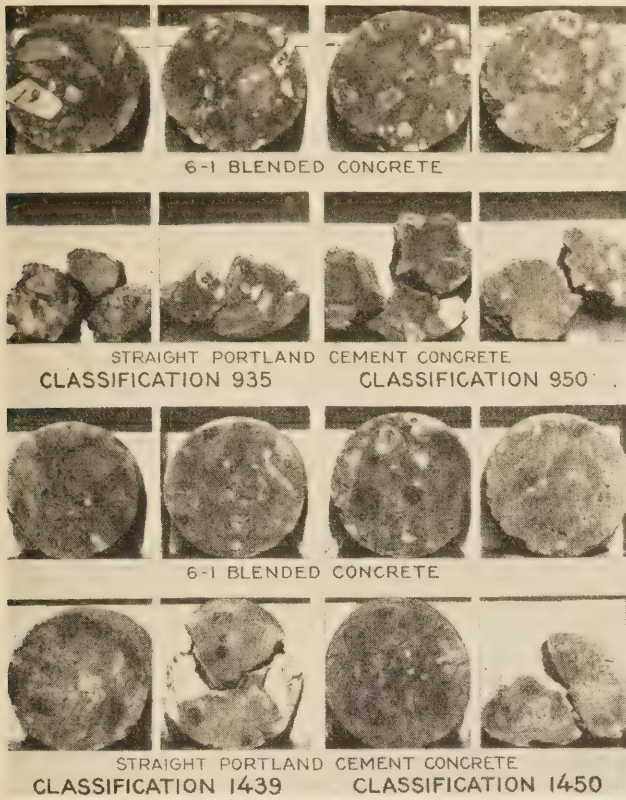


FIGURE 16.—PROJECT A. APPEARANCE OF BLENDED AND STRAIGHT PORTLAND CEMENT CONCRETE SPECIMENS AFTER 50 ALTERNATIONS OF FREEZING AND THAWING IN WATER.

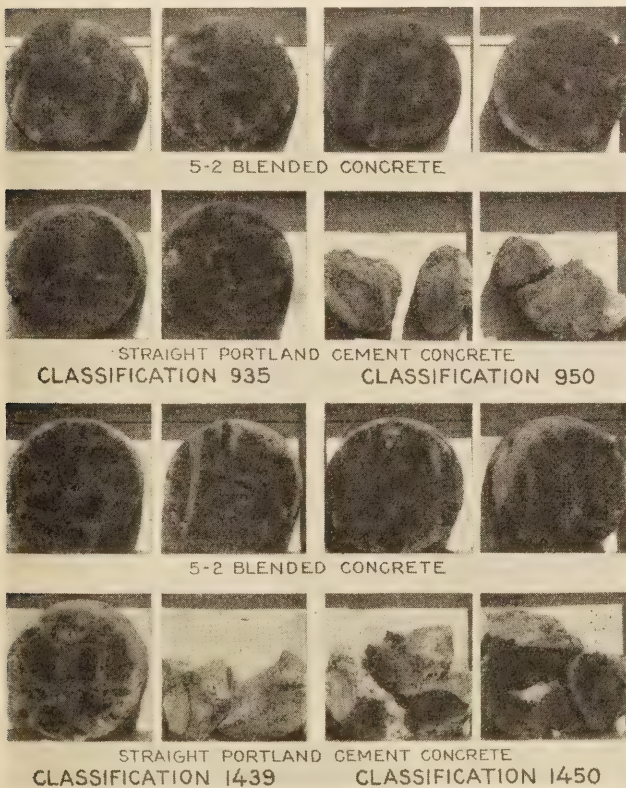


FIGURE 17.—PROJECT B. APPEARANCE OF BLENDED AND STRAIGHT PORTLAND CEMENT CONCRETE SPECIMENS AFTER 50 ALTERNATIONS OF FREEZING AND THAWING IN WATER.

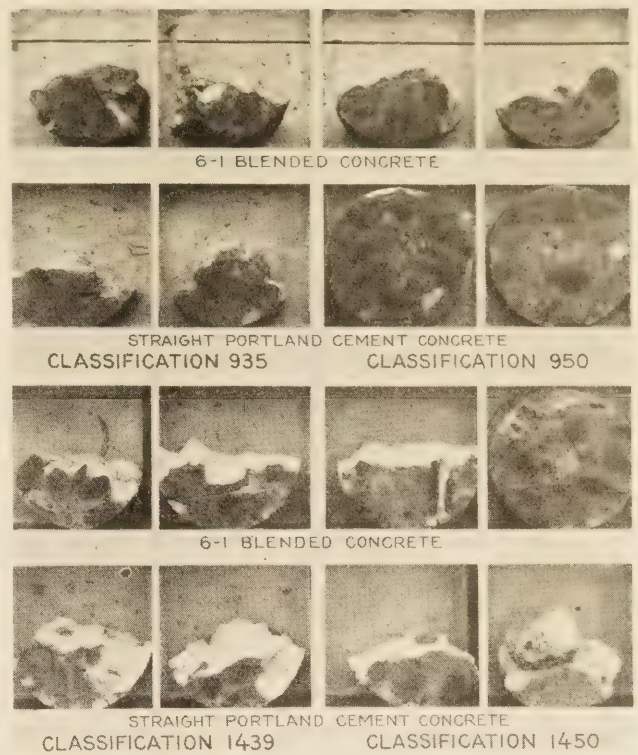


FIGURE 18.—PROJECT C. APPEARANCE OF BLENDED AND STRAIGHT PORTLAND CEMENT CONCRETE SPECIMENS AFTER 50 ALTERNATIONS OF FREEZING AND THAWING IN WATER.

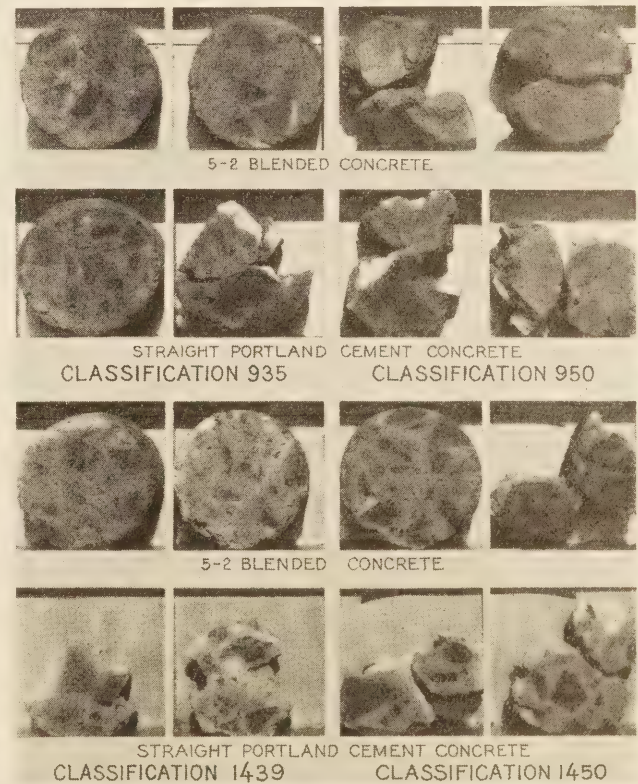


FIGURE 19.—PROJECT D. APPEARANCE OF BLENDED AND STRAIGHT PORTLAND CEMENT CONCRETE SPECIMENS AFTER 50 ALTERNATIONS OF FREEZING AND THAWING IN WATER.

portland cement conforming to New York State specification classification 935, the two specimens in the top row containing the blended cement and the two specimens in the second row the straight portland cement. Similarly, the four specimens in the upper right quadrant contain cement conforming to the 950 classification, those in the lower left quadrant the 1439 cement, and those in the lower right quadrant the 1450 cement.

Referring to table 12 and to figures 12 to 15, inclusive, it will be noted that the use of the blend resulted in a marked increase in resistance to freezing in calcium chloride in Projects A and B, some improvement in Project D and no improvement in Project C. Projects A and B contained natural cement G in the blend, whereas natural cement H was used in Projects C and D.

A study of the individual results shows that in Project A all eight of the straight portland cement specimens disintegrated, while seven of the blended cement specimens remained sound. In Project B, six of the eight straight portland cement specimens disintegrated, whereas all eight of the blended specimens remained sound. In Project C five of the unblended specimens disintegrated as compared to six of the specimens containing the blended cement. In Project D all eight of the unblended specimens disintegrated, whereas three of the blended specimens disintegrated and one was unsound.

The comparative effect of freezing in calcium chloride and in water may be noted by comparing the ratings at 35 alternations as shown in the second and third columns of table 12. It will be observed that in Projects A and B the action in water was less severe than in calcium chloride. However, in Projects C and D the amount of disintegration was about the same for freezing in both liquids. The comparatively low resistance of Projects C and D to freezing in water would indicate that the quality of the concrete was inferior to that in Projects A and B.

Study of the comparative behavior of the various specimens when frozen in water for periods beyond 35 cycles may also be made by referring to table 12. In Projects A and B, virtually the same resistance is shown for the blended cements when frozen in water up to 150 cycles as was indicated when the specimens were frozen in calcium chloride for 35 cycles. However, it will be observed that the straight portland cement concrete in Project B was considerably more resistant to freezing and thawing in water at periods beyond 35 cycles than was the concrete in Project A. The concrete in Project C appeared to be definitely inferior, all specimens in both the straight portland and the blend having disintegrated at either 60 or 65 cycles. In Project D the behavior of the straight portland cement concrete specimens also indicated comparatively inferior concrete as compared to Projects A and B. However, the blended specimens from Project D were considerably more resistant than those containing straight portland cement although, as indicated above, the improvement was not as marked as in Projects A and B.

BLENDING FOUND TO INCREASE DURABILITY BY REDUCING BLEEDING

The comparative resistance of the individual specimens when frozen in water may be studied by referring to figure 20, which shows for each test disk the number of cycles at which the various stages of unsoundness were observed. Thus, for Project A, cement classification 1439, the figure shows that of the two unblended

cement specimens, one developed questionable soundness at 50 alternations, became definitely unsound at 60, and disintegrated at 75. The other specimen was questionable at 20 alternations and disintegrated at 25. The two blended specimens were sound at 150 alternations. It will be observed that there is no consistent relation between composition of cement and resistance to freezing. This has already been commented upon and would appear to justify the method of rating the groups as a whole as shown in table 12.

An interesting feature of figure 20 is the close agreement which was observed in general between the behavior of the two specimens representing a given concrete. In many cases, failure progressed at almost an identical rate. The figure also shows the comparatively low resistance offered by the blended cement specimens from Projects C and D. Whether this was caused by the fact that natural cement H was used or by the fact that the concrete in these projects appeared, in general, to be of poorer quality than the concrete used in Projects A and B was not determined definitely. The fact remains, however, that the use of natural cement H did not improve the concrete in Project C at all and that while some improvement was shown for Project D, the results were not so positive as those indicated for Projects A and B, in both of which natural cement G was used.

Although there will be no extended discussion of the action of the blended cement concrete as compared to concrete containing only portland cement, there were certain physical differences noted in the concrete at the time the specimens were fabricated which will be discussed briefly. It has been observed in the field that there is less bleeding or water gain in concrete made with the blended cement than there is when portland cement is used alone. This observation led to a short laboratory investigation, the purpose of which was to determine the amount of water accumulating on the top of the fabricated specimens.

The concrete was proportioned in accordance with New York State highway specifications and contained approximately 6.3 sacks of cement per cubic yard. A slump of 2½ inches was used, requiring a water-cement ratio of 0.70.

Test specimens consisted of 6- by 6- by 21-inch beams molded in the usual way. After molding, the tops of the specimens were exposed to the laboratory air. Two specimens were made using 100 percent portland cement, two using a blend of 86 percent portland cement and 14 percent natural cement, and two using 72 percent portland cement and 28 percent natural cement.

At stated intervals after molding, the water which had accumulated at the top was carefully removed. The total water removed at the end of 2½ hours is shown in table 13, expressed as a percentage of the free water going into the concrete. This table also gives the results of a second test made several days later, showing the total water removed at the end of 4 hours.

From the results shown in the table it is evident that the substitution of natural cement for portland cement was effective in reducing bleeding. As will be noted, the bleeding was reduced as the amount of natural cement was increased, a replacement of 28 percent resulting in a reduction of more than one-half. Assuming that bleeding leaves the upper layer of concrete porous and weak as compared to the lower structure, it is apparent that the use of materials or methods of manipulation which will reduce bleeding should prove beneficial.

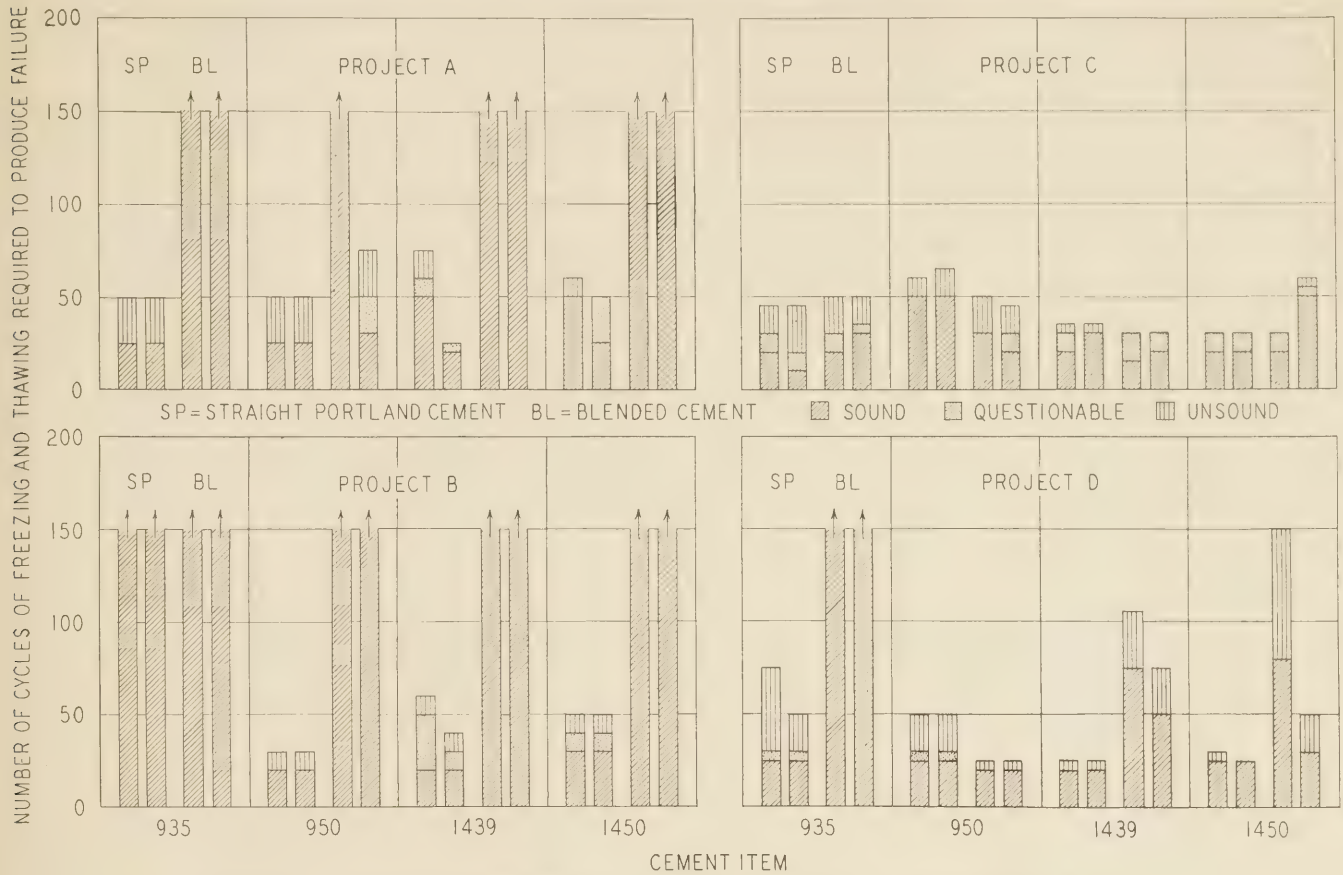


FIGURE 20.—EFFECT OF FREEZING AND THAWING CONCRETE CORE SPECIMENS IN WATER.

TABLE 13.—Water removed from tops of 6 by 6 by 21-inch beam specimens in bleeding tests

Cement blend		Water removed from specimens ¹	
Portland	Natural cement G	First test	Second test
	Percent	Percent	Percent
100 percent.....	0	7.6	7.4
86 percent.....	14	4.6	4.8
72 percent.....	28	3.5	2.9

¹ Based on total free water in concrete.

Data have been presented to show how the substitution of natural cement for portland cement materially increased the resistance to frost action of the top portions of the drilled cores.

In order to determine what effect water travel or bleeding has on durability, 2-inch disks were cut from the tops and also the bottoms of 9 identical cores and alternately frozen and thawed in a 10-percent calcium chloride solution until failure occurred. It was found that the average number of alternations required to produce failure was 20 for the tops of the cores and 35 for the bottoms, a difference of 15 alternations. All of the specimens were taken from Projects A and B and represented five different portland cements, none of which was blended with natural cement.

SUMMARY OF RESULTS

In the summary given below reference to a 1- or 2-sack replacement of portland cement by natural cement

corresponds to a 14 or 28 percent replacement by weight, respectively. The values for strength are the averages of results of tests at all ages at which determinations were made.

A. The results of tests on specimens fabricated in the laboratory indicate that replacing a portion of the portland cement with natural cement:

1. Reduced the average crushing strength of 1:2 mortar specimens 7 percent for the 1-sack replacement and 19 percent for the 2-sack replacement (figs. 2 and 3).
2. Reduced the average flexural strength of 1:2 mortar specimens 6 percent for the 1-sack replacement and 15 percent for the 2-sack replacement (figs. 4 and 5).
3. Increased expansion of 1:2 mortar bars under continuous water storage up to 180 days (figs. 6 and 7).
4. Slightly increased contraction of 1:2 mortar bars under air storage conditions up to 120 days (figs. 6 and 7).
5. Considerably reduced the expansion of 1:2 mortar bars caused by repeated freezing and thawing, followed by heating, cooling and resaturation (figs. 8 and 9).
6. Decreased the bulk specific gravity and increased the water absorption of 1:2 mortar specimens (table 9).
7. Reduced the expansion of 1:5 mortar bars when subjected to the action of a 10-percent sulphate solution (fig. 10).
8. Increased the expansion of neat cement bars when subjected to the autoclave test at 420° F. for 3 hours (fig. 10).

9. Reduced the average crushing strength of concrete specimens containing 6 sacks of cement per cubic yard of concrete, 4 percent for the 1-sack replacement and 12 percent for the 2-sack replacement (fig. 11).

10. Increased the average flexural strength of concrete specimens containing 6 sacks of cement per cubic yard of concrete 1 percent for the 1-sack replacement and reduced the flexural strength 6 percent for the 2-sack replacement (fig. 11).

11. Decreased the average modulus of elasticity of concrete specimens containing 6 sacks of cement per cubic yard of concrete 1 percent for the 1-sack replacement and 5 percent for the 2-sack replacement (fig. 11).

B. The results of tests on cores taken from concrete pavements indicate that replacing a portion of the portland cement with natural cement considerably

increased the resistance of the surface of the concrete to freezing and thawing in water and in a 10-percent solution of calcium chloride (table 12 and fig. 20).

In the tests covered by the above statement greater improvement in resistance was obtained by the use of natural cement G than by the use of natural cement H.

CONCLUSIONS

These tests indicate that, although the crushing and flexural strength of pavement concrete may be slightly reduced by the replacement of 14 or 28 percent of the portland cement with natural cement G, the resistance of the pavement surface to alternate freezing and thawing will be materially increased. No laboratory tests were made using natural cement H and tests of field specimens indicate that natural cement H did not improve the resistance of the concrete to the same extent as natural cement G.

HIGHWAY RESEARCH BOARD TO MEET SOON

The Eighteenth Annual Meeting of the Highway Research Board of the National Research Council will be held in Washington, D. C., Monday, November 28, to Friday, December 2, 1938. Reports on highway research investigations will be presented, and the formal meetings of the Board will be interspersed with open meetings for informal discussion of pertinent topics. A program of reports is to be announced by the Board in the near future.

DISPOSITION OF STATE MOTOR-FUEL TAX RECEIPTS, 1937

[Compiled for calendar year from reports of State authorities]

State	Net total receipts of calendar year	Adjustments undistributed funds etc. 1	Tax on aviation gasoline deducted	Net total funds distributed 2	Expenses of collection, maintenance and administration 3	For State highway purposes				For local roads and streets 6				For nonhighway purposes				Total 1,000 dollars				
						Construction and maintenance administration 4		State highway and police 5		Service of State highway obligations		Total State highway purposes		For work on county and local roads 4		For work on city streets 7			Service of local highway obligations		Total	
						1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars		1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars
Alabama	13,350	61	13,411	4,195	2,829	2,392	2,392	2,392	6,685	104	104	6,685	104	6,685	7	7	3,167	3,239				
Arizona	4,333		4,333	2,251	2,083	2,083	2,083	2,083	1,287			1,287		1,287			82					
Arkansas	9,962		9,962	26,373	4,440	2,111	6,551	26,373	15,283	3,595	3,595	18,878	82	18,878								
California	46,624	-1,205	45,419	26,373	4,440	2,111	6,551	26,373	15,283	3,595	3,595	18,878	82	18,878								
Colorado	7,431	-209	7,222	4,528	236	570	770	4,528	1,955			1,955		1,955								
Connecticut	9,496		9,287	7,406	183	41	1,830	7,406	9,236			9,236		9,236								
Delaware	2,034	16	2,050	1,364	39	41	1,329	1,364	1,830			1,830		1,830								
Florida	22,466		22,462	10,170	56	3	3,300	10,170	12,462	3,142	3,142	13,604		13,604								
Georgia	19,550		19,550	4,056	3		3,269	4,056	3,269	3,142	3,142	6,401		6,401								
Idaho	4,037	43	4,080	11,836	261	353	11,836	11,836	14,355			14,355		14,355								
Illinois	36,266	42	36,308	14,355	92	23	14,355	14,355	17,190			17,190		17,190								
Indiana	23,497	2,957	26,454	3,921	73	106	3,921	3,921	7,316			7,316		7,316								
Iowa	13,047	55	13,102	6,317	72	238	6,317	6,317	12,611			12,611		12,611								
Kansas	10,236	4	10,240	12,461	153	74	12,461	12,461	15,599			15,599		15,599								
Kentucky	12,671	46	12,717	1,100	87	19	1,100	1,100	8,708			8,708		8,708								
Louisiana	16,000	-3	15,997	3,364	165	29	3,364	3,364	4,972			4,972		4,972								
Maine	5,550		5,550	3,929	50	13	3,929	3,929	4,453			4,453		4,453								
Maryland	9,857	13	9,870	1,443	34	50	1,443	1,443	5,885			5,885		5,885								
Massachusetts	19,856		19,856	5,145	247	11	5,145	5,145	2,282			2,282		2,282								
Michigan	29,507	127	29,634	19,699	253	49	19,699	19,699	22,699			22,699		22,699								
Minnesota	15,493		15,493	10,051	144	38	10,051	10,051	10,195			10,195		10,195								
Mississippi	10,222	7	10,229	2,818	100	51	2,818	2,818	3,415			3,415		3,415								
Missouri	11,217	68	11,285	6,913	205	19	6,913	6,913	11,037			11,037		11,037								
Montana	4,581		4,581	4,064	19	19	4,064	4,064	6,630			6,630		6,630								
Nebraska	11,050		11,050	6,327	106	4	6,327	6,327	8,225			8,225		8,225								
Nevada	1,177	1	1,178	1,135	29	9	1,135	1,135	1,173			1,173		1,173								
New Hampshire	3,286		3,286	2,012	40	825	2,012	2,012	7,833			7,833		7,833								
New Jersey	21,786	193	21,979	1,614	138	2	1,614	1,614	7,633			7,633		7,633								
New Mexico	4,022	-25	3,997	2,302	90	2	2,302	2,302	3,905			3,905		3,905								
New York	61,915	-4,408	57,507	8,805	438	55	8,805	8,805	13,011			13,011		13,011								
North Carolina	23,393		23,367	51,216	449	189	51,216	51,216	21,918			21,918		21,918								
North Dakota	2,935	-133	2,802	1,621	61	156	1,621	1,621	1,810			1,810		1,810								
Ohio	146,588	485	147,073	19,329	359	37	19,329	19,329	7,905			7,905		7,905								
Oklahoma	13,772	-298	13,474	9,818	37	25	9,818	9,818	3,352			3,352		3,352								
Oregon	9,801	25	9,826	3,258	298	22	3,258	3,258	2,733			2,733		2,733								
Pennsylvania	55,720	18,000	73,720	63,820	298	31	63,820	63,820	2,973			2,973		2,973								
Rhode Island	3,094	24	3,118	3,118	22	18	3,118	3,118	1,500			1,500		1,500								
South Carolina	11,135	9	11,144	4,495	46	3	4,495	4,495	6,815			6,815		6,815								
South Dakota	4,155	343	4,498	18,223	179	93	18,223	18,223	10,246			10,246		10,246								
Tennessee	18,938	-676	18,262	41,626	642	36	41,626	41,626	8,026			8,026		8,026								
Texas	41,678	-52	41,626	3,312	133		3,312	3,312	3,445			3,445		3,445								
Utah	3,424	69	3,493	2,506	3	3	2,506	2,506	310			310		310								
Vermont	2,323	183	2,506	16,159	26	26	16,159	16,159	518			518		518								
Virginia	16,122	37	16,159	15,282	26	11	15,282	15,282	11,119			11,119		11,119								
Washington	8,496		8,496	4,066	11	214	4,066	4,066	4,419			4,419		4,419								
West Virginia	19,751	400	20,151	2,457	13	13	2,457	2,457	1,662			1,662		1,662								
Wisconsin	2,500		2,500	2,467	13	13	2,467	2,467	1,812			1,812		1,812								
Wyoming	2,732		2,732	2,732			2,732	2,732	2,724			2,724		2,724								
District of Columbia																						
Total	761,998	6,243	768,241	359,797	4,376	70,984	31,296	102,283	466,456	130,972	30,990	9,180	171,142	3,870	2,890	45,181	32,825	31,365	119,404			

1 Amounts distributed during the calendar year often differ from actual collections because of undistributed funds and lag between accounts of collecting and expending agencies.

2 In many States the proceeds of highway user taxes are placed in a common fund from which a distribution is made. (The amounts so distributed have been prorated in proportion to the receipts not otherwise dedicated. See the following tables.)

3 Where reported separately from collection expenses, funds allotted for motor-fuel inspection, administration or motor vehicle department, and regulation of motor vehicles are shown in this column.

4 The following allotments for construction and maintenance of county roads under State control are included in State highway purposes: Delaware, \$259,000; North Carolina, \$7,499,000; Virginia, \$7,000,000; West Virginia, \$1,231,000.

5 Reimbursement to local units of Government for amounts spent on roads now on State system.

6 In States indicated by star (*) law provides that these funds may also be used for service of local highway obligations. Amounts so used not reported separately. In Colorado funds may be used on both State and local roads.

7 This column shows specific allotments for city streets. Where reported separately, funds allotted for urban extensions of State highway system are included in allotments for State highway purposes.

8 To State general funds except in Wisconsin, where amount went to towns, cities, and villages in lieu of personal property taxes formerly imposed on motor vehicles. Allotments to local general funds may have been used in part for highways, but such amounts not reported.

9 For the following purposes: Arizona, irrigation engineering expenses; Delaware, beach protection; Florida, aviation; Louisiana, harbor improvement; New Jersey, service of institutional construction bonds, \$520,000, and Department of Commerce and Aviation, \$243,000; North Carolina, State probation commission; Oklahoma, service of general State debt; Pennsylvania, aircraft landing fields, \$214,000, and cooperative work other departments, \$24,000; South Dakota, payment on real estate bonds; Tennessee, debt service on nonhighway bonds; Virginia, aviation.

10 Legislative restorations from proceeds of sale of general State bonds were applied to debt service payments, thus reducing amounts necessary from current revenue.

11 Includes debt service charges on emergency relief bond issues prorated in proportion to use of proceeds for State highway, local road, other highway, and nonhighway purposes.

12 Paid out of motor-vehicle revenue, \$3,000. See the following table.

13 Service of highway relief bonds, a State obligation incurred for improvement of local roads.

14 Appropriations for highway purposes out of State general fund have been credited against payments of motor-fuel tax and motor-vehicle fees to the general fund and prorated in proportion to net receipts not otherwise deducted.

15 Included in cost of collecting motor-vehicle revenue. See the following table.

16 Tax of \$662,000 on non-motor-vehicle fuels not included.

17 Amounts previously loaned to general fund for relief purposes entirely repaid during 1937.

18 Estimated from fiscal-year appropriations.

19 Paid out of general revenue. Amount not reported.

DISPOSITION OF STATE MOTOR-VEHICLE RECEIPTS, 1937

(Compiled for calendar year from reports of State authorities)

State	Net total receipts of calendar year	Adjustments due to unliquidated funds, etc.	Net funds disbursed	Expenditures and administration	For other administrative purposes	Construction, maintenance, and administration	For State highway purposes			For local roads and streets				For nonhighway purposes				Total		
							State highway police	Service of State highway obligations		Total for State highway purposes	For work on county roads and local roads	For work on city streets	Service of local highway obligations	Total	For other highway purposes (park and forest roads, etc.)	To general funds	For relief of unemployment or destitution		For education	For other specific purposes
								State highway bonds and notes	State highway assumed obligations											
Alabama	4,439	135	4,574	484	1,000	2,085	406	885	3,226	621	621	1,483	222	835	1,000	754	1,000	764		
Arizona	1,143		1,143	235		876	32		908											
Arkansas	3,241		3,241	101		770	130	1,518	2,240	3,140	3,140	3,140		3,140					7,756	
California	24,003	1,069	25,072	3,028	145	3,744	2,041	3,964	10,349	3,794	3,794	10,349		10,349					7,756	
Colorado	2,603	5	2,608	466	55	1,199	371	1,119	2,197	2,963	2,963	2,963		2,963					3	
Connecticut	6,691	-407	6,284	1,124		1,501	325	24	1,852	(3)	(3)	1,852	80	80					5,508	
Delaware	1,202	75	1,277	104		5,790	77		1,082			1,082							3	
Florida	6,196		6,196	464	224	941	320	222	1,483	621	621	1,483		1,483					3	
Georgia	2,368		2,368	264		941	320	222	1,483	621	621	1,483		1,483					5,508	
Idaho	2,498	-25	2,473	71		289	101		390	*2,012	2,012	3,900		3,900					207	
Illinois	21,430	422	21,852	831	361	8,907	1,436	8,837	19,552	891	891	19,552		19,552		207			2,621	
Indiana	9,827	29	9,856	931		3,907	446		4,353	1,561	1,561	4,353	390	4,743		2,621			1,825	
Iowa	11,918	67	11,985	1,387		5,780	446		10,598	904	904	10,598		10,598					1,825	
Kansas	4,537	-260	4,277	709	26	2,301	26	86	2,221	600	600	2,221		2,221					1,825	
Kentucky	5,125	-4	5,121	469		2,174	27	447	3,718	718	718	3,718		3,718					1,825	
Louisiana	4,710	-201	4,509	73		2,946	325	2,946	3,338	375	375	3,338		3,338					1,825	
Maine	3,865	-1	3,864	151		1,691	110	969	3,338	375	375	3,338		3,338					1,825	
Maryland	5,577	-203	5,374	522	56	2,259	1,691	1,107	3,338	375	375	3,338		3,338					1,825	
Massachusetts	6,875	-219	6,656	1,580	35	1,691	378	11,107	3,338	375	375	3,338		3,338					1,825	
Michigan	8,867	35	8,902	428		2,136	103	12,951	3,338	375	375	3,338		3,338					1,825	
Minnesota	22,085	-203	21,882	1,700		4,455	321	1,748	8,369	212	212	8,369		8,369					1,825	
Mississippi	8,248	-11	8,237	108		2,212	168	3,200	9,013	1,354	1,354	9,013	41	9,054					1,825	
Missouri	9,638	95	9,733	73		5,645	159		752	2,482	2,482	752		752					1,825	
Montana	1,532		1,532	201	21	657	75	87	732	232	232	732		732					1,825	
Nebraska	2,603	-159	2,444	201		2,282	25	4	2,307	189	189	2,307		2,307					1,825	
Nevada	282		282	25		166	4		87	75	75	87		87					1,825	
New Hampshire	3,347	56	3,403	126	3	1,770	139	4	1,913	216	216	1,913		1,913					1,825	
New Jersey	19,271	-23	19,248	176		3,897	114	4	3,897	5,794	5,794	3,897		3,897					1,825	
New Mexico	1,544		1,544	93		599	114		713	218	218	713		713					1,825	
New York	52,901	-288	52,613	756	176	15,279	847	6,445	22,571	10,692	10,692	22,571		22,571					1,825	
North Carolina	8,855	-637	8,218	446		5,041	159	2,449	7,772	595	595	7,772		7,772					1,825	
North Dakota	1,581	-19	1,562	130	22	6,273	121	62	6,774	595	595	6,774		6,774					1,825	
Ohio	25,635	1,284	26,919	1,403	180	1,603	294	821	2,471	*2,585	2,585	2,471		2,471					1,825	
Oklahoma	5,584	8	5,592	930		1,579	71		1,897	*2,585	2,585	1,897		1,897					1,825	
Oregon	3,378	19	3,397	460		4,182	137	3,287	46,546	193	193	46,546		46,546					1,825	
Pennsylvania	38,332	13	38,345	49,120		1,300	67	126	3,287	46,546	46,546	3,287		3,287					1,825	
Rhode Island	2,700	69	2,769	258		1,648	282	213	1,648	193	193	1,648		1,648					1,825	
South Carolina	1,690		1,690	142		645	282	408	1,648	193	193	1,648		1,648					1,825	
South Dakota	1,650		1,650	77	4	322	443		3,980	1,212	1,212	3,980		3,980					1,825	
Tennessee	4,233	-2	4,231	261		3,537	443		6,725	*11,671	11,671	6,725		6,725					1,825	
Texas	19,684	25	19,709	993	320	6,282	443		6,725	*11,671	11,671	6,725		6,725					1,825	
Utah	1,049	-241	808	123	12	11	592	592	6,003	862	862	6,003		6,003					1,825	
Vermont	2,410	175	2,585	87		298	94	298	1,631	298	298	1,631		1,631					1,825	
Virginia	6,153	-2	6,151	595		4,976	295	375	5,556	13	13	5,556		5,556					1,825	
Washington	4,402	52	4,454	347		2,604	1,490		4,091	13	13	4,091		4,091					1,825	
West Virginia	6,162	216	6,378	771	32	2,845	10	3,091	5,946	(5)	(5)	5,946		5,946					1,825	
Wisconsin	12,934	-506	12,428	771		5,707	12	166	1,518	2,596	2,596	1,518		1,518					1,825	
Wyoming	597	4	601	86		5,411	12		166	589	589	166		166					1,825	
Dist. of Columbia	878	-2	876	86	75	411			166			166		166					1,825	
Total	399,613	40,788	440,401	30,188	1,861	163,770	14,476	41,821	53,098	231,334	89,332	231,334	12,124	2,483	104,539	25,089	6,858	5,508	38,096	

1 Amounts distributed during the calendar year often differ from actual collections because of undistributed funds and lag between accounts of collecting and expending agencies.

2 In many States the proceeds of highway "user taxes" have been placed in a common fund from which a distribution is made. The amounts so distributed have been prorated in proportion to the receipts not otherwise dedicated. See table that precedes and follow this table.

3 Collection expenses in many States include service charges deducted by county and local collectors.

4 Where reported separately, from collection expenses, funds allotted for collection of motor-fuel tax, payments to auto theft fund, and miscellaneous expenses of motor-vehicle regulation are shown in this column.

5 The following allotments for construction and maintenance of county roads under State control are included in State highway purposes: Delaware, \$150,000; North Carolina, \$2,059,000; West Virginia, \$863,000.

6 In States indicated by star (*) government allotments for amounts spent on roads now on State system.

7 In States indicated by star (*) government allotments for these funds may also be used for service of local highway obligations. Amounts so used not reported separately. In Colorado funds may be used on both State and local roads.

8 This column shows specific allotments for city streets. Where reported separately, funds allotted for urban extensions of State highway system are included in allotments for State highway purposes.

9 To State general funds except in the following States: Alabama, county and municipal general funds; California, towns, cities, and villages in lieu of personal property tax formerly imposed on motor vehicles, \$1,333,000. Allotments to local general funds may have been used in part for highways, but such amounts not reported.

10 For the following purposes: Delaware, beach protection; Ohio, hospitalization of indigent persons injured in motor-vehicle accidents; Pennsylvania, aircraft landing fields, \$237,000, and cooperative work other department, \$27,000.

11 Legislative restorations from proceeds of sale of general State bonds were applied to debt service payments, thus reducing amounts necessary from current revenue.

12 Includes debt service charges on emergency relief bond issues prorated in proportion to use of proceeds for State highway, local road, other highway, and nonhighway purposes.

13 Service of highway relief bonds, a State obligation incurred for improvement of local roads.

14 Appropriations for highway purposes out of State general fund have been credited against payments of motor-fuel tax and motor-vehicle fees to the general fund and prorated in proportion to net receipts not otherwise dedicated.

15 Amounts previously loaned to general fund for relief purposes entirely repaid during 1937.

DISPOSITION OF STATE MOTOR-CARRIER TAX RECEIPTS, 1937

[Compiled for calendar year from reports of State authorities]

State	Net total receipts of calendar year	Adjustments due to undistributed funds, etc. ¹	Net total funds distributed ²	Expenses of collection and administration ³	For State highway purposes				For local roads and streets ⁵				For nonhighway purposes					
					Construction, maintenance, and administration ³	State highway police	Service of State highway obligations		Total for State highway purposes	For work on county and local roads ³	For work on city streets ⁶	Service of local highway obligations	Total	For other highway purposes (park forest roads, etc.)	To general funds	For relief of unemployment or destitution	For education	Total
							1,000 dollars	1,000 dollars										
Alabama	216	-1	217	58	159	5	147	159	147	2	16	2,401			2,401			
Arizona	162	-2	160	13	142		46	46	282	262	86				2,401			
Arkansas	3,048	-41	3,007	544	238	13	31	282	262	86					2,401			
California	621	-21	642	98	238	13	31	282	262	86					2,401			
Colorado	211	-15	196	88			22	22	110						2,401			
Connecticut	(7)														2,401			
Delaware	262	-4	258	32	218		52	270	270						2,401			
Florida	155	226	381	111	81	32		113	113						2,401			
Georgia	155	37	143	30											2,401			
Idaho	106														2,401			
Illinois	(7)														2,401			
Indiana	682	-148	534	134	400			400	400						2,401			
Iowa	537		537	135											2,401			
Kansas	1,163		241	549	20	38	60	667	215	402					2,401			
Kentucky	1,394	85	1,479	57	415	5		420	2	2					2,401			
Louisiana	3														2,401			
Maine	22	-4	18	18											2,401			
Maryland	(8)														2,401			
Massachusetts	55		55	55											2,401			
Michigan	403	-58	345	197	148			148	148						2,401			
Minnesota	23		23	23											2,401			
Mississippi	158		158	4											2,401			
Missouri	521	411	932	79	497	15	281	793	60	154					2,401			
Montana	46	-11	35	35											2,401			
Nebraska	27		27	27											2,401			
Nevada	198		198	12	181			186							2,401			
New Hampshire	4		4	4											2,401			
New Jersey	84	-7	84	22	104	6		22	19	7	26		35		2,401			
New Mexico	153	-9	144	34				110							2,401			
New York	(7)														2,401			
North Carolina	251	-18	233	151	4	73	4	232	(9)						2,401			
North Dakota	33		33	23				10							2,401			
Ohio	492	214	706	108	571			571	27	27					2,401			
Oklahoma	1,271	-5	1,266	91	843			843	332	332					2,401			
Oregon	1,094	-36	1,058	211	443	43	230	716	*124	124					2,401			
Pennsylvania	8		8	8				8							2,401			
Rhode Island	9		9	9											2,401			
South Carolina	189	-54	135	32	93			93							2,401			
South Dakota	484	-15	469	41	423			423							2,401			
Tennessee	369	-2	371	69	234			234							2,401			
Texas	109		109	102				81							2,401			
Utah	113	-15	98	11	81			7							2,401			
Vermont	(7)														2,401			
Virginia	200	-3	197	26	135			145							2,401			
Washington	245		245	245				10							2,401			
West Virginia	91		91	36				55							2,401			
Wisconsin	1,559	-55	1,503	355				91							2,401			
Wyoming	227	-1	226	33	188	5		193							2,401			
District of Columbia	219		219												2,401			
Total	16,216	505	16,721	3,342	6,513	171	700	838	1,709	217	1,936	8	3,872	35	6	3,913		

¹ Amounts distributed during the calendar year often differ from actual collections because of undistributed funds and lag between accounts of collecting and expending agencies.
² In many States the proceeds of highway user taxes are placed in a common fund from which a distribution is made. The amounts so distributed have been prorated in proportion to the receipts not otherwise dedicated. See 2 preceding tables.
³ Approximately \$80,000 allotted for use on county roads under State control in North Carolina included in State highway purposes.
⁴ Reimbursement to local units of Government for amounts spent on roads now on State system.
⁵ In States indicated by star (*) law provides that these funds may also be used for service of local highway obligations. Amounts so used not reported separately. In Colorado funds may be used on both State and local roads.
⁶ This column shows specific allotments for city streets. Where reported separately, funds allotted for urban extensions of State highway system are included in allotments for State highway purposes.
⁷ No special taxes on motor carriers reported.
⁸ Ton-mile and passenger-mile taxes paid by motor carriers in lieu of registration fees included in motor-vehicle receipts, preceding table.

DISPOSITION OF RECEIPTS FROM STATE IMPOSTS ON HIGHWAY USERS, 1937

(Compiled for calendar year from reports of State authorities)

State	Net total receipts of calendar year ¹	For State highway purposes				For local roads and streets ²				For nonhighway purposes																							
		1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars	1,000 dollars																			
															Adjustments due to tributed funds, etc. ³	Tax on aviation fuel, etc. ⁴	Net funds distributed	Expenditures	Construction, maintenance and administration ⁵	State highway police and transportation ⁶	State highway bonds and notes ⁷	State-estimated local obligations ⁸	Total State highway obligations	Total for State highways and streets ⁹	For other highway purposes (park forest roads, etc.) ¹⁰	To general funds ¹¹	For relief employment or destination ¹²	For other specific purposes ¹³	Total				
Alabama	18,005	18,202	400	6,439	1,000	3,227	8,983	3,227	3,227	10,072	6,886	1,297	764	7	82	10,157	7	8	8,681	1,000 dollars	764	7	82	10,157	7	8	8,681	1,000 dollars	764	7	82	10,157	
Arizona	12,208	13,208	344	3,857	131	5,953	3,983	1,970	1,943	19,943	6,233	2,297	1,297	7	82	10,157	7	8	8,681	1,000 dollars	764	7	82	10,157	7	8	8,681	1,000 dollars	764	7	82	10,157	
Arkansas	70,675	70,675	3,885	3,064	2,641	3,964	720	3,243	3,243	36,768	19,093	3,238	3,238	3,595	22,688	3,049	218	218	17,273	1,000 dollars	3,595	218	218	22,688	3,049	218	218	17,273	1,000 dollars	3,595	218	218	22,688
California	16,308	19,707	1,175	8,145	210	65	320	2,223	2,223	11,343	3,049	218	218	218	22,688	3,049	218	218	17,273	1,000 dollars	3,595	218	218	22,688	3,049	218	218	17,273	1,000 dollars	3,595	218	218	22,688
Colorado	16,308	19,707	1,175	8,145	210	65	320	2,223	2,223	11,343	3,049	218	218	218	22,688	3,049	218	218	17,273	1,000 dollars	3,595	218	218	22,688	3,049	218	218	17,273	1,000 dollars	3,595	218	218	22,688
Connecticut	3,226	3,226	1,139	9,133	39	39	11,329	320	2,673	2,673	4,362	2,012	2,012	2,012	17,273	2,680	418	418	13,449	1,000 dollars	2,680	418	418	13,449	2,680	418	418	13,449	1,000 dollars	2,680	418	418	13,449
Delaware	28,924	28,924	1,139	9,133	39	39	11,329	320	2,673	2,673	4,362	2,012	2,012	2,012	17,273	2,680	418	418	13,449	1,000 dollars	2,680	418	418	13,449	2,680	418	418	13,449	1,000 dollars	2,680	418	418	13,449
Florida	22,073	22,073	2,260	9,753	320	320	11,329	320	2,673	2,673	4,362	2,012	2,012	2,012	17,273	2,680	418	418	13,449	1,000 dollars	2,680	418	418	13,449	2,680	418	418	13,449	1,000 dollars	2,680	418	418	13,449
Georgia	22,073	22,073	2,260	9,753	320	320	11,329	320	2,673	2,673	4,362	2,012	2,012	2,012	17,273	2,680	418	418	13,449	1,000 dollars	2,680	418	418	13,449	2,680	418	418	13,449	1,000 dollars	2,680	418	418	13,449
Idaho	57,696	58,000	1,806	20,743	136	136	4,426	382	8,537	8,537	31,598	9,303	9,303	9,303	17,273	2,680	418	418	13,449	1,000 dollars	2,680	418	418	13,449	2,680	418	418	13,449	1,000 dollars	2,680	418	418	13,449
Illinois	24,006	25,024	1,236	9,701	446	344	8,087	8,087	8,087	8,087	19,108	10,769	10,769	10,769	17,273	2,680	418	418	13,449	1,000 dollars	2,680	418	418	13,449	2,680	418	418	13,449	1,000 dollars	2,680	418	418	13,449
Indiana	24,006	25,024	1,236	9,701	446	344	8,087	8,087	8,087	8,087	19,108	10,769	10,769	10,769	17,273	2,680	418	418	13,449	1,000 dollars	2,680	418	418	13,449	2,680	418	418	13,449	1,000 dollars	2,680	418	418	13,449
Iowa	19,926	19,926	1,653	9,167	186	186	3,444	3,444	3,444	3,444	10,647	3,600	3,600	3,600	13,449	1,000 dollars	3,600	3,600	13,449	1,000 dollars	3,600	3,600	3,600	13,449	3,600	3,600	3,600	13,449	1,000 dollars	3,600	3,600	3,600	13,449
Kansas	19,926	19,926	1,653	9,167	186	186	3,444	3,444	3,444	3,444	10,647	3,600	3,600	3,600	13,449	1,000 dollars	3,600	3,600	13,449	1,000 dollars	3,600	3,600	3,600	13,449	3,600	3,600	3,600	13,449	1,000 dollars	3,600	3,600	3,600	13,449
Kentucky	20,713	20,713	20,709	237	15,030	185	7,968	7,968	7,968	7,968	12,425	8,980	8,980	8,980	13,449	1,000 dollars	8,980	8,980	13,449	1,000 dollars	8,980	8,980	8,980	13,449	8,980	8,980	8,980	13,449	1,000 dollars	8,980	8,980	8,980	13,449
Louisiana	4,437	4,437	4,437	188	5,623	275	2,412	2,412	2,412	2,412	8,310	934	934	934	13,449	1,000 dollars	934	934	13,449	1,000 dollars	934	934	934	13,449	934	934	934	13,449	1,000 dollars	934	934	934	13,449
Maine	14,434	14,434	1,720	7,281	350	350	3,243	3,243	3,243	3,243	8,564	4,150	4,150	4,150	13,449	1,000 dollars	4,150	4,150	13,449	1,000 dollars	4,150	4,150	4,150	13,449	4,150	4,150	4,150	13,449	1,000 dollars	4,150	4,150	4,150	13,449
Massachusetts	26,764	26,764	26,764	142	20,302	281	2,611	2,611	2,611	2,611	23,623	25,679	25,679	25,679	13,449	1,000 dollars	25,679	25,679	13,449	1,000 dollars	25,679	25,679	25,679	13,449	25,679	25,679	25,679	13,449	1,000 dollars	25,679	25,679	25,679	13,449
Michigan	31,938	31,938	31,938	600	14,265	281	1,748	1,748	1,748	1,748	18,555	5,098	5,098	5,098	13,449	1,000 dollars	5,098	5,098	13,449	1,000 dollars	5,098	5,098	5,098	13,449	5,098	5,098	5,098	13,449	1,000 dollars	5,098	5,098	5,098	13,449
Minnesota	24,358	24,358	24,358	203	3,650	388	7,400	7,400	7,400	7,400	20,843	60	60	60	13,449	1,000 dollars	60	60	13,449	1,000 dollars	60	60	60	13,449	60	60	60	13,449	1,000 dollars	60	60	60	13,449
Mississippi	12,628	12,628	12,628	401	7,777	800	13,055	13,055	13,055	13,055	20,843	60	60	60	13,449	1,000 dollars	60	60	13,449	1,000 dollars	60	60	60	13,449	60	60	60	13,449	1,000 dollars	60	60	60	13,449
Missouri	21,376	21,376	21,376	152	6,311	127	4,064	4,064	4,064	4,064	7,889	4,502	4,502	4,502	13,449	1,000 dollars	4,502	4,502	13,449	1,000 dollars	4,502	4,502	4,502	13,449	4,502	4,502	4,502	13,449	1,000 dollars	4,502	4,502	4,502	13,449
Montana	15,139	15,139	15,139	159	6,388	388	7,400	7,400	7,400	7,400	20,843	60	60	60	13,449	1,000 dollars	60	60	13,449	1,000 dollars	60	60	60	13,449	60	60	60	13,449	1,000 dollars	60	60	60	13,449
Nebraska	13,660	13,660	13,660	159	6,388	388	7,400	7,400	7,400	7,400	20,843	60	60	60	13,449	1,000 dollars	60	60	13,449	1,000 dollars	60	60	60	13,449	60	60	60	13,449	1,000 dollars	60	60	60	13,449
Nevada	1,657	1,657	1,657	41	4,852	38	87	87	87	87	7,069	4,502	4,502	4,502	13,449	1,000 dollars	4,502	4,502	13,449	1,000 dollars	4,502	4,502	4,502	13,449	4,502	4,502	4,502	13,449	1,000 dollars	4,502	4,502	4,502	13,449
New Hampshire	5,637	5,637	5,637	57	3,782	38	1,616	1,616	1,616	1,616	96	9	9	9	13,449	1,000 dollars	9	9	13,449	1,000 dollars	9	9	9	13,449	9	9	9	13,449	1,000 dollars	9	9	9	13,449
New Jersey	40,955	41,118	41,118	163	1,915	179	3,782	3,782	3,782	3,782	4,790	461	461	461	13,449	1,000 dollars	461	461	13,449	1,000 dollars	461	461	461	13,449	461	461	461	13,449	1,000 dollars	461	461	461	13,449
New Mexico	5,719	5,719	5,719	142	3,333	505	5,555	5,555	5,555	5,555	13,166	9,256	9,256	9,256	13,449	1,000 dollars	9,256	9,256	13,449	1,000 dollars	9,256	9,256	9,256	13,449	9,256	9,256	9,256	13,449	1,000 dollars	9,256	9,256	9,256	13,449
New York	114,816	110,120	3,009	24,087	1,335	10,160	10,160	10,160	10,160	10,160	36,582	19,237	19,237	19,237	13,449	1,000 dollars	19,237	19,237	13,449	1,000 dollars	19,237	19,237	19,237	13,449	19,237	19,237	19,237	13,449	1,000 dollars	19,237	19,237	19,237	13,449
North Carolina	32,499	31,818	502	19,408	612	9,429	473	9,902	9,902	29,922	2,635	1,500	1,500	1,500	13,449	1,000 dollars	1,500	1,500	13,449	1,000 dollars	1,500	1,500	1,500	13,449	1,500	1,500	1,500	13,449	1,000 dollars	1,500	1,500	1,500	13,449
North Dakota	4,589	4,589	4,589	142	3,333	505	5,555	5,555	5,555	5,555	13,166	9,256	9,256	9,256	13,449	1,000 dollars	9,256	9,256	13,449	1,000 dollars	9,256	9,256	9,256	13,449	9,256	9,256	9,256	13,449	1,000 dollars	9,256	9,256	9,256	13,449
Ohio	72,665	74,648	4,397	248	2,263	121	251	251	251	251	26,674	22,570	22,570	22,570	13,449	1,000 dollars																	

STATUS OF FEDERAL-AID HIGHWAY PROJECTS

AS OF SEPTEMBER 30, 1938

STATE	COMPLETED DURING CURRENT FISCAL YEAR			UNDER CONSTRUCTION			APPROVED FOR CONSTRUCTION			BALANCE OF FUND AVAILABLE FOR PROGRAMMED PROJ. ECTS	
	Estimated Total Cost	Federal Aid	Miles	Estimated Total Cost	Federal Aid	Miles	Estimated Total Cost	Federal Aid	Miles		
Alabama	1,659,461	735,930	17.6	8,126,378	4,053,960	344.6	2,176,494	1,085,955	94.4	2,875,596	
Arizona	988,010	787,011	69.3	1,289,778	941,811	44.2	194,821	120,880	4.3	1,451,953	
Arkansas	541,415	539,721	63.1	1,733,621	1,730,820	111.3	1,652,270	1,630,454	97.6	1,063,230	
California	2,950,086	1,547,565	73.9	10,179,207	5,360,556	181.0	2,137,333	1,140,456	49.6	1,808,453	
Colorado	1,466,214	806,752	64.1	2,917,207	1,605,878	73.3	920,810	511,980	31.5	1,794,558	
Connecticut				1,350,040	660,511	14.7	244,930	120,990	2.0	1,536,794	
Delaware	75,320	37,660		1,010,202	500,965	23.5	231,940	115,910	.2	1,184,277	
Florida	480,110	240,055	15.1	3,787,138	1,831,569	79.7	468,000	234,000	.3	2,981,176	
Georgia	2,807,740	1,398,558	147.4	5,981,044	2,990,522	277.9	768,970	384,485	45.1	5,527,478	
Illinois	1,299,877	775,218	116.6	1,830,239	1,088,878	111.4				1,134,504	
Indiana	1,782,500	908,384	48.7	11,668,268	5,832,370	262.2	3,149,550	1,574,774	91.8	2,328,226	
Iowa	2,055,190	1,017,595	55.5	4,891,291	2,445,645	109.0	1,793,604	896,672	36.1	2,350,710	
Kansas	2,649,293	1,232,519	81.0	7,195,830	3,242,183	237.9	1,763,024	827,500	57.2	349,747	
Kentucky	2,117,710	1,058,754	104.9	5,045,834	2,479,217	678.3	3,162,952	1,800,473	191.5	2,955,696	
Louisiana	2,229,306	1,101,911	88.4	5,150,474	2,575,237	146.5	1,520,760	760,390	45.4	2,073,578	
Maine	496,596	248,267	15.8	12,060,405	2,403,818	52.8	1,648,572	721,592	28.6	2,347,042	
Maine	1,911,860	955,864	45.6	1,792,486	891,888	34.7	435,005	217,501	10.3	230,174	
Maine	308,000	194,000	5.3	1,891,567	944,021	29.0	1,688,981	820,830	28.3	1,670,916	
Massachusetts	567,951	283,974	3.8	2,644,499	1,322,246	13.5	859,151	429,990	8.9	2,316,555	
Michigan	2,790,200	1,395,100	86.6	6,481,208	3,168,694	124.2	1,736,469	853,222	40.5	1,713,765	
Minnesota	567,280	256,994	10.5	7,088,576	3,528,072	345.5	2,123,224	1,053,519	103.4	1,924,238	
Mississippi	759,208	352,662	31.9	8,326,480	3,205,471	368.9	1,409,600	351,970	71.1	3,115,942	
Missouri	2,018,904	1,004,195	65.1	3,985,170	1,950,258	101.3	4,096,238	1,778,114	159.4	3,370,342	
Montana	1,383,975	778,153	69.4	6,748,888	3,191,264	19.4	317,859	176,783	11.0	4,263,986	
Nebaska	2,097,633	1,048,733	248.7	6,178,086	3,079,442	457.8	2,070,308	401,601	52.5	2,553,158	
Nevada	1,080,635	936,162	106.1	623,465	534,727	75.2	468,897	406,166	8.2	1,217,185	
New Hampshire	711,007	351,969	19.8	558,750	278,256	4.9	143,270	71,614	1.5	1,138,227	
New Jersey	1,157,176	705,751	145.4	3,028,331	1,511,228	21.7	305,990	152,370	3.3	2,537,873	
New Mexico	4,850,956	2,396,750	102.1	1,884,391	1,249,455	119.5	210,862	128,604	6.6	798,787	
New York	2,642,358	1,304,224	136.1	14,251,023	7,037,212	238.6	2,024,030	982,665	25.0	2,817,777	
North Carolina	509,340	460,000	50.2	6,837,996	3,304,827	328.2	1,769,990	873,470	95.0	1,933,713	
North Dakota	2,098,470	1,047,200	28.9	2,889,891	2,643,078	212.0	136,652	73,189	10.2	3,587,977	
Ohio	2,390,057	1,259,286	81.8	8,541,422	4,242,300	89.8	5,579,690	2,772,519	49.6	5,846,598	
Oklahoma	1,240,688	707,212	49.2	5,266,416	2,748,303	185.5	1,401,390	744,465	58.6	2,656,311	
Oregon	4,469,212	2,230,026	74.9	1,747,565	1,058,367	47.5	425,782	259,860	56.7	1,982,244	
Pennsylvania	202,800	101,400	1.7	5,963,978	2,793,958	78.3	6,560,257	3,261,179	60.0	2,672,558	
Rhode Island	2,730,437	1,209,193	152.5	939,242	469,621	14.3	287,710	143,855	2.1	1,028,944	
South Carolina	611,862	353,755	73.9	3,370,242	1,503,432	125.8	2,003,011	900,100	56.7	1,165,475	
South Dakota	1,623,080	811,540	56.0	4,089,492	2,261,560	384.5	1,580,220	872,770	117.3	2,720,117	
Tennessee	6,823,031	3,387,715	140.9	4,868,714	2,434,357	128.5	868,860	433,930	15.4	4,310,972	
Texas	725,860	521,130	102.4	10,183,495	5,046,449	509.9	4,263,605	1,886,002	212.5	6,275,668	
Utah	819,759	364,476	23.3	1,613,295	1,146,535	55.2	354,730	251,750	10.0	894,864	
Virginia	2,594,411	1,296,500	86.2	971,790	461,233	25.7	328,570	164,095	5.3	148,696	
Washington	2,249,597	1,183,100	46.2	4,646,135	2,320,402	157.1	1,510,354	752,827	40.0	688,815	
West Virginia	703,299	518,649	26.0	3,138,123	1,644,159	45.8	804,454	423,000	13.5	828,161	
Wisconsin	2,439,158	1,181,453	72.0	1,910,644	1,203,321	57.3	708,719	342,923	9.8	2,176,930	
Wyoming	1,163,657	715,911	140.0	6,856,365	3,187,350	206.2	987,244	479,220	28.8	1,551,416	
District of Columbia				1,377,252	845,491	154.8	238,950	147,820	24.8	670,022	
Hawaii											
Puerto Rico											
TOTALS	80,226,394	41,895,836	3,553.5	220,836,907	109,355,354	7,550.3	70,007,282	34,569,044	2,173.9	106,510,185	

STATUS OF FEDERAL-AID SECONDARY OR FEEDER ROAD PROJECTS

AS OF SEPTEMBER 30, 1938

STATE	COMPLETED DURING CURRENT FISCAL YEAR			UNDER CONSTRUCTION			APPROVED FOR CONSTRUCTION			Miles	FINANCE AND PLANNING AVAILABLE FOR FISCAL YEAR FROM OTHER SOURCES
	Estimated Total Cost	Federal Aid	Miles	Estimated Total Cost	Federal Aid	Miles	Estimated Total Cost	Federal Aid	Miles		
Alabama	\$ 281,361	\$ 187,459	18.6	\$ 562,000	\$ 280,700	37.8	\$ 82,605	\$ 40,200	6.2	\$ 732,072	
Arizona				45,324	27,889	2.0	76,142	45,117	4.7	395,957	
Arkansas				19,426	12,863		107,411	107,396	12.6	743,889	
California	392,268	226,045	34.8	1,228,447	670,195	62.6	421,417	230,347	33.8	720,639	
Colorado	425,450	236,550	31.6	417,036	230,283	19.5	88,360	32,000	4.8	422,735	
Connecticut				57,470	28,735	1.2	29,410	14,510	3.3	275,033	
Delaware				20,122	10,061		103,704	51,852	16.9	195,023	
Florida	123,837	63,897	17.6	498,260	249,130	58.4	105,900	52,950	7.7	611,841	
Georgia	243,115	110,501	39.8	229,211	97,855	14.7	35,697	128,040	37.1	817,955	
Idaho	553,000	276,500	44.7	1,672,992	782,496	125.2	648,400	324,200	2.3	247,261	
Illinois				772,700	334,350	76.9	1,059,148	516,702	43.7	643,719	
Indiana	56,600	24,600	9.2						97.1	366,786	
Iowa				67,822	33,911	7.6	268,154	134,076	48.4	1,298,449	
Kansas	43,670	21,835	.5	630,278	186,311	50.4	1,262,037	335,098	94.9	1,129,142	
Kentucky	309,086	104,551	52.0	155,231	72,985	11.6	878,653	371,430	75.4	188,466	
Louisiana				327,126	163,563	19.7	138,600	63,433	6.0	279,721	
Maine				6,228	3,132		64,800	29,602	8.7	14,377	
Maryland				5,300	2,650		143,050	71,315	2.4	379,739	
Massachusetts				616,866	308,433	46.1	305,290	152,645	25.8	572,435	
Michigan				321,168	151,900	24.9	318,140	158,740	40.0	1,074,461	
Minnesota	130,131	61,694	21.3	104,800	52,400	4.0	194,200	97,100	40.0	1,007,246	
Mississippi	224,357	111,427	34.3	231,370	115,290	15.3	616,650	295,705	84.1	739,427	
Missouri				13,383	7,865					612,827	
Montana				451,070	225,535	72.9	247,552	120,556	47.4	1,087,170	
Nebaska	149,082	74,541	21.6	119,166	100,152	32.7	94,427	81,932	22.1	575,420	
Nevada	228,041	194,840	30.8	111,335	55,301	2.8	39,820	19,880	1.5	117,369	
New Hampshire	97,426	48,277	1.7	123,040	61,520	2.4	283,920	103,630	1.5	123,417	
New Jersey				327,188	190,070	22.5	299,156	162,318	2.7	507,623	
New Mexico	289,750	176,717	14.4	1,420,300	692,550	94.6	1,101,600	469,450	15.2	277,667	
New York	1,484,731	739,812	102.8	1,004,664	502,310	97.0	293,440	136,520	42.2	458,403	
North Carolina	54,380	27,190	10.1	106,900	57,253	26.8	91,100	48,792	19.6	415,635	
North Dakota				184,400	92,200	3.8	60,600	29,750	1.8	681,207	
Ohio				396,738	209,979	34.2	449,030	215,734	29.9	1,709,691	
Oklahoma	12,200	6,491		152,117	92,122	9.3	60,504	28,000	6.6	752,711	
Oregon	279,315	169,410	44.7	1,201,117	581,021	73.0	1,347,074	694,837	72.8	463,622	
Pennsylvania	738,719	369,060	54.8	68,848	34,424	2.6	187,830	93,915	3.1	26,132	
Rhode Island	66,840	33,420	6.6	752,519	319,662	84.9	482,027	191,400	46.6	144,918	
South Carolina	50,120	22,700		11,300	6,250					816,436	
South Dakota				400,866	184,973	23.4	275,920	101,810	14.6	746,107	
Tennessee	53,200	26,600	1.3	2,070,709	919,140	220.8	734,944	306,775	111.0	1,585,744	
Texas	650,705	313,110	122.6	372,398	177,140	24.8	170,330	75,400	26.9	172,460	
Utah	226,240	125,370	28.1	101,376	49,153	4.8	114,700	56,200	3.8	33,093	
Vermont	163,580	71,150	9.7	728,647	315,041	53.1	243,230	113,931	20.6	337,527	
Virginia	255,430	125,500	28.8	310,853	163,278	42.2	395,197	208,000	24.6	219,378	
Washington	266,218	139,900	19.9	215,150	107,575	20.0	122,300	61,150	6.2	366,499	
West Virginia	28,900	14,450	1.4	924,461	441,190	38.5	292,117	115,380	14.4	632,253	
Wisconsin	46,947	23,405	1.4	533,660	206,180	20.9	124,440	76,890	14.0	122,723	
Wyoming	290,340	179,400	33.7	56,250	28,125	2.4	35,610	17,455	1.5	218,750	
District of Columbia				244,000	121,950	13.7				107,470	
Hawaii											
Puerto Rico											
TOTALS	8,455,439	4,393,602	853.3	20,192,329	9,757,091	1,602.0	14,670,716	6,690,141	1,231.0	26,619,502	

PUBLICATIONS of the BUREAU OF PUBLIC ROADS

Any of the following publications may be purchased from the Superintendent of Documents, Government Printing Office, Washington, D. C. As his office is not connected with the Department and as the Department does not sell publications, please send no remittance to the United States Department of Agriculture.

ANNUAL REPORTS

- Report of the Chief of the Bureau of Public Roads, 1931. 10 cents.
Report of the Chief of the Bureau of Public Roads, 1933. 5 cents.
Report of the Chief of the Bureau of Public Roads, 1934. 10 cents.
Report of the Chief of the Bureau of Public Roads, 1935. 5 cents.
Report of the Chief of the Bureau of Public Roads, 1936. 10 cents.
Report of the Chief of the Bureau of Public Roads, 1937. 10 cents.

HOUSE DOCUMENT NO. 462

- Part 1 . . . Nonuniformity of State Motor-Vehicle Traffic Laws. 15 cents.
Part 2 . . . Skilled Investigation at the Scene of the Accident Needed to Develop Causes. 10 cents.
Part 3 . . . Inadequacy of State Motor-Vehicle Accident Reporting. 10 cents.
Part 4 . . . Official Inspection of Vehicles. 10 cents.
Part 5 . . . Case Histories of Fatal Highway Accidents. 10 cents.
Part 6 . . . The Accident-Prone Driver. 10 cents.

MISCELLANEOUS PUBLICATIONS

- No. 76MP . . . The Results of Physical Tests of Road-Building Rock. 10 cents.
No. 191MP . . . Roadside Improvement. 10 cents.
No. 272MP . . . Construction of Private Driveways. 10 cents.
No. 279MP . . . Bibliography on Highway Lighting. 5 cents.
Highway Accidents. 10 cents.
The Taxation of Motor Vehicles in 1932. 35 cents.
Guides to Traffic Safety. 10 cents.
Federal Legislation and Rules and Regulations Relating to Highway Construction. 15 cents.
An Economic and Statistical Analysis of Highway-Construction Expenditures. 15 cents.
Highway Bond Calculations. 10 cents.

DEPARTMENT BULLETINS

- No. 1279D . . . Rural Highway Mileage, Income, and Expenditures, 1921 and 1922. 15 cents.
No. 1486D . . . Highway Bridge Location. 15 cents.

TECHNICAL BULLETINS

- No. 55T . . . Highway Bridge Surveys. 20 cents.
No. 265T . . . Electrical Equipment on Movable Bridges. 35 cents.
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Single copies of the following publications may be obtained from the Bureau of Public Roads upon request. They cannot be purchased from the Superintendent of Documents.

MISCELLANEOUS PUBLICATIONS

- No. 296MP . . . Bibliography on Highway Safety.

SEPARATE REPRINT FROM THE YEARBOOK

- No. 1036Y . . . Road Work on Farm Outlets Needs Skill and Right Equipment.

TRANSPORTATION SURVEY REPORTS

- Report of a Survey of Transportation on the State Highway System of Ohio (1927).
Report of a Survey of Transportation on the State Highways of Vermont (1927).
Report of a Survey of Transportation on the State Highways of New Hampshire (1927).
Report of a Plan of Highway Improvement in the Regional Area of Cleveland, Ohio (1928).
Report of a Survey of Transportation on the State Highways of Pennsylvania (1928).
Report of a Survey of Traffic on the Federal-Aid Highway Systems of Eleven Western States (1930).

UNIFORM VEHICLE CODE

- Act I.—Uniform Motor Vehicle Administration, Registration, Certificate of Title, and Antitheft Act.
Act II.—Uniform Motor Vehicle Operators' and Chauffeurs' License Act.
Act III.—Uniform Motor Vehicle Civil Liability Act.
Act IV.—Uniform Motor Vehicle Safety Responsibility Act.
Act V.—Uniform Act Regulating Traffic on Highways.
Model Traffic Ordinances.
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A complete list of the publications of the Bureau of Public Roads, classified according to subject and including the more important articles in *PUBLIC ROADS*, may be obtained upon request addressed to the U. S. Bureau of Public Roads, Willard Building, Washington, D. C.

STATUS OF FEDERAL-AID GRADE CROSSING PROJECTS

AS OF SEPTEMBER 30, 1938

STATE	COMPLETED DURING CURRENT FISCAL YEAR				UNDER CONSTRUCTION				APPROVED FOR CONSTRUCTION				BALANCE OF FUNDABLE FOR UNCOMPLETED PROJECTS		
	Estimated Total Cost	Federal Aid	NUMBER		Estimated Total Cost	Federal Aid	NUMBER		Estimated Total Cost	Federal Aid	NUMBER				
			Grade Crossing by Separate Contract or Relocation	Grade Crossing by Other			Grade Crossing by Separate Contract or Relocation	Grade Crossing by Other			Grade Crossing by Separate Contract or Relocation	Grade Crossing by Other			
Alabama	\$ 153,610	\$ 153,610	4	4	\$ 502,248	\$ 501,224	5	5	\$ 548,190	\$ 548,190	6	6	\$ 782,285		
Arizona					9,452	9,452							620,761		
Arkansas	366,760	366,760	2	2	425,535	425,296	10	10	209,841	209,472	4	4	1,125,001		
California	17,980	17,979	1	1	1,086,383	1,086,403	5	2	326,626	317,134	3	3	1,929,887		
Colorado					71,588	71,588	1	1	25,042	21,282			1,177,463		
Connecticut													844,490		
Delaware													416,480		
Florida					10,616	10,616			77,270	77,270			1,003,281		
Georgia					18,346	18,346			362,690	362,690			2,036,351		
Idaho	87,825	87,652	1	1	12,342	12,342			250,314	250,314			472,562		
Illinois	166,500	166,500	1	1	1,544,675	1,544,675	5	2	660,230	660,230	6	6	2,813,238		
Indiana	314,886	288,900	3	3	555,600	528,700	4	2	689,780	689,780	1	1	966,769		
Iowa	527,765	498,900	4	4	694,219	653,600	7	1	48,001	44,400	1	1	1,365,724		
Kansas	326,940	326,940	7	7	506,321	506,321	6	6	360,123	360,123	6	2	1,381,124		
Kentucky	145,000	145,000	1	1	51,100	51,100			548,746	548,746	5	2	1,069,198		
Louisiana					225,285	196,478	3	3	248,872	248,872	9	9	1,132,503		
Maine	48,590	48,590	2	2	327,315	327,315	1	1	2,610	2,610			312,688		
Maryland					64,586	64,586							962,247		
Massachusetts	393,000	393,000	5	5	122,260	118,859			279,660	279,660	1	1	1,514,879		
Michigan	40,218	40,218	1	1	912,458	912,458	6	6	238,600	238,600	3	3	1,741,127		
Minnesota					770,180	770,180	3	5					1,614,049		
Mississippi	128,120	128,120	2	2	355,700	355,700	4	4	363,800	363,800	3	3	864,651		
Missouri	248,112	243,230	3	3	223,970	223,970	2	2	380,320	380,320	2	2	2,292,643		
Montana	29,220	29,220	1	1	569,221	569,221	5	5					511,120		
Nebraska	97,187	97,187	2	2	296,443	296,443	12	12	497,755	497,755	5	5	940,992		
Nevada	61,732	61,425	1	1	7,406	7,406			57,561	57,561	2	2	303,893		
New Hampshire					210,005	204,778	2	1	86,081	86,081	5	1	1,634,678		
New Jersey	141,400	141,400	1	1	200,118	200,118	4	4	130,800	130,800	1	1	523,018		
New Mexico	73,550	73,550	1	1	2,472,372	2,417,071	9	7	128,355	128,355	3	3	4,028,308		
New York					470,810	470,810	5	2	214,000	183,150	2	2	1,324,404		
North Carolina					606,278	606,278	2	2	608,330	575,630	3	4	972,943		
Ohio					103,830	103,830	2	2	465,700	465,700	5	5	3,659,638		
Oklahoma	95,263	94,614	2	2	17,343	17,343			52,075	52,075			2,212,091		
Oregon	138,043	122,837	1	1	220,687	220,687	1	1	200,021	191,000	1	1	608,955		
Pennsylvania					161,073	161,073	1	1	800,021	800,021	1	1	5,257,625		
Rhode Island					314,484	314,484	1	1	272,740	272,740	12	12	1,158,731		
South Carolina					86,379	86,379	1	1	202,535	202,535	1	1	1,203,369		
South Dakota					316,708	314,568	2	2	105,850	105,850	5	5	891,343		
Tennessee	7,530	7,530			14,381	14,381			87,360	87,360			1,790,272		
Texas	36,660	35,990	2	2	928,672	928,672	8	8	389,767	389,155	5	5	3,987,972		
Utah	1,760	1,760	1	1	142,818	142,818	4	4	5,319	5,319	3	3	423,166		
Vermont	202,882	197,882	6	6	27,454	27,454			24,470	24,470			230,321		
Virginia	245,670	245,670	12	12	241,018	241,018	3	1	304,555	304,555	3	3	1,004,787		
Washington	149,718	147,618	1	2	613,457	613,457	7	2	31,117	31,117	1	1	591,609		
West Virginia	120,100	119,080	1	1	291,909	291,909	3	3	60,870	60,870	2	2	852,239		
Wisconsin	100,586	100,586	1	1	1,110,736	1,068,297	10	2	210,283	155,538	1	1	1,149,088		
Wyoming					161,544	161,544			12,410	12,410			502,776		
District of Columbia					33,230	33,230	1	1	34,262	34,262			291,169		
Hawaii					110,340	110,340	3	3	116,086	115,440			307,950		
Puerto Rico													504,470		
TOTALS	4,486,607	4,341,748	57	19	36	18,220,995	17,989,367	192	42	51	10,333,302	10,135,465	117	21	64,846,066

