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DRINKING FOUNTAIN ON AN OREGON HIGHWAY

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R. E. ROYALL, Editor

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# DRINKING FOUNTAINS ALONG OREGON HIGHWAYS 

Reported by T. M. DAVIS. Highway Engineer, United States Bureau of Public Roads


On the Ashland-Klamath Falls Highway

MOTORISTS driving along the highways in the State of Oregon will occasionally pass a sign notifying them that there is good drinking water 300 feet ahead. At such places they will find an artistically designed drinking fountain erected by the Oregon State Highway Commission and a water supply which they may feel safe in using.

No standard plans have been prepared for the construction of these fountains, each fountain being designed by the engineers to harmonize with the surroundings and to make use of the native materials at hand Though the same general scheme is followed, the designs differ, being influenced by available materials, site, and quantity of water.

At the springs a small reservoir of concrete or cement rubble masonry is constructed, from which a pipe leads to the fountain. Each reservoir has overflow and drain pipes. The fountains are of native rock, some of them built of hand-faced stone, while others are of rubble masonry, using bowlders or large-size gravel.

In most instances the fountains were constructed by the regular maintenance forces of the State highway department, though some were constructed by contractors' forces. In the erection of five fountains in Oregon by the United States Bureau of Roads, built according to the same general plans as the State fountains, it was found cement rubble masonry fountains cost from $\$ 200$ to $\$ 300$ each.

The fountains have bubblers for drinking, and a pool from which water for filling radiators may be dipped. When there is not a sufficient supply of water for bubblers a small pipe has been used for drinking pur-
poses. Many of these fountains have been built with a step at one of the bubblers, making it more convenient for children. The pools are used mostly for obtaining water for filling radiators, and a few of the fountains have a small pool constructed near the ground for the use of dogs or other small animals. Shut-off and drain valves are used so that proper care of the fountain may be taken during severe freezing weather.

The water used is from live springs and is tested by the State to ascertain its suitability for drinking before the erection of a fountain. Signs are placed about 300 feet each side of the fountains, calling attention to drinking water ahead, and at the fountains themselves ample parking space is provided so that cars may be parked off the highway.

It is found that these fountains are a great convenience to motorists, and they are used by a great many people during the summer months, particularly on the heavier traveled highways. Many are situated at scenic locations with tall evergreen trees and beautiful shrubbery, ferns, and wild flowers; and motorists find it a joy to tarry for a few minutes' rest. During the summer days there will generally be found one or more cars at each fountain, the travelers resting a minute or two and obtaining a drink of cold spring water, or filling a hot radiator with fresh cool water.

At present there are some 30 fountains along the Oregon highways, and additional ones are placed wherever suitable conditions are found. The construction of these fountains by the Oregon State Highway Commission is in line with their policy of developing the scenic features of the highways of the State.


Drinking-fountain Designs are Varied Accorping to Avallable Materials and the Water Supply


On Federal-aid Project No. 86 Between Ashland and Klamath Falls


On United States Highway Route No. 199 Near the California State Line


# DRINKING <br> FOUNTAINS <br> ON OREGON HIGHWAYS 




Many of the Fountains are in Attractive Locations Where Travelers Often Stop for a Short Rest

# FLOW OF FLOOD WATER OVER RAILWAY AND HIGHWAY EMBANKMENTS 

By DAVID L. YaRNELL, Serior Drainage Engineer United States Bureau of Public Roads, and FLOYD A. NAGLER, Professor of Hydraulic Engincering, State University of lowa

T${ }^{\top}$ HIS report presents the results of 572 experiments on the flow of water over full-size sections of railway and highway embankments, made for the purpose of developing formulas for use in calculating the quantity of flood water flowing over such embankments. The experiments were conducted by the Bureau of Public Roads of United States Department of Agriculture and the State University of Iowa at the University hydraulic laboratory. The rails and ties used were furnished by the Cedar Rapids and Iowa City Railroad, J. D. Wardle, chief engineer.

## THE PROBLEM OUTLINED

A knowledge of the quantity of water that flows down a given valley during extreme floods is often a matter of considerable importance to the engineer. It is one of the fundamental bases of design, not only of railroad and highway embankments across such valleys, but of works planned for the protection of agicultural land and of cities and towns. Engineers, theretore, should avail themselves of every opportunity to obtain reliable data on this suject.

Highway and railroad embankments across river valleys generally act as barriers to the flow of flood water and often are overtopped during the highest floods. At such an embankment the total discharge is the sum of that through bridge, trestle, and culvert openings, and that flowing over the embankment. The discharge through a bridge opening during a flood may be computed with a fair degree of accuracy if the drop-down at the bridge opening is known. A highway, railroad, or levee embankment over which water is flowing may be treated as a broad-crested weir. Therefore if the proper coefficient for a particular type of embankment is known the quantity of water flowing over an embankment of that type may be determined with reasonable accuracy. The specific purpose of this investigation was to determine these coefficients for certain types and widths of embankments so that the usual formulas for computing flow over broad-crested weirs could be employed.

## THE MODEL EMBANKMENT DESCRIBED

The hydraulic laboratory of the University of Iowa is located on the west bank of the Iowa River, at the university dam. The principal testing canal is 190 feet long, 10 feet wide, and 10 feet deep. At the upstream end of the canal, where it joins with the end of the dam, is a wooden head gate 10 feet wide by 12 feet deep. A 10 -foot weir of the suppressed type for measuring flow in the canal is located 60 feet downstream from the head gate. Numerous baffles were placed in the canal immediately below the head gate to obtain uniform velocity distribution as the water approached the weir, and a smooth flow over the crest. Similar baffles were placed in the canal immediately downstream from the weir to prevent commotion of the water as it approached the embankments.

[^0]

Figure 1.-Single-Track Embankment Without Rails


Figure 2.-Single-Track Embankment Without Rails, 50.4 Cubic Feet per Second Flowing Over Embankment, Head on Embankment, 1.42 Feet

A full-size section of a single-track embankment, 10 feet in length, was built in the testing canal 92 feet downstream from the weir. The embankment proper consisted of a wooden structure which was covered with gravel and upon which was placed the standard depth of gravel ballast and the ties and rails. The American Railway Engineering Association specifications for standard single and double track, gravel ballast, were followed. Seventy-pound rails were used. The top of rail of the single track was 3.8 feet above the floor of the canal. A view of the single-track embankment without rails is shown in Figure 1. The same embankment under test is shown in Figure 2. The embankment without rails corresponds to a highway with approximately a 12 -foot roadway.

In building the double track the extra track was added to the upstream side of the single track, the distance center to center of tracks being 13 feet. The height of the top of rail of the double track, however, was only 1.8 feet, as the floor here is 2 feet higher


Figure 3.-Double-Track Embankment With Rails in Place
than the floor in the downstream end of the canal. This difference is due in part to a false wooden floor placed in the canal for use in connection with other experiments. The double-track embankment with rails in place is shown in Figure 3, and Figure 4 shows the double-track embankment while under test. Corresponding views with one track removed are shown in Figures 5 and 6. This embankment, without rails, is representative of a highway with a roadway 23 feet wide.

Twenty-seven staff gages, graduated to hundredths of a foot, were placed along the walls of the canal at frequent intervals, 14 being placed along the west wall and 13 along the east wall. The zero of these staffs was set level with the top of rail.

A bear-trap weir 6 feet high and located 22 feet downstream from the center of the single track, or 28.5 feet from the center of the double track, was used to regulate the water level downstream from the embankment, and in some experiments to submerge the embankment. This weir was hung on hinges and was regulated by means of a block and tackle attached to a windlass.

It will be noted in Figure 3 that the downstream face of the embankment is grouted. This was done to prerent the material from washing off of the wooden base.

The side slopes of most highway and railroad embankments are covered with a heavy growth of grass and weeds which resist erosion.


Figure 4.-Double-Track Embankment With Rails in Place, 46 Cubic Feet per Second Flowing Over Tracks. Head on Upstream Rall, 1.33 Feet. Standing Waves Caused by the Rails

THE TESTS
Tests on each embankment were begun with a head of 0.5 foot of water discharging over the measuring weir, and continued with successive increases of 0.1 foot in head on the weir, until the maximum flow was obtained. For each head on the measuring weir the following conditions of outflow were imposed by means of the beartrap weir: (1) With water surface on the downstream side of the track raised until the ratio $\frac{d}{D}$ (depth of water over rail on the downstream side of the embankment, to depth on upstream side of embankment as illustrated on drawings) was about 0.95 ; (2) with $\stackrel{d}{D}$ less than 0.95 (obtained by gradually lowering the bear-trap weir); (3) with water discharging freely over the embankment, thus simulating free flow over one type of broad-crested weir.
In each test a constant head was first obtained on the measuring weir and then the bear trap was adjusted to obtain a definite ratio of depth of flow. When this had become constant, one observer first took readings on the weir hook gage and weir staff, then successively read the staff's downstream on one side of the channel and upstream on the other side. After determining that the readings on the two opposite upstream staffs


Figure 5.-Double-Track Embankment With Upstream Track in Place and Downstream Track Removed
checked. he again took readings on the weir hook gage and weir staff. In the meantime, another observer plotted a profile of the water surface. Horizontal lines 0.1 foot apart, as well as vertical lines 1 foot apart, had been painted on both sides of the canal. By means of these continuous gage lines (shown in fig. 3) an accurate profile of the many variations in the water surface was ohtained which it was not possible to obtain with the staff gages. This method gave a record of the waves caused by the rails as illustrated in Figures 4 and 6.

In testing the single-track embankment with rails in place it was noted that a standing wave was sometimes formed over the downstream rail. To determine the effect of the downstream rail on the flow some tests were run with only the upstream rail in place. On the double-track embankment tests were also run with the upstream track in place and the downstream track remored.

The results of the tests were computed as the experiments were under way, thus making it possible to rerun any doubtful tests.

## FOIRMULAS DERIVED FOR VARIOUS CONDITIONS OF FLOW

The following basic formula was used in all of the computations:

$$
Q=r L H^{3 / 2}
$$

$\qquad$
In this formula,
$Q=$ volume of discharge per unit of time,
$C=$ an empirical coefficient,
$L=$ length of the embankment overflowed,


Iigure 6.-Double-Track Embankment With Upstream Track in Place and Downstream Track Removed. 42.5 Cubic Feet per Second Flowing Over Track. Depth of Flow, 1.12 Feet Over Track
$H=$ the head corrected for the effect of the velocity of approach; i. e., measured depth plus $\frac{V}{2 g}$ where $V$ is the mean velocity of approach to the weir
The discharge over the measuring weir was computed by Bazin's weir formula. The readings on the various staffs gave the heights of the water above the top of rail. In the tests on the embankments with the rails removed the height of the rail was therefore added to the staff readings to get the depth of water on the embankment. For the tests in which the embankment was submerged the degree of submergence is expressed by the ratio $\frac{d}{D}$ before mentioned.

The results of the experiments were plotted on crosssection paper with the ratio $\frac{d}{D}$ as abscissas and the empirical coefficient $C$ as ordinates. These curves are shown in Figures 7 to 12, inclusive. It will be noted that when $\stackrel{d}{D}=0$, the water flowing over the embankment has a free drop. Under such conditions the coefficient $C$ varies more for some embankments than for others. For example, in Figure 11 the points are grouped whereas in Figure 8 they are more scattered. Since it was impossible to show all such values of the coefficient on the diagrams (many of them coincided), the points through which the curves are drawn were


Figure 7.-Relation Between Coffficient and Per Cent Submergence, Single Track, Both Rails in Place


Figure 8.-Relation Between Coefficent and Per Cent Submergence, Single Track, Downstream Rail Removed


Figure 9.-Relation Between Coefficient and Per Cent Submergence, Single Track, Both Rails Removed. Equivalent to a Highway Embankment With a 12-Foot Roadway


Figure 10.-Relation Between Coefficient and Per Cent Submergence, Double Track, Four Rails in Place


Figure 11.-Relation Between Coeffictent and Per Cent Submergence, Double Track, Two Downstream Rails Removed


Figure 12.-Relation Between Coefficient and Per Cent Submergence, Double Track, All Four Ralls Removed. Equivalent to a Highway Embankment With a 23-Foot Roadway


Figure 13-Variation of Coefficient With Head. Single Track


Figure 14.-Variation of Coefficient With Head. Double Track
determined in the following manner: Values, the discharges, $O$, were plotted on logarithmic paper as ordinates and the heads, corrected for velocity of approach, as abscissas. A line having a slope of $\dot{3} / 2$ was drawn through the majority of the points, the greater weight being placed on the higher heads. The intercept on the unity vertical axis was taken as the value of $C$ through which the curve should pass.
During the progress of the tests it was observed that for values of $\frac{d}{1}$ less than 0.6 , the coefficient was practically constant since the elevation of the tailWater had no eflect on the height of the headwater. This was due to the fact that the critical velocity occurred at the downstream edge of the embankment. Therefore, to conserve time in experimenting on some of the embankments, comparatively few tests were run with values of ${ }_{D}^{d}$, between 0.1 and 0.5 .

For those tests in which the water had a free drop over the embankment it was noted that the coefficient varied somewhat with the head when the exponent of $H$ Was taken as $3 / 2$. This variation is shown in Figures 13 and 14 in which the coefficients have been plotted as ordinates and the heads as abscissas. These curves show that the exponent of $H$ is not exactly 1.5. The test data were therefore plotted on logarithmic paper with the discharge $O$ as ordinates and the head $H$ (corrected for velocity of approach) as abscissas. Lines were drawn through the points and the discharge equations determined for the rarious types of embankments.

The equations as determined for free flow over the embankments are as follows:

Single track, both rails in placeQ $=3.27 \mathrm{LH}^{1.47}$
Single track, upstream rail oniy in placeQ $=3.66 L^{1.37}$
Single track, both rails removed$Q=3.00 \mathrm{LH}^{1.49}$
Double track, all four rails in place$Q=2.95 L I^{1.47}$
Double track, upstream track only in place$Q=3.17 L H^{1.48}$
Double track, all four rails removed$Q=2.66 \mathrm{LII}^{1.60}$

## use of formulas illustrated by examples

These experiments have made available coefficients for use in hydraulic formulas for computing the flow over embankments during tloods. When free flow exists over an embankment one of formulas (2) to (7) may be used, depending upon the type of the embankment. If the embankment is submerged the appropriate coefficient may be taken from the curves shown in Figures 7 to 12, after the percentage of submergence has been determined. In all of the computations for the curres and formulas the head corresponding to the velocity of approach has been added to the depth of water over the embankment to get the true head. Thus these coefficients are applicable to all embankments regardless of the velocity of approach. In a specific case, however, the velocity of approach must be assumed in the preliminary calculations and the assumed value checked after the discharge has been computed. This procedure can best be illustrated by practical examples.

Example 1.-Assume that water has been discharging over a single-track railroad embankment. The track at elevation 75 was not washed out. After the flood a survey for high-water marks showed that the water on the upstream side of the track had been at elevation 76.6, and on the downstream side of the track at eleration 74. Assume the height of the embankment (to top of rail) above the natural ground surface to be 6.5 feet, and for convenience consider a length of 100 feet of track. The formula applicable to this problem is equation (2) which is

$$
Q=3.27 L I^{1.47}
$$

in which

$$
L=100 \text { feet. }
$$

The measured head is 1.6 feet. Assume the velocity of approach to be 0.80 foot per second, then
and

$$
\begin{aligned}
& \text { Velocity head }=0.0099 \\
& \text { Total head, } I=1.61 \\
& Q=3.27 \times 100 \times 1.61^{1.47} \\
& Q=658 \text { cubic feet per second. }
\end{aligned}
$$

or
But the cross-sectional area of flow is 810 square feet and

$$
\frac{Q}{A}=\frac{658}{810}=0.81 \text { foot per second, }
$$

which approximates the assumed velocity of approach.
Example 2.-A single-track railroad was overtopped by flood water. A survey showed the track to be at elevation 50 , the water level upstream at 53 , and downstream at 52.5 . The height of the embankment or top of rail is 3.5 feet above the natural ground surface, and the length of track overflowed 100 feet. The percentage of submergence would then be

$$
\frac{d}{D}=\frac{2.5}{3.0}=83.3 .
$$

From Figure 7 the coefficient, $C$, is found to be 2.80 . Hence the formula would be

$$
\begin{aligned}
& Q=2.80 L H^{3 / 2} \\
& L=100 .
\end{aligned}
$$

The measured head is 3 feet. Assuming the velocity of approach to be 2 feet per second, the velocity head is 0.0622 and the total head, $H$, is 3.0622 .

Thus

$$
\begin{aligned}
& Q=2.80 \times 100 \times 3.0622^{3 / 2} \\
& Q=1,500 .
\end{aligned}
$$

The area of flow is 650 square feet and the velocity of approach is $\frac{1,500}{650^{-}}=2.31$ feet per second. The first assumption being too small, assume the velocity of approach as 2.3 feet per second. The velocity head then is 0.0822 ; and the total head, $I I=3.0822$.

$$
\begin{aligned}
& Q=2.80 \times 100 \times 3.0822^{3 / 5} \\
& Q=1,515 .
\end{aligned}
$$

The velocity of approach is ${ }^{1,515} 650=2.33$ feet per second which agrees quite closely with the second assumption. Thus the discharge over a 100 -foot section of the track is 1,515 cubic feet per second, or 15.15 cubic feet per second per lineal foot of track overflowed.

## PLANS FOR INTERNATIONAL ROAD CONGRESS ANNOUNCED

The American Organizing Commission announces that the Sixth International Road Congress will be held in this country in October, 1930, at the invitation of the United States Government, with the object of continuing the studies begun in Paris in 1908 and continved in Brussels in 1910, in London in 1913, in Seville in 1923, and in Milan in 1926.

The congress will open in Washington on Monday, October 6, and close on Saturday, October 11.

Simultaneously with the congress it is contemplated that a private international road exhibition will be held by the American Road Builders' Association.

## TENTATIVE PROGRAM OF THE CONGRESS

The official opening of the congress will occur Monday afternoon, October 6, at Constitution Hall, preceded by a meeting in the morning of the International Permanent Commission at the offices of the Chamber of Commerce of the United States.

Registration will begin Monday morning and continue through Thursday, October 9. Members are urged to register as early during the week as possible.

All sessions of the congress, except the official opening, will be at the building of the Chamber of Commerce of the United States.

Section meetings will be held the morning and afternoon of October 7, the morning sessions being followed by motion-picture exhibits of road-building operations.
The same procedure will hold for Wednesday, October 8, and for Thursday, October 9.

Friday is reserved for a visit to the experiment station of the United States Bureau of Public Roads, and over Mount Vernon Memorial Highway to Mount Vernon, home of George Washington.

Saturday morning, October 11, will see the official closing of the congress, with the afternoon of that day reserved for excursions, to be followed by sight-seeing tours on Sunday, October 12.

The detailed program will be announced at a later date.

## AGENDA

(Prepared by the Permanent International Commission)

## First Section

CONSTRUCTION AND MAINTENANCE
First question.-Results obtained by the use of:
(a) Cement;
(b) Bricks or other artificial paving.
(Methods employed for road construction and maintenance in these materials.)

Second question.-The most recent methods adopted for the use of tar, bitumen, and asphalt in road construction.

Third question.-The construction of roads in new countries, such as colonies and undeveloped regions.

## Second Section

TRAFFIC AND ADMINISTRATION
Fourth question.-Ways and means of financing highways:
(a) Road construction;
(b) Maintenance.

Fifth question.-Highway transport: Correlation and coordination with other methods of transport; adaptation to collective (organizations) and individual uses.

Sixth question.- (1) Traffic regulation in large cities and their suburbs; traffic signals; design and layout of roads and adaptation to traffic requirements in built-up areas. (2) Parking and garaging of vehicles.

There probably will be a number of unofficial luncheons and dinners tendered the delegates and members by various organizations. The jurisdiction of the American Organizing Commission, however, is limited to the official events appearing on the program.

## CONDITIONS FOR PARTICIPATION IN THE CONGRESS

In addition to the delegates of the various countries, expressly appointed by their respective Governments, the permanent and temporary members of the Permanent International Association of Road Congresses may take part in the Congress.

Permanent members.- The permanent members of the association are members who have the right to participate in all the road congresses. Individuals or representatives of groups already enrolled as permanent members are required only to notify the American Organizing Commission at Washington of their intention to participate in the sixth congress. (Blank forms for this purpose will be supplied upon request.)

Individuals or groups desiring to be enrolled as permanent members of the association should request suitable application forms from one of the addresses given below. ${ }^{1}$

For individual permanent members the regular annual dues are 25 French francs (\$1). These dues, however, are increased to 125 French francs (\$5) for new members enrolled during a congress year, as in the case for the year 1930. For life membership the subscription fee is 500 French francs (\$20).

For groups, the minimum annual dues in any year are 100 French francs (\$4). For each 100 French francs paid, one delegate may be sent to the congress. All fees received from the above classes of membership are paid to the Permanent International Association of Road Congresses.

Temporary members.-Individuals or groups who do not belong to the association and who desire only to participate in the sixth congress, are considered as temporary members of the association, under articles 2,8 , and 10 of the statutes. Honorary members are enrolled as temporary members.

The fee for enrollment in the congress for each temporary member is 125 French francs (\$5). The payment of 250 French francs (\$10) bestows the right to the title of honorary member. Threc-fifths of the fees received from temporary members are allocated to the American Organizing Commission and two-fifths to the Permanent International Association of Road Congresses:

Applications for temporary membership should be made on a form which can be obtained from any of the addresses given in footnote 1

Membership privileges.-All classes of members mentioned above have the right to attend all the meetings of the congress, and to participate in all official receptions and official excursions during the congress proper.

[^1]They are also entitled to the special transportation rates established for attendance at the congress.

They will receire, previous to the opening of the congress, the official papers on the subjects indicated in the program, and after the congress a copy of the proceedings. These publications can be obtained only through membership in the congress.

The executive bureau, and the American Organizing Commission, however, do not undertake to send the papers previous to the opening of the congress to those members who have not been enrolled at least one
month prior to the opening. (Art. 11 of the statutes of the association.)

A circular is available for distribution giving information concerning reductions in rates of ocean steamship lines, passports, and hotel accommodations and rates. Hotel reservations may be made through the American Organizing Commission.

A railroad rate of three-fourths the regular round-trip fare has been arranged for members attending the congress.

## MOTOR VEHICLE REGISTRATIONS AND REVENUE AND GASOLINE TAXES IN 1929

According to reports received from State registration authorities by the Bureau of Public Roads, a total of 26,501,443 motor vehicles were registered in 1929. From the owners of these vehicles the States and the District of Columbia collected in license fees, registration fees, permit fees, fines, etc., the sum of $\$ 347$,843,543.

The registration figure includes passenger automobiles, taxis. buses, motor trucks and road tractors, trailers, and motor cycles, and represents an increase of $2,008,319$ or 8 per cent over the 1928 figure. The total fees collected represent an increase of $\$ 25,213,518$ over the 1928 figure. After deducting $\$ 24,505,737$ for collection and miscellaneous purposes, the balance of $\$ 323,337,806$ was applied to highway purposes; $\$ 223$,292,969 to State funds, $\$ 66,861,364$ to local funds, and $\$ 33,183,473$ to State and county bond funds.

The States having the 10 highest registration figures are as follows: New York, 2,263,259; California 1,974,341; Ohio, 1,766, 614; Pennsylvania, 1,733,283; Illinois, $1,615,088$; Michigan $1,395,102$; Texas, $1,348,107$; Indiana, 866,715; New Jersey, 832,332 ; and Massachusetts, 817,704.

In percentage gain, the District of Columbia and New Mexico each show 19 per cent. Nevada reports a gain of 16 per cent, Arizona 15 per cent, and Utah 14 per cent. Four States, Georgia, Maryland, Massachusetts, and Tennessee, each show a gain of 12 per cent, and three States, Michigan, Montana, and Texas each report an increase of 11 per cent. California, Idaho, Kentucky, New Jersey, and Washington each show a gain of 9 per cent.

Details as to the number of registrations are shown on page 37 and registration receipts are shown on page 38.

## gasoline tax yields large revenue for highways

Reports received by the Bureau of Public Roads show that $\$ 431,636,454$ were collected in taxes on the sale of $13,400,180,062$ gallons of motor fuel in 1929. These figures include a 12 -month collection in 46 States and the District of Columbia, a 5 -month collection in Illinois, and the collections of eight months in New York. The entrance of the latter two States into the
list of those collecting a gasoline tax marks the adoption of this method for part payment of the highway bill by all States. The pioneer States-Oregon and Colorado-led the way in 1919. All others have followed in the succeeding period of 11 years, but the tax did not become effective in New York until May 1 and in Illinois until August 1.
The average fee per gallon was 3.22 cents as against 3 cents in 1928, 20 States having increased the rate of taxation either 1 or 2 cents. The highest tax per gallon was 6 cents; the lowest 2 cents. At the close of the year, 3 States had a 6 -cent tax; 8 a 5 -cent tax; 19 a 4 -cent tax; 1, Utah, a $31 / 2$-cent tax; 10 a 3 -cent tax; and 7 States and the District of Columbia a 2-cent tax.

During the year the rate per gallon was increased 1 cent in Colorado, Florida, Indiana, Kansas, Minnesota, North Carolina, North Dakota, Ohio, Oklahoma, Pennsylvania, South Carolina, Vermont, Washington, and Wyoming; a 2-cent increase became effective in Georgia, Louisiana, Montana, Nebraska, Tennessee, and Texas.

Comparison of the total number of vehicles registered in 1929, with the total tax collected and with the taxable gallonage in all States (except New York and Illinois) and in the District of Columbia shows an average tax revenue of $\$ 17.72$ per vehicle and an average purchase of 532 gallons of gasoline.

After deducting collection costs, the entire net revenue in 34 States was used for construction and maintenance of rural roads. In the other 14 States and the District of Columbia, a total of $\$ 24,405,207$ was used for other purposes. In three States a portion was used for public-school purposes. In eight States, a part of the revenue went to cities for repair and improvement of streets, as did the entire collection for the District of Columbia. In six States, small sums were deposited in general funds; in Mississippi, a special extra tax was collected in two counties for seawall protection of highways; and in New Jersey a small portion of the receipts was turned over to the department of commerce and navigation.

Of the portion of the total revenue applied to rural road purposes, $\$ 297,967,756$ was used for construction and maintenance of State highways; $\$ 85,113,708$ for construction and maintenance of local roads; and the balance of $\$ 23,371,785$ was used for payments on State and county road bonds.

The table on page 39 shows the total tax earnings and total number of gallons taxed in the various States.
MOTOR VEHICLE REGISTRATIONS, $1929^{1}$
[Compiled for calendar year from reports of State authorities]

| State | Registered motor vehicles individually and commercially owned ${ }^{3}$ |  |  | Other registered vehicles |  | Tax-exempt official motor cars and motor cycles |  |  | Number of licenses or permits |  | Total registered motor cars and trucks, in 1928 | Year's change in motorvehicle registrations |  | State |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Grand total registered motor cars and trucks | $\begin{aligned} & \text { Passenger } \\ & \text { automobiles, } \\ & \text { taxis, and } \\ & \text { buses }{ }^{3} \end{aligned}$ | Motor trucks and road tractors | Trailers ${ }^{4}$ | Motor cycles | United States cars ${ }^{6}$ | State and local cars | Motor cycles (official) | Dealers | Operators and chauffeurs |  | Number increase or decrease (-) | Per cent |  |
| Alabama | 285, 533 | 247, 701 | 37, 832 | 3,024 | 704 | 167 | 869 |  | 4,712 | 2,714 | 269, 519 | 16,014 | 6.0 | Alabama. |
| Arizona | 109,013 | 98, 327 | 10,686 | 1,249 | 396 | 176 | 1,173 | 43 | 182 | 20,643 | 94, 372 | 14,641 | 15. 5 | Arizona. |
| Arkansas. | 233, 128 | 193,396 1,760308 | 39, 732 | 2, 808 | 400 | 39 217 | 15,005 |  | 14, 515 | $\begin{array}{r}5,023 \\ 250 \\ \hline 855\end{array}$ | 214,931 $1,799,890$ | 18, 1974 | $\begin{aligned} & 8.5 \\ & 9.7 \end{aligned}$ | ${ }_{\text {A }}^{\text {Arkansas. }}$ California. |
| Colorado. | 1, 303, 489 | 1, 274,988 | 28, 501 | 42, 159 | 1,142 | 1,283 |  |  | 3, 684 | 7,916 | 284, 867 | 18, 622 | 6.5 | Colorado. |
| Connecticut | 328, 063 | 278, 057 | 50,006 | 324 | 2, 314 | 71 | 2, 068 | 270 | 4, 073 | 379, 122 | 309, 792 | 18,271 | 5. 9 | Connecticut. |
| Delaware. | 54,960 | 44,728 | 10, 232 | 389 | 308 | 44 | 3,788 | 204 | + 684 | 60,887 2 326 | 51,210 352,961 | $\begin{array}{r}3,750 \\ -6,984 \\ \hline\end{array}$ | 7.3 -2.0 | Delaware. |
| ${ }_{\text {Georgia }}$ | 345, 977 | 288, 381 | 57, 48,543 | 2,597 | 1,104 1,138 | 934 | 3,788 |  |  | 2,973 | 318, 856 | 40,049 | 12.6 | Georgia. |
| Idaho.. | 118, 074 | 104, 398 | 13, 676 | 485 | 355 | 103 | 1,092 | 15 | 466 | 780 | 108, 154 | 9,920 | 9.2 | Idaho. |
| Illinois | 1,615,088 | 1, 411,753 | 203, 335 | 5, 068 | 6, 055 | 979 |  |  | 4, 605 | 106, 551 | 1, 504, 359 | 110, 729 | 7.4 | Illinois. |
| Indiana | 866, 715 | 741,366 | 125, 349 | 10,407 | 2,983 | 3, 184 | 4,965 |  | 2, 483 | 55, 19,067 | 823, 466 | 50,984 | 5. 9 | Indiana. |
| Kansas | 581, 223 | 507, 529 | 673,694 | 360 | 1, 178 | 192 | 2,470 | 60 | 2,784 |  | 533, 799 | 47,424 | 8.9 | Kansas. |
| Kentucky | 332, 848 | 298, 716 | 34, 132 |  | 727 | 90 | 1,916 |  | 1,158 | 11,859 | 304, 231 | 28, 617 | 9.4 | Kentucky. |
| Louisiana | 280, 868 | 234, 565 | 46, 303 | 8,000 | 600 | 64 |  | 82 | 1,151 | 226,087 | 172, 638 | 11,868 | 6. 9 | Maine. |
| Maryland | 319,873 | 281,034 | ${ }^{7} 38,839$ | 876 | 1,986 | 1,969 |  |  | 7,231 | 82, 714 | 285, 311 | 34, 562 | 12.1 | Marylan |
| Massachusetts | 817, 704 | 719, 436 | 98, 268 | 556 | 5, 568 | 556 | 900 |  | 3, 094 | 944, 338 | 726, 295 | 91, 409 | 12.6 | Massachusetts. |
| Michigan | 1, 395, 102 | 1, 219, 158 | ${ }^{6} 175,944$ | 31,577 | 3,985 | 371 |  |  | 2, 282 |  | 1, 249, 221 | 145, 881 | 11.7 | Michigan. |
| Minnesota | 730, 399 | 630,703 | 99,696 | 4,783 | 1,900 | 252 | 1,304 |  | 2,160 |  | 673, 573 | 56, 826 | 8.4 | Minnesota, |
| Mississipp | 250, 011 | 217, 671,237 | 32,649 685,443 | 4,321 2,686 | 1,893 | 74 311 | 1,604 | 5 | 2, 686 | 33, 265 | 712,965 | 43,715 | 6.1 | Missouri |
| Montana | 140, 387 | 115, 285 | 25, 102 |  | , 234 | 229 |  |  | 537 | 414 | 126, 035 | 14,352 | 11.4 | Montana. |
| Nebraska | 418, 226 | 375, 946 | 42, 280 | 6,515 | 972 | 226 | 1,101 |  | 3, 584 |  | 391, 355 | 26, 871 | 6.9 | Nebraska. |
| Nevada. | 31,915 | 25, 302 | 6, 613 | 273 | 95 | 42 | 468 |  | 119 |  | 27, 376 | 4, 5339 | 16.6 | Nevada. |
| New Hampshire | 108,880 | 94, 900 | 13,980 133,373 | 635 2,424 | 1,263 | 22 708 | 7,173 | 933 | 3,418 | 1291, 235 | 102,644 | 73,902 | 9.7 | New Jamps |
| New Jersey- | 832,332 | 698,959 76,000 | 133,373 2,374 | 2,424 | - 203 | 156 | , 820 | , | 3, 218 |  | 65,737 | 12, 637 | 19.2 | New Mexico. |
| New York. | 2, 263, 259 | 1,922,068 | 341, 191 | 8, 232 | 13, 527 | 1,666 | 17,499 | 1,313 | 5, 039 | 2, 708, 036 | 2, 083, 942 | 179, 317 | 8.6 | New York. |
| North Carolina | 483, 602 | 430,651 | 52,951 | 3,584 | 1, 298 | 429 | 7,092 |  | 7,614 |  | 464, 376 | 19, 226 | 4.1 | North Carolina. |
| North Dakota | 188, 046 | 162,092 | 25,954 | 15,000 | 9 230 | - 3 | 10.843 |  | ${ }^{29}{ }^{8} 298$ | 4,436 | 1,649,699 | 116,915 | 7.1 | Ohio. |
| Oklahoma | 1, 570,791 | -510,401 | 60,390 |  | 1,196 | 2,530 | 4,725 |  | 3,923 |  | 529, 843 | 40,948 | 7.7 | Oklahoma. |
| Oregon. | 269,007 | 247, 131 | 21, 876 | 1,308 | 1,694 | 141 | 1,483 | 102 | ${ }_{6}^{633}$ | 52,451 | -248, 118 | 20,889 | 8. 4 | Oregon. |
| Pennsylvania | 1,733, 283 | 1,515,875 | 217, 408 | 5,067 | 13, 375 | 1,383 | 1,057 |  | 29,599 | 2, 153, 944 | 1, 642, 207 | 91, 076 | 5.5 | Pennsylvania. |
| South Dakota. | 204, 199 | 181, 419 | ${ }^{6} 22,780$ |  | 207 | 85 | 999 |  | 1,110 |  | 191, 374 | 12,825 | 6. 7 | South Dakota. |
| Tennessee | 362, 431 | 329, 697 | ${ }^{6} 32,734$ |  | 1,203 | 132 | 4,377 |  | 711 |  | 322, 137 | 40, 294 | 12.5 | Tennessee. |
| Texas | 1,348, 107 | 1, 165, 150 | 182,957 | 16, 260 | 4,016 | 2, 505 |  |  | 4,423 | 14,897 | 1, 214, 297 | 133, 810 | 11.0 | Texas. |
| Utah | 112, 661 | 95, 661 | 17,000 |  | 525 | 173 | 1,200 |  |  |  | 98, 541 | 14, 120 | 14.3 | Utah |
| Vermont | 93, 030 | 84,471 | 8,559 | 280 | 487 | 28 |  |  | 4,561 | 10,376 | 360, 545 | -6,660 |  | Vermont. |
| Virginia-- | 387, 205 | 328, 525 | 58,680 | 966 | $\stackrel{2}{2,141}$ | 1,141 | 4, 349 | 180 | 5,109 | 460, 622 | 360,545 402,875 | 39,466 | 9.8 | Virginia. <br> Washington. |
| W est Virginia | 268, 888 | 228, 715 | 40, 173 | , 725 | 1, 397 | 33 | 2,388 | 120 | 11, 854 | 85, 514 | 251, 556 | 17,332 | 6.9 | West Virginia. |
| Wisconsin_ | 793, 502 | 689, 447 | 104, 055 | 497 | 2,723 | 92 |  | 128 | 3, 138 |  | 742,135 | 51, 367 | 6. 9 | isconsin. |
| W yoming- ${ }_{\text {District of }}$ | 60,680 151,450 | 51,880 135,455 | 8,800 15,995 |  | 1, 92 | 209 837 | 1,586 | 183 | 2, 107 | 63, 921 | 56,336 126,556 | 4,344 24,894 | 19.7 | District of Columbia. |
| Tota | 26, 501, 443 | 23, 121, 589 | 3, 379, 854 | 193, 044 | 114, 845 | ${ }^{8} 33,179$ | 118, 828 | 4, 551 | 188, 719 | 9, 143, 364 | 24, 493, 124 | 2, 008, 319 | 8.2 | Total. |

MOTOR VEHICLE REGISTRATION FEES, LICENSES, PERMITS, FINES, ETC., 19294


[^2]$|$| Net gallons of |
| :---: |
| gasoline taxed, |
| and used hy |
| motor velicles |




8, 178


297, 967, 756 85, 113,708
$\qquad$Construction and mainte-
nance on rural roads


[^3]11 For town and city streets.
resenve.
12 For public schools.
${ }_{20}$ Consists of $\$ 400,000$ for city streets and $\$ 3,420$ (from dealers' licenses) to state general fund. Exemption claimed upon purchase.
For state department of commerce and navigation.
For last 8 months of year.
0 Includes $4,025,392$ gallons of distillate taxed at $21 / 2$ cents.
1 Consists of $\$ 652,833$ for state bond payments and $\$ 1,935$,
品
${ }_{3}$ Payments for State highway bonds except as noted.
${ }_{8}^{1}$ Includes $1,075,385$ galions from state budget, $\$ 14,433$.
6 No refunds reported this year.
8 Consists of $\$ 51+439$ for State bonds and $\$ 1,516,694$ for county bonds.
8 Reserve for refunds.
7 Consists of $\$ 50,000$ for reserve for refunds and $\$ 951,870$ for New York City.
To Consists of $\$ 75,000$ for rotary fund for refunds and $\$ 6,122,43$ for municipal st reets.
Tax increased to 4 cents on Jan. 1,1930 .
Consists of $\$ 652,833$ for state bond payments and $\$ 1,935,2+9$ for county bonds.
Free school fund.
State appropriation of $\$ 11,088$.
Paid from motor-vehicle fund, $\$ 7,000$.
For repair and improvement of $W$ ash
rade crossings
Charged to state general fund, $\$ 24,504$.
35 For repair and improvement of Washington streets.
used in Aproximate grand total of gallons of gasoline consumed hy motor vehicles allowing for estimated gallons
und New lork during early mont hs in year when no tax was assessed.

# SOIL SAMPLING WITH A COMPRESSED AIR UNIT 

By H. F. BLANEY, Associate Irrigation Engineer, and C. A. TAYLOR, Assistant Irrigation Engineer, Division of Agricultural Engineering, Bureau of Public Roads


Figure 1.-Compressor Unit Mounted on Truck

In making investigations regarding rainfall penetration in southern California,


Figure 2.-Air Hammer

## hose.

 the authors ${ }^{1}$ are conducting soil moisture studies in which soil samples are taken at 1 foot intervals to a depth of 18 feet, using the improved soil tube. ${ }^{2}$ The success of this work depends, to a considerable extent, upon the speed with which a large number of samples can be obtained, and since the physical effort required may become the limiting factor in obtaining samples at considerable depths, a compressed-air unit has been adapted for use in driving the tubes in order that as many samples as desired can be taken quickly and easily. A description of the apparatus is presented as it may be of interest to investigators in several fields, such as that of highway encincering.The apparatus consists of a compressor unit mounted on a truck, a light air hammer, and a 100 -foot length of

[^4]paratus and the manner in which soil tubes are driven. The compressor unit consists of a 2 -cylinder air compressor coupled directly to a 4 -cylinder gasoline engine with a self-starter. The air receiver and gasoline tank are mounted in front of the compressor. The complete unit is mounted on a channel iron frame and bolted to one side of the truck floor, thus leaving one-half of the truck body space available for carrying other equipment. A reel with 100 feet of $1 / 2$-inch rubber hose is mounted at the back of the truck, and special hose couplings permit connections to be quickly made to the hammer and compressor. An air pressure control automatically maintains any desired pressure up to 150 pounds per square inch and the displacement of the compressor is 59 cubic feet per minute at 800 revolutions per minute.

The hammer is of the clay-digger type, capable, when working under an air pressure of 100 pounds per square inch, of delivering 2,250 blows per minute, each blow striking with a force of 16 foot-pounds. Extending down in to the soil tube is a 6 -inch guide rod, the shoulder of which may be seen resting against the end of the soil tube in Figure 2. The trigger grip gives the operator ready control of the hammer, and by properly cramping the guide rod in the end of the soil tube, very little vibration is transmitted to the arms of the operator. The jack ${ }^{3}$ used for pulling the soil tubes is shown on the ground in Figure 1.

The set-up for sampling is quickly made, as it is only necessary to reel the hose out to the desired location and snap it onto the compressor. This unit has been in use since October, 1929, and has proved very satisfactory. It is estimated that the time of sampling has been cut to one-third of that required for handwork.

[^5]
## ROAD PUBLICATIONS OF BUREAU OF PUBLIC ROADS



## ANNUAL REPORTS

Report of the Chief of the Bureau of Public Roads, 1924.
Report of the Chief of the Bureau of Public Roads, 1925.
Report of the Chief of the Bureau of Public Roads, 1927.
Report of the Chief of the Bureau of Public Roads, 1928.
Report of the Chief of the Bureau of Public Roads, 1929.

## DEPARTMENT BULLETINS

No. *136D. Highway Bonds. 20c.
257D. Progress Report of Experiments in Dust Prevention and Road Preservation, 1914.
*314D. Methods for the Examination of Bituminous Road Materials. 10 c .
*347D. Methods for the Determination of the Physical Properties of Road-Building Rock. 10c.
*370D. The Results of Physical Tests of Road-Building Rock. 15c.
386D. Public Road Mileage and Revenues in the Middle Atlantic States, 1914.
387D. Public Road Mileage and Revenues in the Southern States, 1914.
388D. Public Road Mileage and Revenues in the New England States, 1914.
390D. Public Road Mileage and Revenues in the United States, 1914. A Summary.
407D. Progress Reports of Experiments in Dust Prevention and Road Preservation, 1915.
463D. Earth, Sand-Clay, and Gravel Roads.
*532D. The Expansion and Contraction of Concrete and Concrete Roads. 10 c.
*583D. Reports on Experimental Convict Road Camp, Fulton County, Ga. 25c.
*660D. Highway Cost Keeping. 10c.
*670D. The Results of Physical Tests of Road-Building Rock in 1916 and 1917.
*691D. Typical Specifications for Bituminous Road Materials. 10 c .
*724D. Drainage Methods and Foundations for County Roads. 20c.
1216D. Tentative Standard Methods of Sampling and Testing Highway Materials, adopted by the American Association of State Highway Officials and approved by the Secretary of Agriculture for use in connection with Federal-aid road construction.
1259D. Standard Specifications for Steel Highway Bridges, adopted by the American Association of State Highway Officials and approved by the Secretary of Agriculture for use in connection with Federalaid road work.
1279D. Rural Highway Mileage, Income, and Expenditures 1921 and 1922.
1486D. Highway Bridge Location.

## DEPARTMENT CIRCULARS

No. 94C. T. N. T. as a Blasting Explosive.
331C. Standard Specifications for Corrugated Metal Pipe Culverts.

## TECHNICAL BULLETIN

No. 55. Highway Bridge Surveys.

## MISCELLANEOUS CIRCULARS

No. 62M. Standards Governing Plans, Specifications, Contract Forms, and Estimates for Federal-Aid Highway Projects.
*93M. Direct Production Costs of Broken Stone. 25c.
*109M. Federal Legislation and Regulations Relating to the Improvement of Federal-Aid Roads and NationalForest Roads and Trails. 10c.

## SEPARATE REPRINTS FROM THE YEARBOOK

No. 914Y. Highways and Highway Transportation.
937Y. Miscellaneous Agricultural Statistics
1036Y. Road Work on Farm Outlets Needs Skill and Right Equipment.

## TRANSPORTATION SURVEY REPORTS

Report of a Survey of Transportation on the State Highway System of Connecticut.
Report of a Survey of Transportation on the State Highway System of Ohio.
Report of a Survey of Transportation on the State Highways of Vermont.
Report of a Survey of Transportation on the State Highways of New Hampshire.
Report of a Plan of Highway Improvement in the Regional Area of Cleveland, Ohio.
Report of a Survey of Transportation on the State Highways of Pennsylvania.

## REPRINTS FROM THE JOURNAL OF AGRICULTURAL RESEARCH

Vol. 5, No. 17, D- 2. Effect of Controllable Variables upon the Penetration Test for Asphalts and Asphalt Cements.
Vol. 5, No. 19, D- 3. Relation Between Properties of Hardness and Toughness of Road-Building Rock.
Vol. 5, No. 24, D-6. A New Penetration Needle for Use in Testing Bituminous Materials.
Vol. 6, No. 6, D- 8. Tests of Three Large-Sized ReinforcedConcrete Slabs Under Concentrated Loading.
Vol. 11, No. 10, D-15. Tests of a Large-Sized Reinforced-Concrete Slab Subjected to Eccentric Concentrated Loads.

[^6]


[^0]:    ${ }^{1}$ Acknowledgment is made to Prof. S. M. Woodward, State University of Iowa, fou valuable advice and suggestions, and to P. L. Hopkins, junior civil engineer, Burea ${ }_{t}$ of Public Roads, and Nolan Page, graduate research assistant in hydraulics, Sta ${ }_{\text {of }}$ University of Iowa, who assisted in the laboratory work and in the computation esults.

[^1]:    ${ }^{1}$ The address of the secretary general of the Permanent International Association of Road Congresses is M. Le Gavrian, secretary general, Permanent International Association of Road Congresses, 1, Avenue d'Iéna, Paris, XVI, France. The address of the secretary general of the American Organizing Commission is Thos. H. NacDonald, secretary general, American Organizing Commission, 1723 N Street N W, Washington, D. C., U. S. A. The address of the secretary of the British Organizing Committee is E. B. Hart, secretary, British Organizing Committee, 7 Whitehall Gardens, London. S. W. 1, England,

[^2]:    Financial data only on this table. For number of repistrations, etce, see preceding page. "detail totals."
    Fightstates thus noted do not report complete details, and reeeipts are not included in "der
    States for which no figures are shown make appropriations for administration. States for whieh no figures are shown make appropp
    Payments on state hiighway bonds, exept as noted.
    Tamdes
    
    10 County bond payments assumed by State.

[^3]:    1 This is the net gasoline tax earned after deduction of refunds for exemptions according to law.
    2 Many States pay collections costs from other State funds, and when the amounts are reported they are

[^4]:    ${ }^{1}$ Under the supervision of W. W. McLaughlin, Associate Chief of the Division of Agricultural Engineering, Bureau of Public Roads, United States Department of Agriculture, in cooperation with the State of California, Department of Public W orks. "An mproved soi-sampling Tube," by F. J. Veihmeyer, Soil Science, Vol XXV1I, Ňo. 2 (February, 1929).

[^5]:    ${ }^{3}$ "An Efficient Soil Tube Jack," by C. A. Taylor and H. F. Blaney, Soil Science, Vol. XXVII, No. 5 (May, 1929).

[^6]:    * Department supply exhausted.

